

10 February 2022  
Project: 210351

Breymark Homes  
1315 Bishop Street North  
Cambridge, ON N1R 6Z2

## **RE: RESIDENTIAL DEVELOPMENT, MAIN STREET & LORNE STREET, PALMERSTON ON, TRANSPORTATION IMPACT BRIEF**

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Paradigm Transportation Solutions Limited (Paradigm) was retained to conduct this Transportation Impact Brief for a proposed residential subdivision located on the north side of Main Street and west of Lorne Street in the community of Palmerston, Wellington County. **Figure 1** illustrates the subject site location.

### **Purpose and Scope**

The purpose of this report is to identify and assess the potential traffic impact resulting from the subdivision. The scope, as reviewed with Wellington County staff via email in July 2021, includes:

- ▶ Assessment of the current traffic and site conditions within the study area;
- ▶ Estimates of background traffic growth for five years from the study date (2026);
- ▶ Analyses of the impact of future traffic on the surrounding road network, including the study area intersection of Main Street (Wellington Road 123) and Ontario Street; and
- ▶ Recommendations necessary to mitigate the future traffic in a satisfactory manner, if necessary.

### **Existing Conditions**

#### **Road Characteristics**

The main roadways near the subject site considered in assessing the traffic impacts of the subdivision include:

- ▶ **Main Street (Wellington Road 123)** is an east-west County collector road with a posted speed limit of 50 km/h within the study area. A sidewalk is provided along the southern side of the road throughout the study area. No visible cycling infrastructure is provided along Main Street.

- ▶ **Ontario Street** is a north-south laneway with no posted speed limit; therefore, it is assumed the statutory speed limit of 50 km/h governs. Neither sidewalks nor cycling facilities are provided on either side of the road.

**Figure 2** details the existing traffic control and lane configurations at the study area intersections.

### Traffic Volumes

Paradigm conducted an eight-hour turning movement count at the intersection of Main Street and Ontario Street on Wednesday, 7 July 2021.

**Figure 3** displays the existing traffic volumes for the AM and PM peak hours.

**Appendix A** contains the detailed turning movement counts for the study area intersections.

### Traffic Operations

Intersection level of service (LOS) is a recognized method of quantifying the average delay experienced by drivers at intersections. It is based on the delay experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles intending to make a particular movement, compared to the estimated capacity for that movement. The capacity is based on a number of criteria related to the opposing traffic flows and intersection geometry.

The highest possible rating is LOS A, under which the average total delay is equal to or less than 10.0 seconds per vehicle. When the average delay exceeds 80 seconds for a signalized intersection, 50 seconds for an unsignalized intersection or when the volume to capacity (v/c) ratio is greater than 1.00, the movement is classified as LOS F and remedial measures are usually implemented if they are feasible. LOS E is usually used as a guideline for the determination of road improvement needs on through lanes, while LOS F may be acceptable for left-turn movements at peak times, depending on delays.

The operations of the study area intersections were evaluated using the existing lane configuration, traffic controls, and the existing traffic AM and PM peak hour volumes. The LOS conditions on the existing road network have been assessed using Synchro 10.

**Table 1** summarizes the existing intersection operations with the entries in the table indicating the AM and PM peak hour LOS, v/c ratios, and 95<sup>th</sup> percentile queues experienced.

The study area intersections are currently operating with acceptable levels of service and within capacity. **Appendix B** contains the detailed Synchro 10 reports.



**TABLE 1: EXISTING TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall				
				Eastbound				Westbound				Northbound				Southbound								
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach					
AM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay	<	A	>	A	<	A	>	A	<	A	>	A	<	A	>	A	<	A	>	A	9
			V/C	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	9
			Q	<	0	>	0	<	0	>	0	<	0	>	0	<	0	>	0	<	0	>	0	0
PM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay	<	A	>	A	<	A	>	A	<	A	>	A	<	A	>	A	<	A	>	A	0
			V/C	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	<	0.00	>	0	0
			Q	<	0	>	0	<	0	>	0	<	0	>	0	<	0	>	0	<	0	>	0	0

MOE - Measure of Effectiveness

LOS - Level of Service

Delay - Average Delay per Vehicle in Seconds

V/C - Volume to Capacity Ratio

Q - 95th Percentile Queue Length (m)

TWSC - Two-Way Stop Control

</> - Shared with through movement

## Development Concept

### Development Description

The subject site is located on the north side of Main Street, west of Lorne Street in Palmerston, Ontario. The property owner is proposing to construct 71 townhouse units.

Vehicle access is proposed via Ontario Street. An additional access will be provided to Mary Street for emergency vehicles only.

**Figure 4** illustrates the proposed draft plan of subdivision.

### Development Trip Generation

The Institute of Transportation Engineers (ITE) trip generation rates were used to estimate the weekday AM and PM peak hour trip generation of the proposed development. The fitted curve equations of Land Use Code (LUC) 220, Multifamily Housing (Low-Rise), have been used.

**Table 2** summarizes the forecast number of trips generated by the proposed development. As shown, the site is expected to generate approximately 34 trips during the AM peak hour and 44 trips during the PM peak hour.

**TABLE 2: TRIP GENERATION**

Land Use	Units	AM Peak Hour				PM Peak Hour			
		Rate	In	Out	Total	Rate	In	Out	Total
Multifamily Housing (Low-Rise) - LUC 220	71	Eq <sup>1</sup>	8	26	34	Eq <sup>2</sup>	28	16	44
<b>Total Trip Generation</b>			<b>8</b>	<b>26</b>	<b>34</b>		<b>28</b>	<b>16</b>	<b>44</b>

$$^1\text{Ln}(T) = 0.95 \text{Ln}(X) - 0.51$$

$$^2\text{Ln}(T) = 0.89 \text{Ln}(X) - 0.02$$



## Development Trip Distribution and Assignment

The site-generated trips were assigned to the road network using the existing trip distribution and most logical routing to and from the site. The site trips are expected to follow similar routing to the existing distribution within the study area due to the similar land uses surrounding the subject site.

**Table 3** summarizes the estimated trip distribution for the development.

**Figure 5** illustrates the site-generated trip assignments for the weekday AM and PM peak hour trips.

**TABLE 3: TRIP DISTRIBUTION**

Distribution	AM Peak Hour		PM Peak Hour	
	Inbound	Outbound	Inbound	Outbound
West via Main Street	46%	52%	48%	49%
East via Main Street	52%	46%	49%	49%
<b>Total</b>	<b>100%</b>	<b>100%</b>	<b>100%</b>	<b>100%</b>

## Future Traffic Conditions

The assessment of future traffic conditions includes background and total traffic five years after the date of this study (2026), as agreed upon with County staff during pre-study consultation.

### Background Traffic Volumes

The 2026 future traffic volumes reflect a compounded growth rate of 2.0% per annum applied to the existing through volumes on Main Street. The growth rate was not applied to movements along, to, or from Ontario Street because the road terminates approximately 40 metres north of Main Street, and its volumes are not expected to significantly change within the horizon year.

No other background developments were provided or included in the background traffic volumes. It is assumed that the generalized growth rate would capture the traffic generated by any other development constructed within the horizon year.

**Figure 6** illustrates the 2026 forecast background traffic volumes for the weekday AM and PM peak hours.

### Background Traffic Operations

Based on the estimated background traffic volumes, level of service analyses have been conducted using Synchro 10 with HCM procedures for the weekday AM and PM peak hour conditions at the study area intersections. The same parameters as in the analysis of the base year conditions were used.



**Table 4** summarizes the results of the 2026 background traffic operations. Similar to base year traffic operations, the results indicate that all study area intersections are forecast to operate at acceptable levels of service and within capacity.

