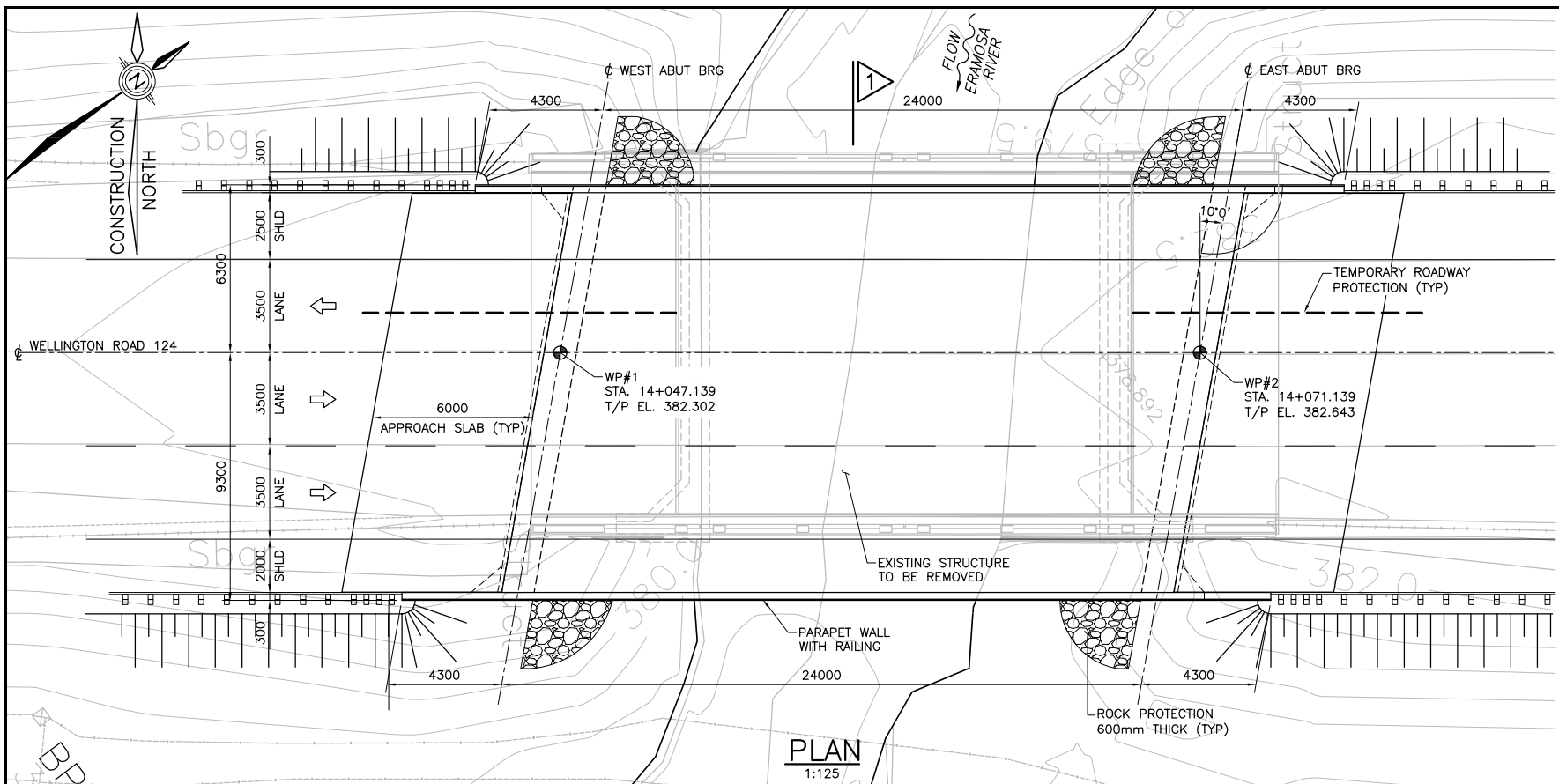


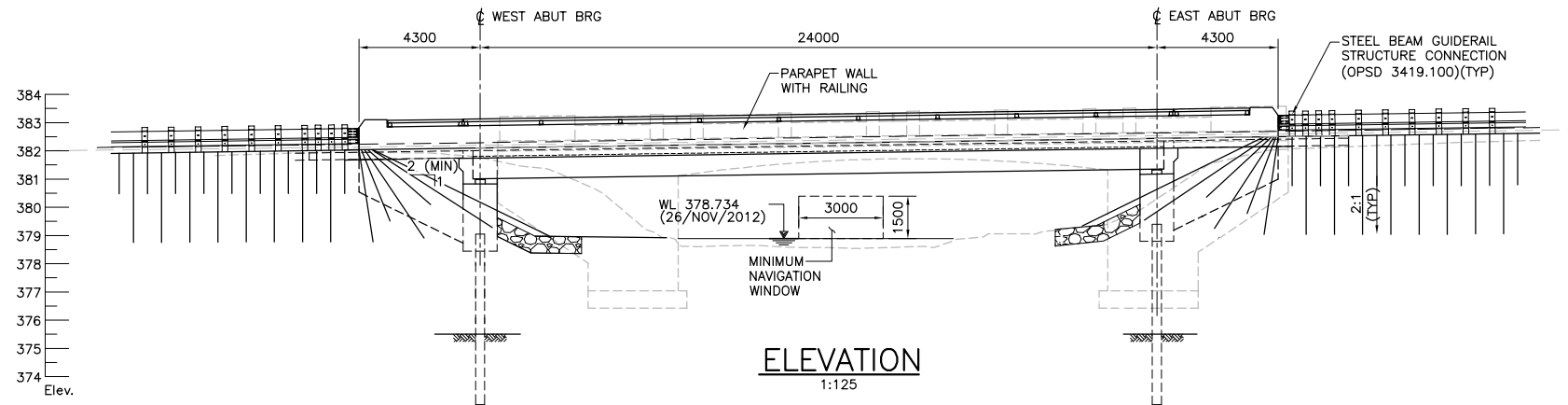
APPENDIX

G

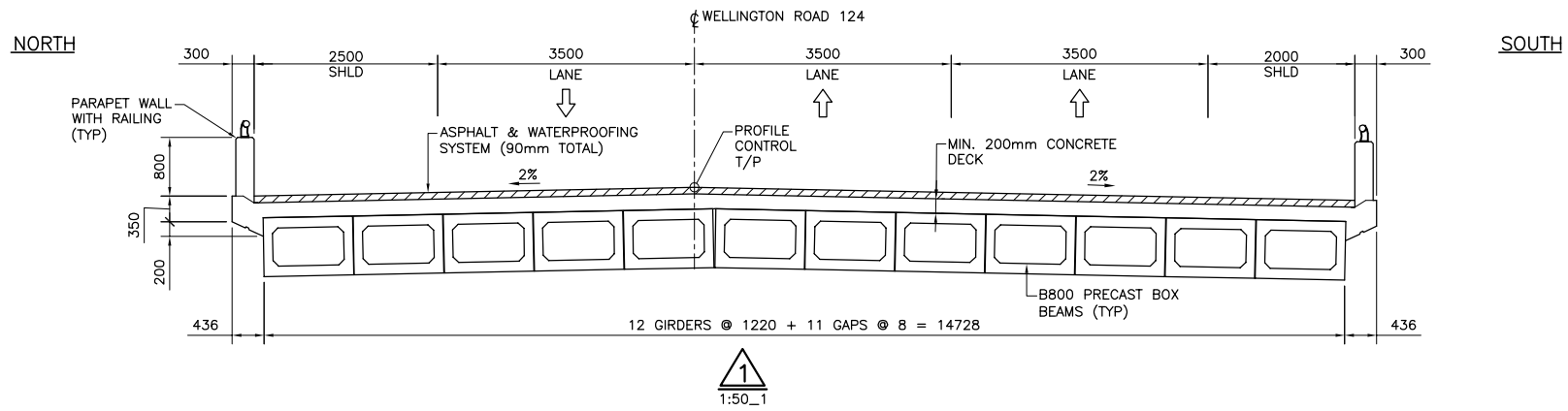
PRELIMINARY
GENERAL
ARRANGEMENT



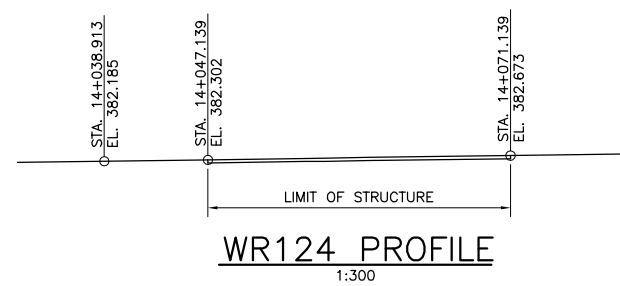
PLAN
1:125



ELEVATION
1:125



1:50_1



WR124 PROFILE
1:300

GENERAL NOTES:

CLASS OF CONCRETE

PRECAST GIRDERS 50 MPa
ALL CONCRETE UNLESS NOTED OTHERWISE 35 MPa

CLASS OF CONCRETE FOR PRECAST GIRDERS ARE GIVEN ON PRESTRESSED GIRDER DRAWINGS.