**Appendix C** contains the supporting detailed Synchro 10 reports.

**TABLE 4: BACKGROUND TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 9 0.00 0	> > > >	A 9 > >	
PM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 V/C - Volume to Capacity Ratio  
 Q - 95th Percentile Queue Length (m)  
 TWSC - Two-Way Stop Control  
 </> - Shared with through movement

**Total Traffic Volumes**

Total traffic volumes include the background forecasts added to the site-generated forecasts. **Figure 7** illustrates the weekday AM and PM peak hour forecast total traffic volumes for the 2026 horizon.

**Total Traffic Operations**

Based on the estimated total traffic volumes, level of service analyses have been conducted using Synchro 10 with HCM procedures for the weekday AM and PM peak hour conditions at the study area intersections. The same parameters as in the analysis of the base year and background conditions were used.

**Table 5** summarizes the results of the 2026 total horizon operations. Similar to background traffic operations, the results indicate that all study area intersections are forecast to operate at acceptable levels of service and within capacity.

**Appendix D** contains the supporting detailed Synchro 10 reports.

**TABLE 5: TOTAL TRAFFIC OPERATIONS**

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay V/C Q	< < < <	A 8 0.00 0	> > > >	A 0 > 0	< < < <	A 0 0.00 0	> > > >	A 0 > 0	< < < <	A 0 0.00 0	> > > >	< < < <	B 11 0.04 1	> > > >	B 11 > >		
PM Peak Hour	Private Driveway/Ontario Street & Main Street West	TWSC	LOS Delay V/C Q	< < < <	A 8 0.01 0	> > > >	A 0 > 0	< < < <	A 0 0.00 0	> > > >	A 0 > 0	< < < <	A 0 0.00 0	> > > >	< < < <	B 12 0.03 1	> > > >	B 12 > >		

MOE - Measure of Effectiveness  
 LOS - Level of Service  
 Delay - Average Delay per Vehicle in Seconds  
 V/C - Volume to Capacity Ratio  
 Q - 95th Percentile Queue Length (m)  
 TWSC - Two-Way Stop Control  
 </> - Shared with through movement

**Auxiliary Left-Turn Lanes**

The intersection of Main Street and Ontario Street was assessed to determine if the forecast future traffic volumes warrant installation of an eastbound left-turn lane. The warrants for left-turn lanes follow the requirements in the Ministry of Transportation of Ontario’s (MTO) Design Supplement for Transportation Association of Canada’s (TAC) Geometric Design Guide for Canada Roads<sup>1</sup> (TAC-GDGCR). A design speed of 60 km/h (10 km/h over the posted speed limit) was used for the analysis.

The percentages of left-turning vehicles in the approaching volume were rounded to the nearest 5%, as nomographs are provided in 5% increments. This apparent requirement is due to the nature of the warrant procedure that assumes a minimum of 5% of left turning vehicles in the advancing volume.

**Table 6** summarizes the left-turn lane warrants for the intersection of Main Street and Ontario Street. The warrant analysis suggests that an eastbound left-turn lane is not warranted under 2026 total traffic conditions.

<sup>1</sup> Ministry of Transportation, Ontario, Design Supplement for TAC Geometric Design Guide for Canadian Roads, June 2017.

**TABLE 6: LEFT-TURN LANE WARRANT SUMMARY**

Roadway	Eastbound at Main Street and Ontario Street	
Design Speed	60 km/h	
Peak Hour	AM	PM
Advancing Volume	177	275
Opposing Volume	202	277
Left Turning Volume	4	14
% of Left Turning Traffic	2.3%	5.1%
Warranted	No	No
Storage Length	-	-

Based on MTO Design Supplement for TAC-GDGCR |

## Traffic Signal Warrant

The requirement for traffic signal control at the intersection of Main Street and Ontario Street was assessed using the Ontario Traffic Manual (OTM) signal warrant guidelines<sup>2</sup>.

Based on the warrant analysis, traffic signal control is not warranted under the 2026 forecast total traffic conditions at Main Street and Ontario Street.

**Appendix E** contains the warrant analysis worksheets.

## Conclusions & Recommendations

Based on the above analysis, the following is concluded:

- ▶ The subject site is forecast to generate 34 AM peak hour trips and 44 PM peak hour trips;
- ▶ The existing, forecast background, and forecast total traffic conditions show good operations with no problem movements;
- ▶ The addition of the site-generated traffic volumes does not result in the need for an eastbound left-turn storage lane on Main Street at Ontario Street; and
- ▶ No changes to the existing lane geometrics are recommended at this time.

<sup>2</sup> Ontario Traffic Manual Book 12: Traffic Signals, March 2012.



We trust that this letter sufficiently outlines the impacts of the proposed development and its impacts on the surrounding road network.

Yours very truly,

**PARADIGM TRANSPORTATION SOLUTIONS LIMITED**



**Erica Bayley**  
P.Eng.  
Senior Project Manager







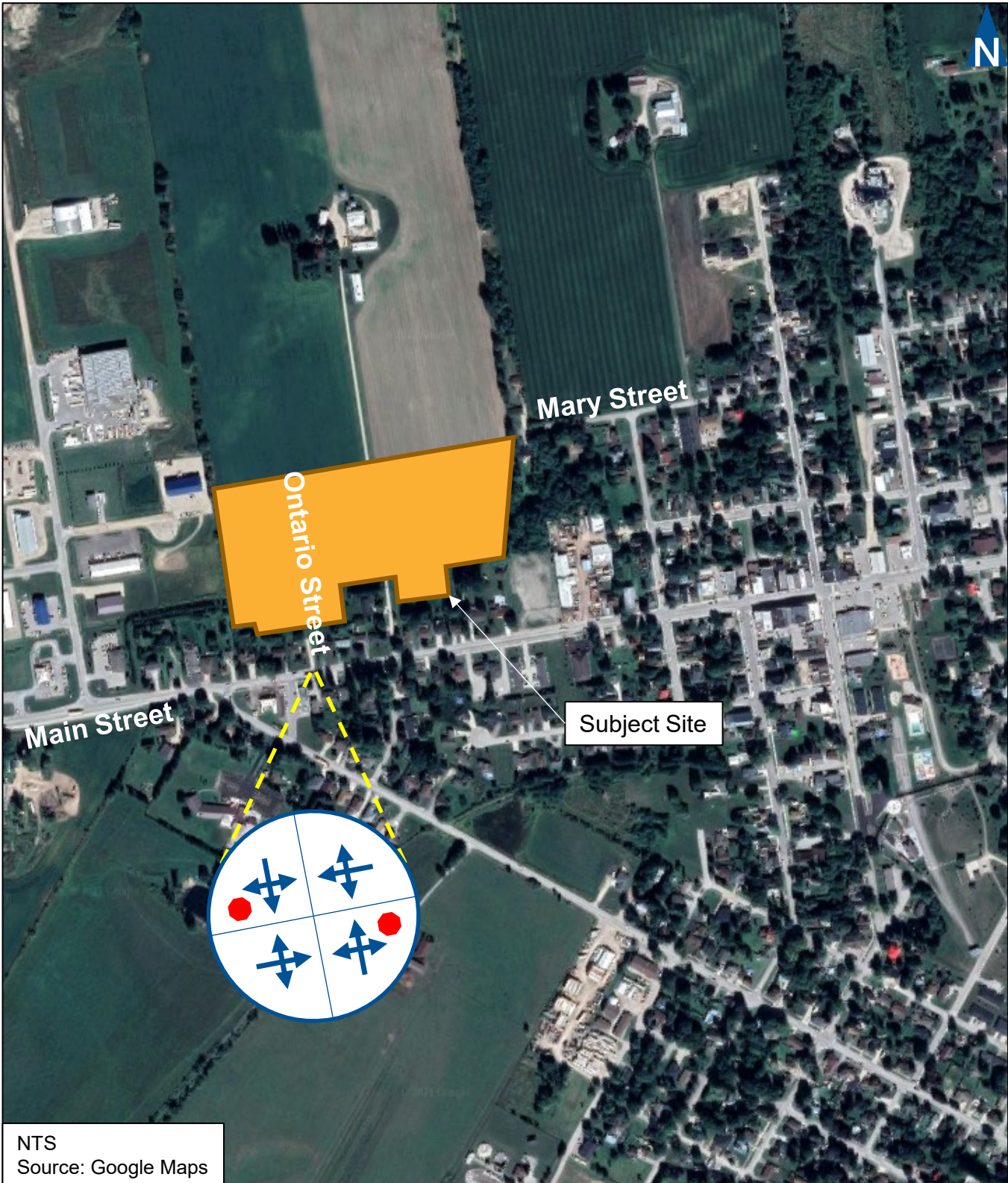
NTS  
Source: Google Maps



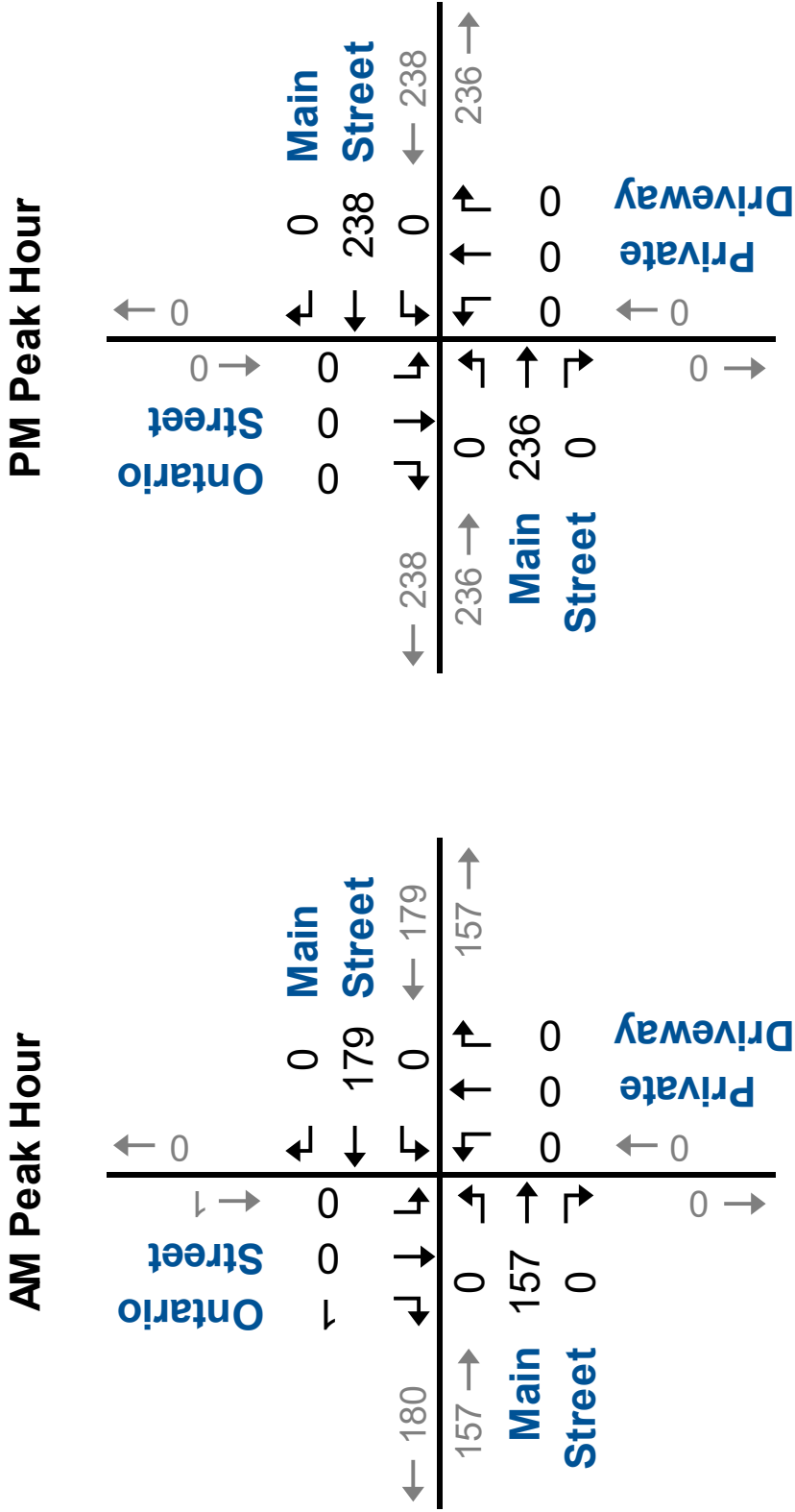
# Subject Site Location

Figure 1



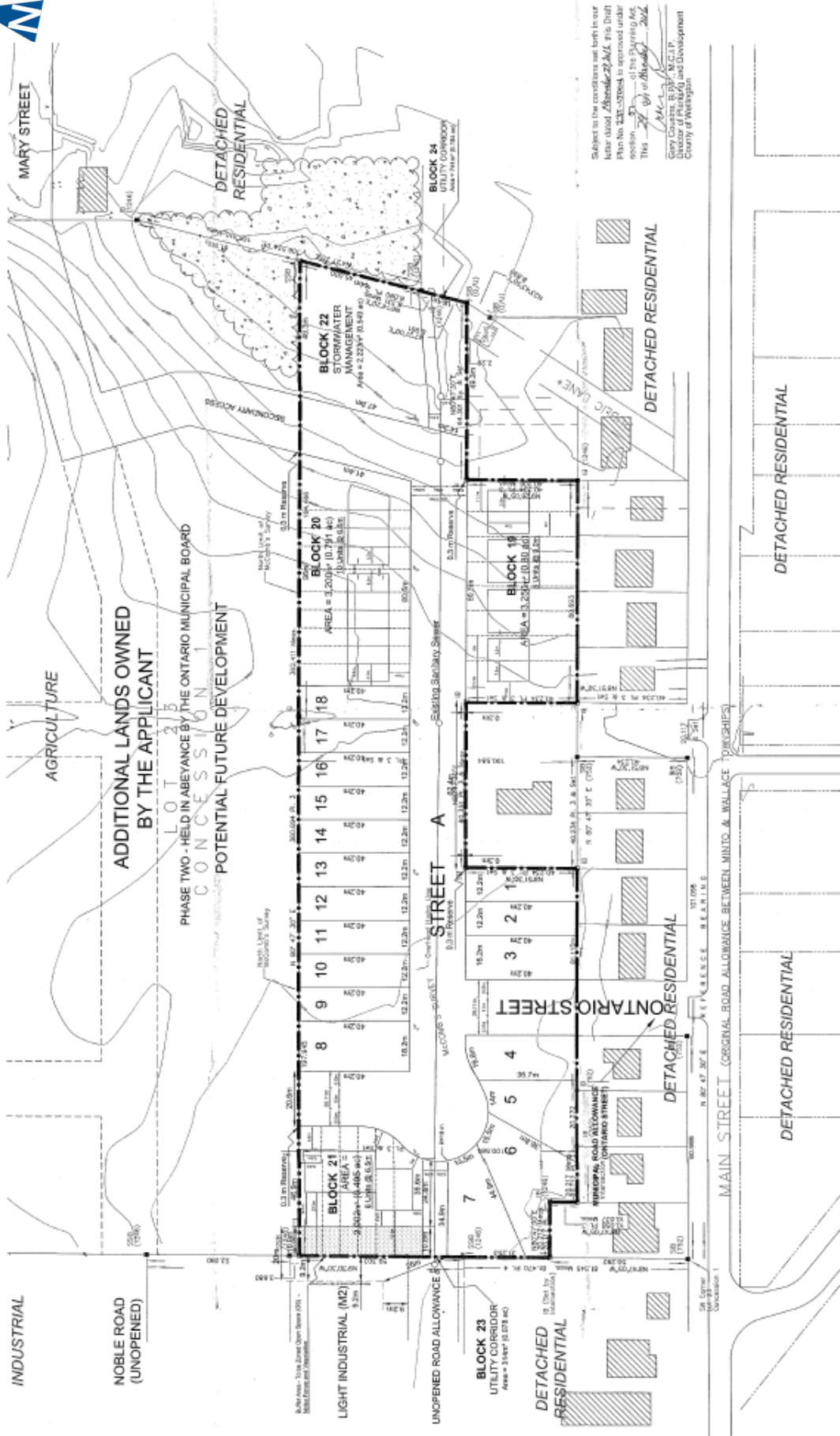


# Existing Lane Configuration and Traffic Control



Existing Traffic Volumes

Figure 3



Subject to the conditions set forth in our later dated Zoning By-Law, this Draft Plan No. 210351 is approved under section 33 of the Planning Act. This is a Plan of Subdivision.