CLEAR COVER TO REINFORCING STEEL

BOTTOM OF ABUTMENTS 100 ± 25
REMAINDER 70 ± 20 UNLESS OTHERWISE NOTED

REINFORCING STEEL

1. REINFORCING STEEL SHALL BE GRADE 400W. UNLESS OTHERWISE NOTED.
2. BAR MARKS WITH PREFIX 'S' DENOTE STAINLESS STEEL.
3. UNLESS SHOWN OTHERWISE, TENSION LAP SPLICES FOR REINFORCING STEEL BARS SHALL BE CLASS B.
4. BAR HOOKS SHALL HAVE STANDARD HOOK DIMENSIONS USING MINIMUM BEND DIAMETERS, WHILE STIRRUPS AND TIES SHALL HAVE MINIMUM HOOK DIMENSIONS. ALL HOOKS SHALL BE IN ACCORDANCE WITH THE STRUCTURAL STANDARD DRAWINGS SS12-1 AND SS12-2, UNLESS INDICATED OTHERWISE.

REINFORCING GFRP

1. STAINLESS REINFORCING STEEL SHALL BE TYPE 316LN or DUPLEX 2205 AND HAVE A MINIMUM YIELD STRENGTH OF 500 MPa, UNLESS OTHERWISE SPECIFIED.
2. GLASS FIBRE REINFORCED POLYMER (GFRP) REINFORCING BARS SHALL BE GRADE III. DESIGNATED BAR DIAMETER SHALL BE AS SPECIFIED IN THE CONTRACT DRAWINGS AND SHALL HAVE A NOMINAL CROSS SECTIONAL AREA ACCORDING TO CAN/CSA S-807
3. GFRP SHALL BE IN ACCORDANCE WITH THE DESIGNATED SOURCES FOR MATERIALS LIST DSM #9.65.90

CONSTRUCTION NOTES

1. THE CONTRACTOR SHALL ESTABLISH THE BEARING SEAT ELEVATIONS BY DEDUCTING THE ACTUAL BEARING THICKNESSES FROM THE TOP OF BEARING ELEVATIONS. IF THE ACTUAL BEARING THICKNESSES ARE DIFFERENT FROM THOSE GIVEN IN THE DRAWINGS, THE CONTRACTOR SHALL ADJUST THE REINFORCING STEEL TO SUIT.
2. BACKFILL SHALL NOT BE PLACED BEHIND ABUTMENTS UNTIL THE DECK IS IN PLACE AND HAS REACHED 70% OF ITS DESIGN STRENGTH.
3. BACKFILL SHALL BE PLACED SIMULTANEOUSLY BEHIND BOTH ABUTMENTS KEEPING THE HEIGHT OF BACKFILL APPROXIMATELY THE SAME. AT NO TIME SHALL THE DIFFERENCE IN ELEVATION BE GREATER THAN 500mm.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR STABILITY OF STRUCTURE DURING CONSTRUCTION.
5. THE CONTRACTOR IS RESPONSIBLE FOR THE METHOD AND DESIGN OF THE DEWATERING SCHEME. SEE SPECIFICATIONS FOR DETAILS. DEWATERING SCHEME TO ACCOMMODATE 5 YEAR STORM EVENT (379.79m).
6. TURBIDITY CURTAIN TO BE PLACED WHERE ROCK PROTECTION WORK OCCURS BELOW WATER LEVEL.
7. THE CONTRACTOR SHALL DESIGN AND INSTALL A PERFORMANCE LEVEL 2 PROTECTION SYSTEM TO ACCOMMODATE THE HIGHWAY WIDENING. THE LIMITS OF THE PROTECTION SYSTEM SHALL BE DETERMINED BY THE CONTRACTOR TO SUIT THE WORK.

LIST OF DRAWINGS:

S1 GENERAL ARRANGEMENT

CAD FILE LOCATION AND NAME: s:\2013\32\3213095\300\13M-00225-01-300-001GA.dwg
 MODIFIED: 7/11/2018 9:02:20 AM BY: VILASENORD
 DATE PLOTTED: 7/11/2018 9:36:30 AM BY: VILASENORD

THESE DESIGN DOCUMENTS ARE PREPARED SOLELY FOR THE USE BY THE PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS ENTERED INTO A CONTRACT AND THERE ARE NO REPRESENTATIONS OF ANY KIND MADE BY THE DESIGN PROFESSIONAL TO ANY PARTY WITH WHOM THE DESIGN PROFESSIONAL HAS NOT ENTERED INTO A CONTRACT.

		SCALE		AS SHOWN	DESIGNED	WK								
		DATE	JUNE 2018	CHECKED										
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		FIELD BK.	PAGE											
		BENCH M			PROJECT									

COUNTY OF WELLINGTON
ERAMOSA RIVER BRIDGE
 STRUCTURE No. B124135
 GENERAL ARRANGEMENT

CW2019-XXX
 PROJECT NO.
 DRAWING NO.
S1

METRIC
 ALL DIMENSIONS SHOWN HERE ARE IN MILLIMETRES UNLESS OTHERWISE NOTED