AK  
 Gary Coates, E.P.S. - M.C.I.P.  
 Municipal Engineer and Development  
 County of Wellington



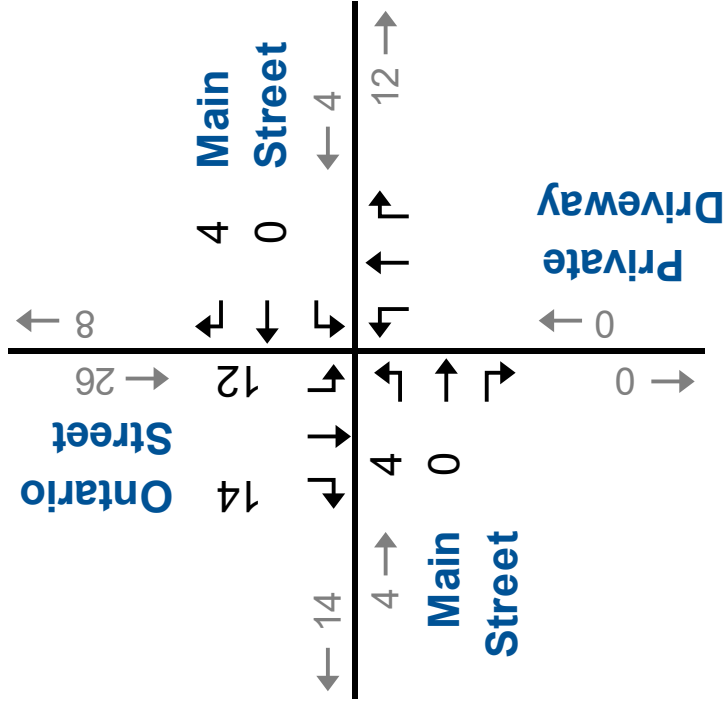
# Draft Plan of Subdivision

Figure 4

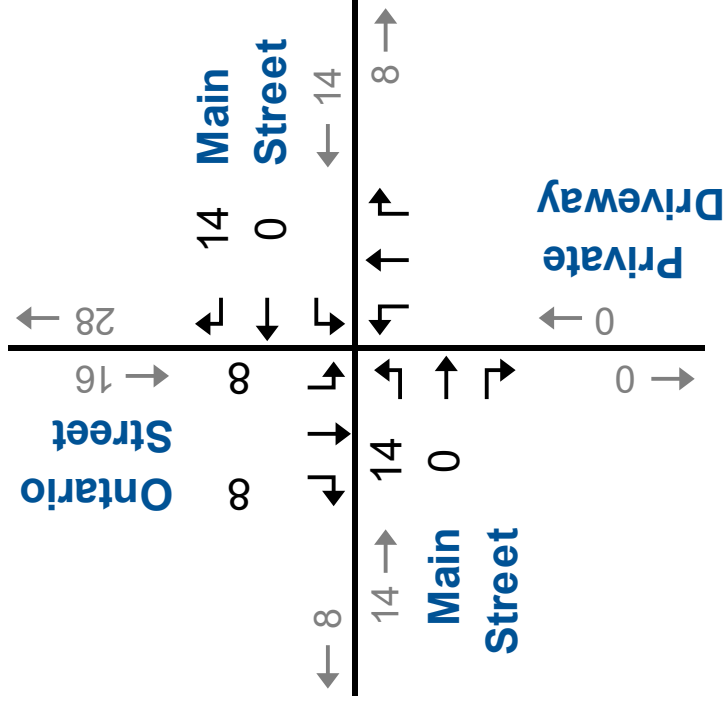




### AM Peak Hour

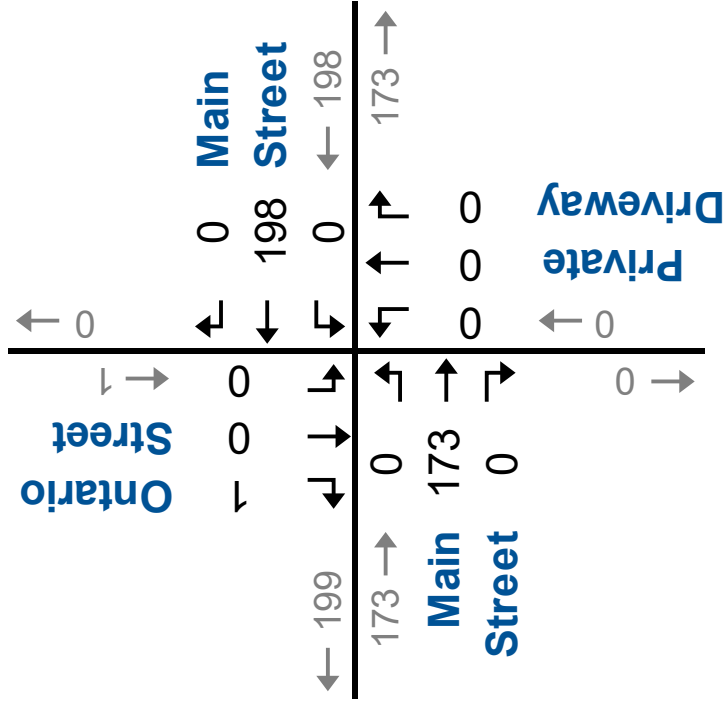


### PM Peak Hour

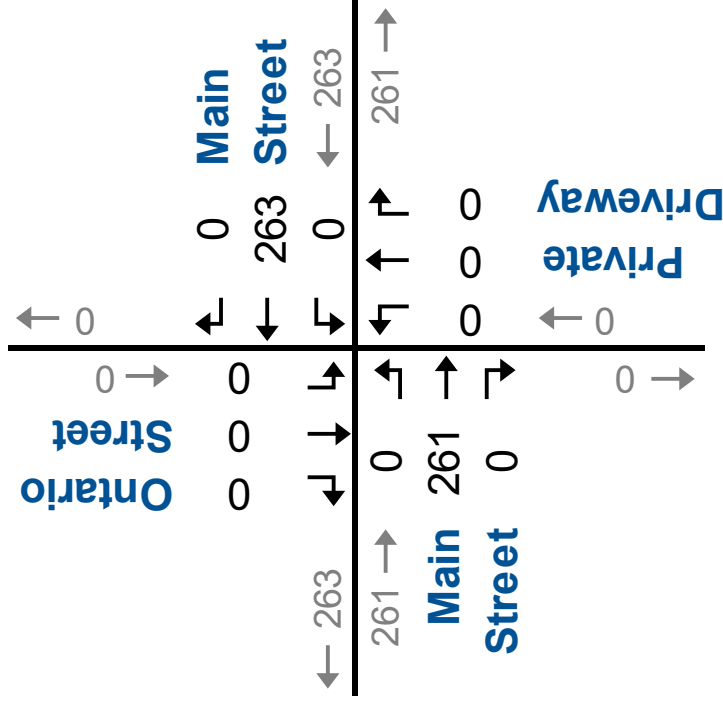




### AM Peak Hour

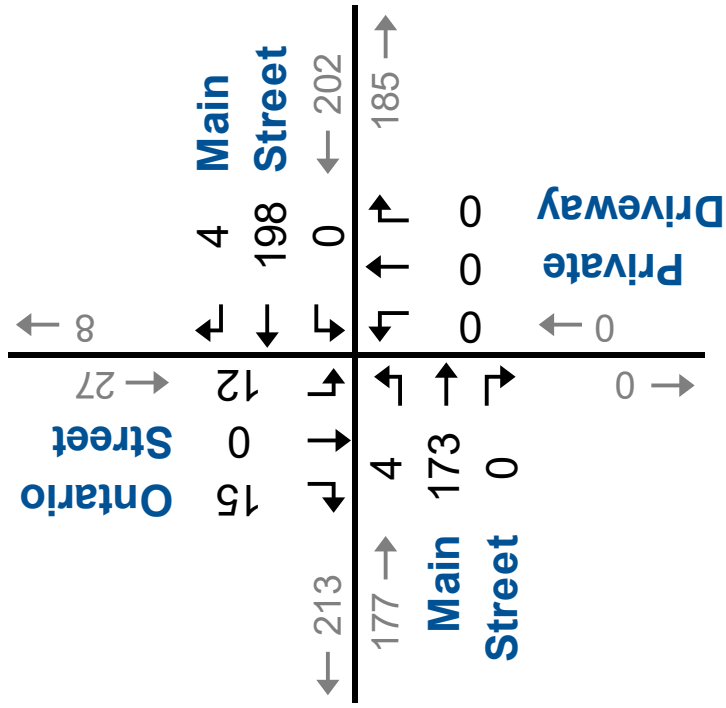


### PM Peak Hour

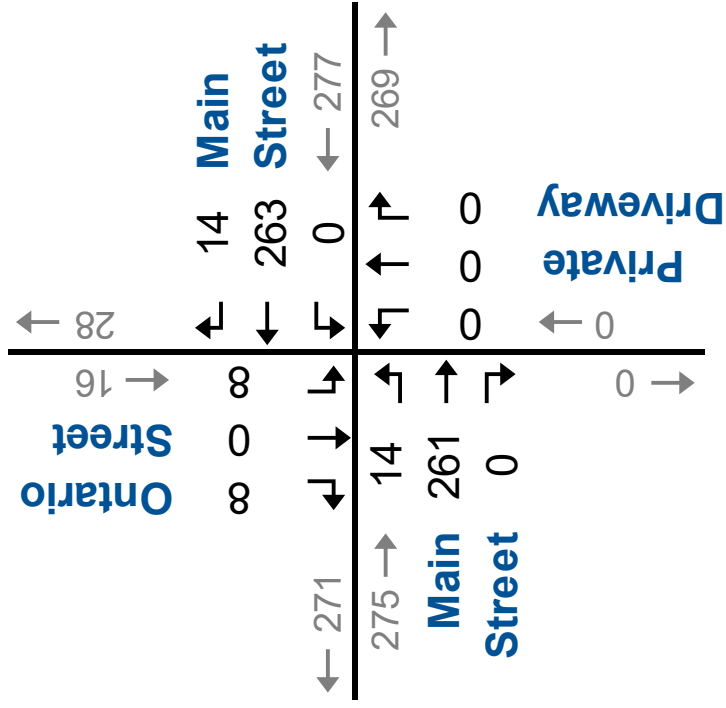




### AM Peak Hour



### PM Peak Hour



## 2026 Total Traffic Volumes

### Figure 7

## Appendix A

### Existing Traffic Volumes







5:45 PM	0	46	0	0	0	46	0	56	0	0	0	56	0	0	0	0	0	0	0	0	0	0	102		
Hourly Total	0	226	0	0	0	226	0	231	0	0	0	231	0	0	0	0	0	0	0	0	0	0	457		
6:00 PM	0	42	0	0	4	42	0	44	0	0	0	44	0	0	0	0	0	0	0	0	0	0	86		
6:15 PM	0	30	0	0	0	30	0	35	0	0	0	35	0	0	0	0	0	0	0	0	0	0	65		
6:30 PM	0	34	0	0	0	34	0	37	0	0	0	37	0	0	0	0	0	0	0	0	0	0	71		
6:45 PM	0	30	0	0	0	30	0	32	0	0	0	32	0	0	0	0	0	0	0	0	0	0	62		
Hourly Total	0	136	0	0	4	136	0	148	0	0	0	148	0	0	0	0	0	0	0	0	0	0	284		
Grand Total	0	1496	0	0	4	1496	0	1543	1	0	0	1544	0	0	0	0	0	0	1	0	1	0	2	2	3042
Approach %	0.0	100.0	0.0	0.0	-	-	0.0	99.9	0.1	0.0	-	-	0.0	0.0	0.0	0.0	-	-	50.0	0.0	50.0	0.0	-	-	-
Total %	0.0	49.2	0.0	0.0	-	49.2	0.0	50.7	0.0	0.0	-	50.8	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.1	-	-
Motorcycles	0	2	0	0	-	2	0	5	0	0	-	5	0	0	0	0	-	0	0	0	0	-	0	0	7
% Motorcycles	-	0.1	-	-	-	0.1	-	0.3	0.0	-	-	0.3	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.2
Cars & Light Goods	0	1381	0	0	-	1381	0	1401	1	0	-	1402	0	0	0	0	-	0	1	0	1	0	-	2	2785
% Cars & Light Goods	-	92.3	-	-	-	92.3	-	90.8	100.0	-	-	90.8	-	-	-	-	-	-	100.0	-	100.0	-	-	100.0	91.6
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	-	-	-	0.0	-	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	57	0	0	-	57	0	54	0	0	-	54	0	0	0	0	-	0	0	0	0	0	-	0	111
% Single-Unit Trucks	-	3.8	-	-	-	3.8	-	3.5	0.0	-	-	3.5	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	3.6
Articulated Trucks	0	55	0	0	-	55	0	83	0	0	-	83	0	0	0	0	-	0	0	0	0	0	-	0	138
% Articulated Trucks	-	3.7	-	-	-	3.7	-	5.4	0.0	-	-	5.4	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	4.5
Bicycles on Road	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Bicycles on Road	-	0.1	-	-	-	0.1	-	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	4	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-





Paradigm Transportation Solutions Limited  
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8  
519-896-3163 cbowness@ptsI.com

Count Name: Main Street & Ontario Street  
Site Code: 210351  
Start Date: 07/07/2021  
Page No: 4

### Turning Movement Peak Hour Data (7:30 AM)

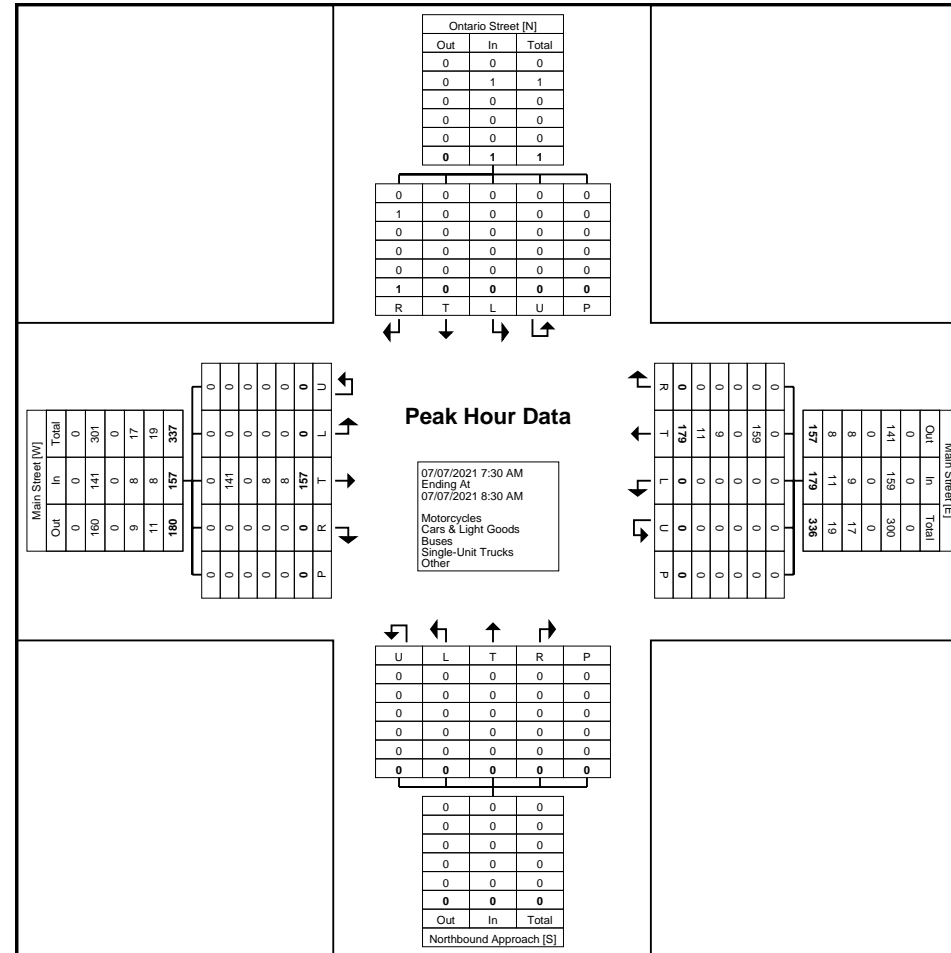
Start Time	Main Street Eastbound						Main Street Westbound						Northbound Approach Northbound						Ontario Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:30 AM	0	51	0	0	0	51	0	40	0	0	0	40	0	0	0	0	0	0	0	0	1	0	0	1	92
7:45 AM	0	39	0	0	0	39	0	40	0	0	0	40	0	0	0	0	0	0	0	0	0	0	0	0	79
8:00 AM	0	30	0	0	0	30	0	47	0	0	0	47	0	0	0	0	0	0	0	0	0	0	0	0	77
8:15 AM	0	37	0	0	0	37	0	52	0	0	0	52	0	0	0	0	0	0	0	0	0	0	0	0	89
<b>Total</b>	0	157	0	0	0	157	0	179	0	0	0	179	0	0	0	0	0	0	0	0	1	0	0	1	337
Approach %	0.0	100.0	0.0	0.0	-	-	0.0	100.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	100.0	0.0	-	-	-
Total %	0.0	46.6	0.0	0.0	-	46.6	0.0	53.1	0.0	0.0	-	53.1	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.3	0.0	-	0.3	-
PHF	0.000	0.770	0.000	0.000	-	0.770	0.000	0.861	0.000	0.000	-	0.861	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.250	0.000	-	0.250	0.916
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	0.0	-	-	0.0	0.0
Cars & Light Goods	0	141	0	0	-	141	0	159	0	0	-	159	0	0	0	0	-	0	0	0	1	0	-	1	301
% Cars & Light Goods	-	89.8	-	-	-	89.8	-	88.8	-	-	-	88.8	-	-	-	-	-	-	-	-	100.0	-	-	100.0	89.3
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	8	0	0	-	8	0	9	0	0	-	9	0	0	0	0	-	0	0	0	0	0	-	0	17
% Single-Unit Trucks	-	5.1	-	-	-	5.1	-	5.0	-	-	-	5.0	-	-	-	-	-	-	-	-	0.0	-	-	0.0	5.0
Articulated Trucks	0	8	0	0	-	8	0	11	0	0	-	11	0	0	0	0	-	0	0	0	0	0	-	0	19
% Articulated Trucks	-	5.1	-	-	-	5.1	-	6.1	-	-	-	6.1	-	-	-	-	-	-	-	-	0.0	-	-	0.0	5.6
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Paradigm Transportation Solutions Limited  
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8  
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Count Name: Main Street & Ontario Street  
Site Code: 210351  
Start Date: 07/07/2021  
Page No: 5



Turning Movement Peak Hour Data Plot (7:30 AM)

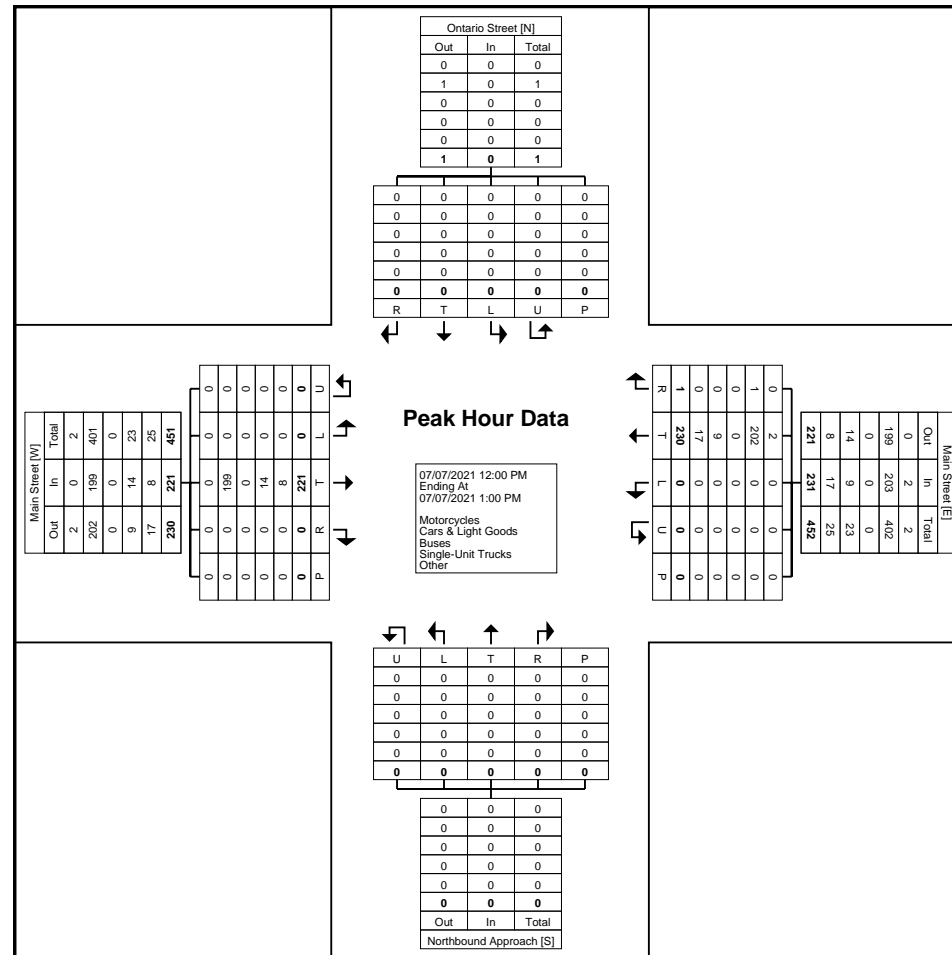




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Turning Movement Peak Hour Data Plot (12:00 PM)



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Count Name: Main Street & Ontario Street  
Site Code: 210351  
Start Date: 07/07/2021  
Page No: 8

### Turning Movement Peak Hour Data (4:00 PM)

Start Time	Main Street Eastbound						Main Street Westbound						Northbound Approach Northbound						Ontario Street Southbound						Int. Total	
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total		
4:00 PM	0	77	0	0	0	77	0	62	0	0	0	62	0	0	0	0	0	0	0	0	0	0	0	0	0	139
4:15 PM	0	65	0	0	0	65	0	63	0	0	0	63	0	0	0	0	0	0	0	0	0	0	0	0	0	128
4:30 PM	0	43	0	0	0	43	0	63	0	0	0	63	0	0	0	0	0	0	0	0	0	0	1	0	0	106
4:45 PM	0	51	0	0	0	51	0	50	0	0	0	50	0	0	0	0	0	0	0	0	0	0	0	0	0	101
Total	0	236	0	0	0	236	0	238	0	0	0	238	0	0	0	0	0	0	0	0	0	0	1	0	0	474
Approach %	0.0	100.0	0.0	0.0	-	-	0.0	100.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-	-
Total %	0.0	49.8	0.0	0.0	-	49.8	0.0	50.2	0.0	0.0	-	50.2	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	-
PHF	0.000	0.766	0.000	0.000	-	0.766	0.000	0.944	0.000	0.000	-	0.944	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000	0.853
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0
% Motorcycles	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Cars & Light Goods	0	226	0	0	-	226	0	222	0	0	-	222	0	0	0	0	-	0	0	0	0	0	-	0	0	448
% Cars & Light Goods	-	95.8	-	-	-	95.8	-	93.3	-	-	-	93.3	-	-	-	-	-	-	-	-	-	-	-	-	-	94.5
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0
% Buses	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Single-Unit Trucks	0	7	0	0	-	7	0	9	0	0	-	9	0	0	0	0	-	0	0	0	0	0	-	0	0	16
% Single-Unit Trucks	-	3.0	-	-	-	3.0	-	3.8	-	-	-	3.8	-	-	-	-	-	-	-	-	-	-	-	-	-	3.4
Articulated Trucks	0	3	0	0	-	3	0	7	0	0	-	7	0	0	0	0	-	0	0	0	0	0	-	0	0	10
% Articulated Trucks	-	1.3	-	-	-	1.3	-	2.9	-	-	-	2.9	-	-	-	-	-	-	-	-	-	-	-	-	-	2.1
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0
% Bicycles on Road	-	0.0	-	-	-	0.0	-	0.0	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-

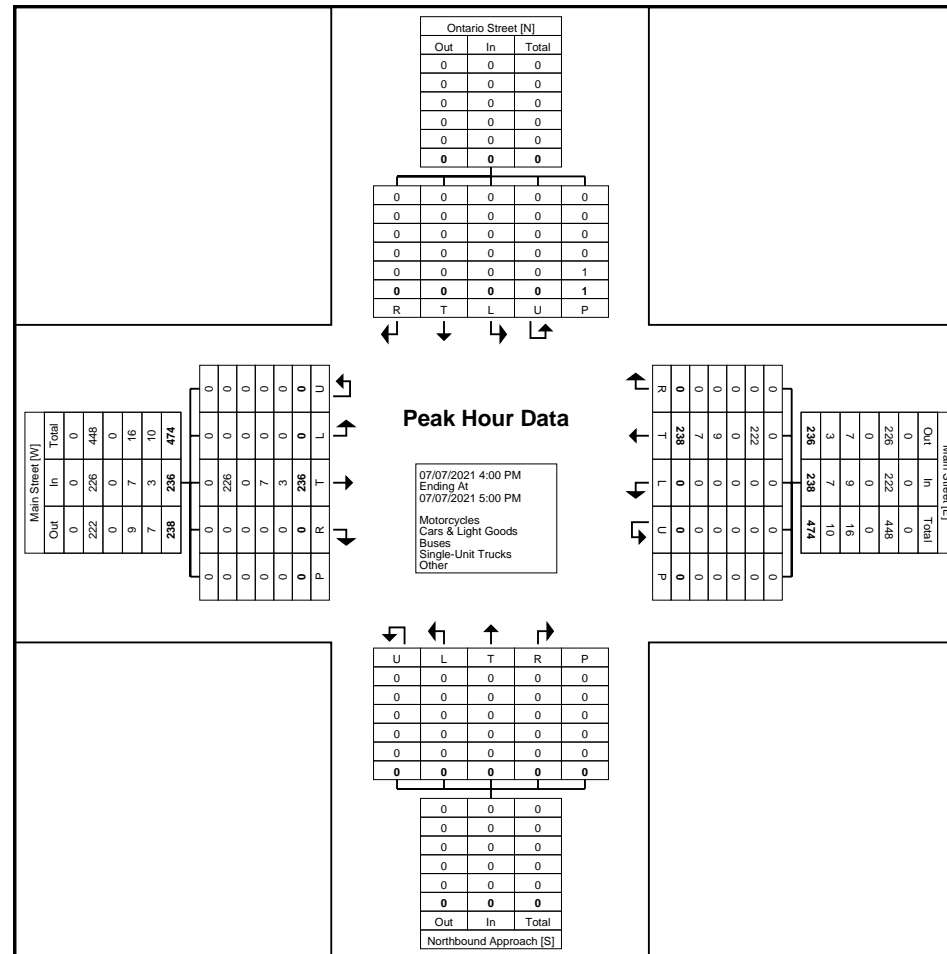




Paradigm Transportation Solutions Limited  
5A-150 Pinebush Rd

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Count Name: Main Street & Ontario Street  
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Start Date: 07/07/2021  
Page No: 9



Turning Movement Peak Hour Data Plot (4:00 PM)



Paradigm Transportation Solutions Limited  
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8  
519-896-3163 cbowness@ptsl.com

Count Name: Mary Street & Jane Street  
Site Code: 210351  
Start Date: 07/07/2021  
Page No: 1

### Turning Movement Data

Start Time	Mary Street Eastbound					Jane Street Northbound					Jane Street Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	0	0	0	0	0	0	1	0	0	1	0	1	0	0	1	2
7:15 AM	1	1	0	0	2	0	2	0	0	2	2	0	0	0	2	6
7:30 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	4
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	1	1	0	0	2	0	3	0	0	3	6	1	0	0	7	12
8:00 AM	0	0	0	0	0	0	1	0	0	1	1	1	0	0	2	3
8:15 AM	0	0	0	0	0	0	1	0	0	1	2	1	0	0	3	4
8:30 AM	1	1	0	0	2	1	1	0	0	2	1	0	0	0	1	5
8:45 AM	0	1	0	0	1	0	1	0	0	1	0	1	0	0	1	3
Hourly Total	1	2	0	0	3	1	4	0	0	5	4	3	0	0	7	15
9:00 AM	0	1	0	0	1	1	1	0	0	2	1	1	0	0	2	5
9:15 AM	0	0	0	1	0	0	3	0	0	3	2	0	0	0	2	5
9:30 AM	0	0	0	0	0	0	2	0	1	2	2	0	0	2	2	4
9:45 AM	0	0	0	0	0	1	1	0	0	2	1	0	0	0	1	3
Hourly Total	0	1	0	1	1	2	7	0	1	9	6	1	0	2	7	17
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11:00 AM	0	1	0	0	1	0	3	0	0	3	2	0	0	0	2	6
11:15 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	2
11:30 AM	0	1	0	1	1	1	2	0	0	3	1	0	0	0	1	5
11:45 AM	0	1	0	0	1	3	2	0	0	5	1	0	0	0	1	7
Hourly Total	0	3	0	1	3	4	7	0	0	11	6	0	0	0	6	20
12:00 PM	1	1	0	1	2	0	1	0	0	1	2	0	0	0	2	5
12:15 PM	0	0	0	0	0	2	4	0	0	6	4	1	0	0	5	11
12:30 PM	0	0	0	0	0	1	3	0	0	4	4	0	0	0	4	8
12:45 PM	1	0	0	0	1	1	7	0	0	8	1	0	0	0	1	10
Hourly Total	2	1	0	1	3	4	15	0	0	19	11	1	0	0	12	34
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	0	0	0	0	0	2	0	0	2	1	0	0	0	1	3
4:15 PM	1	1	0	0	2	0	4	0	0	4	1	0	0	0	1	7
4:30 PM	0	1	0	0	1	3	0	0	0	3	2	0	0	0	2	6
4:45 PM	0	1	0	0	1	0	1	0	0	1	1	0	0	0	1	3
Hourly Total	1	3	0	0	4	3	7	0	0	10	5	0	0	0	5	19
5:00 PM	0	0	0	0	0	1	2	0	0	3	0	0	0	0	0	3
5:15 PM	0	1	0	0	1	2	5	0	0	7	2	1	0	0	3	11
5:30 PM	0	0	0	0	0	2	3	0	0	5	2	0	0	0	2	7
5:45 PM	0	0	0	0	0	0	1	0	0	1	3	0	0	0	3	4

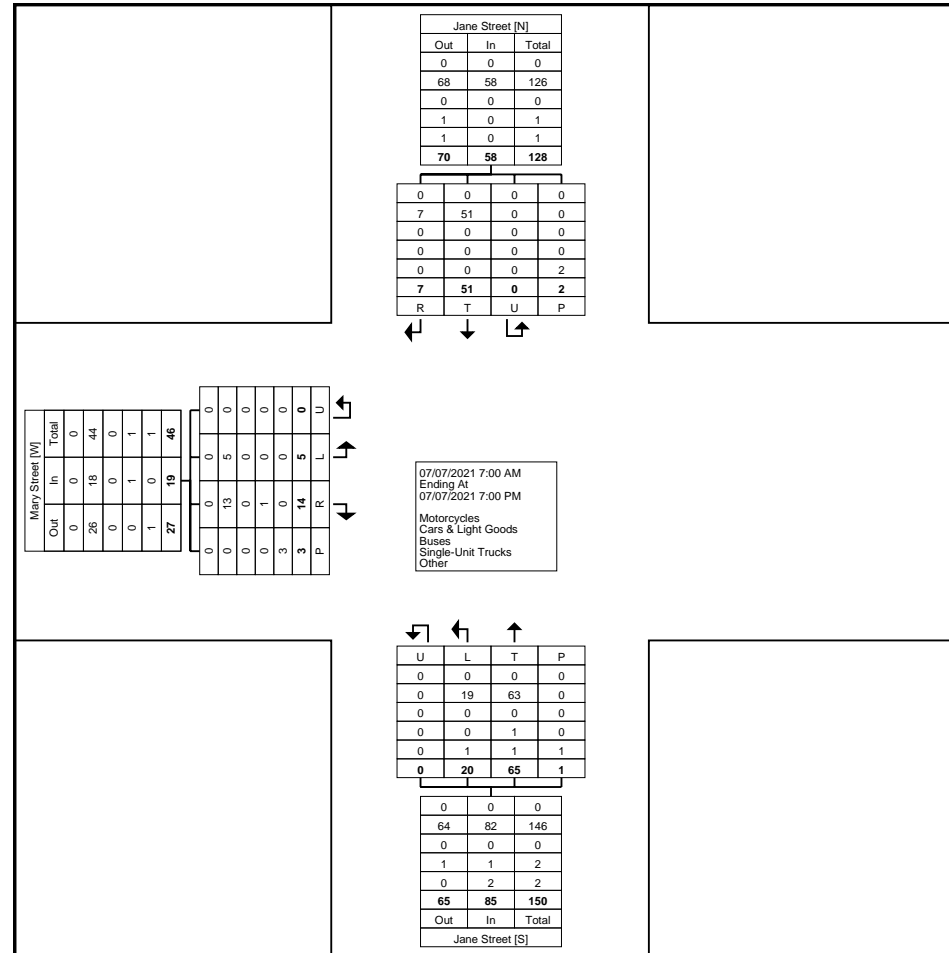
Hourly Total	0	1	0	0	1	5	11	0	0	16	7	1	0	0	8	25
6:00 PM	0	1	0	0	1	0	2	0	0	2	0	0	0	0	0	3
6:15 PM	0	1	0	0	1	0	1	0	0	1	4	0	0	0	4	6
6:30 PM	0	0	0	0	0	1	5	0	0	6	1	0	0	0	1	7
6:45 PM	0	0	0	0	0	0	3	0	0	3	1	0	0	0	1	4
Hourly Total	0	2	0	0	2	1	11	0	0	12	6	0	0	0	6	20
Grand Total	5	14	0	3	19	20	65	0	1	85	51	7	0	2	58	162
Approach %	26.3	73.7	0.0	-	-	23.5	76.5	0.0	-	-	87.9	12.1	0.0	-	-	-
Total %	3.1	8.6	0.0	-	11.7	12.3	40.1	0.0	-	52.5	31.5	4.3	0.0	-	35.8	-
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	5	13	0	-	18	19	63	0	-	82	51	7	0	-	58	158
% Cars & Light Goods	100.0	92.9	-	-	94.7	95.0	96.9	-	-	96.5	100.0	100.0	-	-	100.0	97.5
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	1	0	-	1	0	1	0	-	1	0	0	0	-	0	2
% Single-Unit Trucks	0.0	7.1	-	-	5.3	0.0	1.5	-	-	1.2	0.0	0.0	-	-	0.0	1.2
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	1	1	0	-	2	0	0	0	-	0	2
% Bicycles on Road	0.0	0.0	-	-	0.0	5.0	1.5	-	-	2.4	0.0	0.0	-	-	0.0	1.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	3	-	-	-	-	1	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited  
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Count Name: Mary Street & Jane Street  
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Start Date: 07/07/2021  
Page No: 3



Turning Movement Data Plot

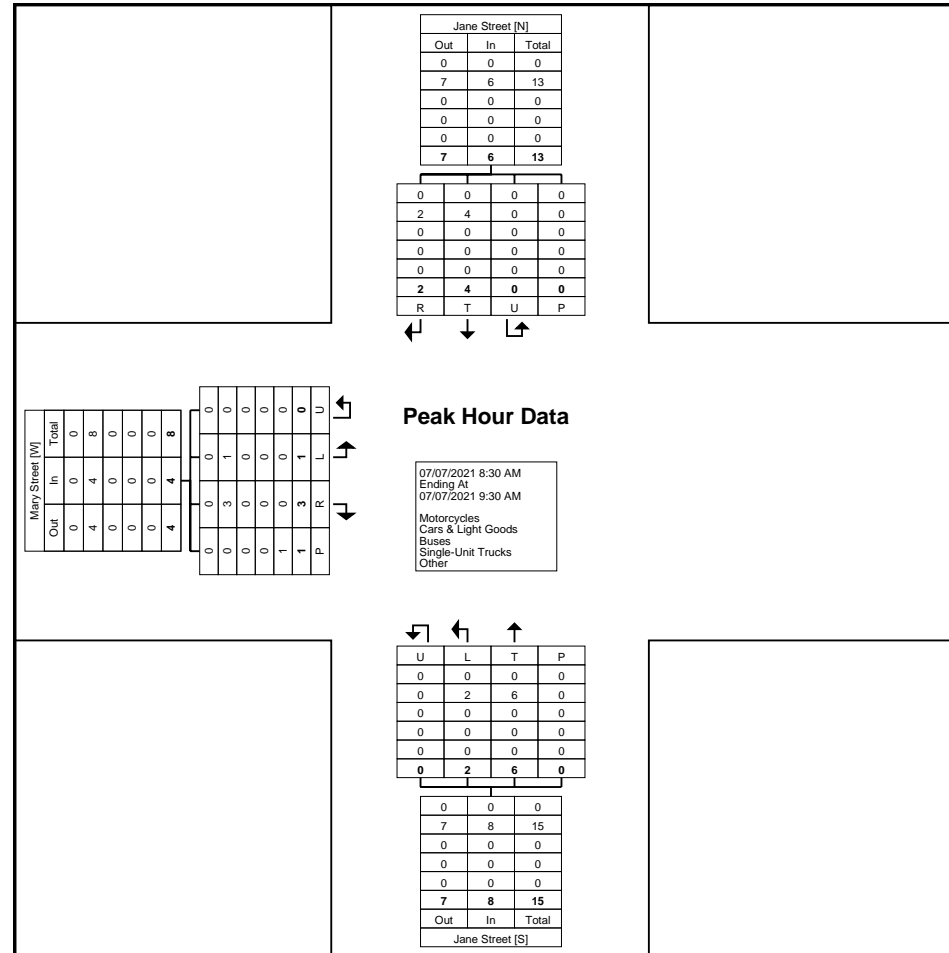




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Count Name: Mary Street & Jane Street  
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Page No: 5



Turning Movement Peak Hour Data Plot (8:30 AM)

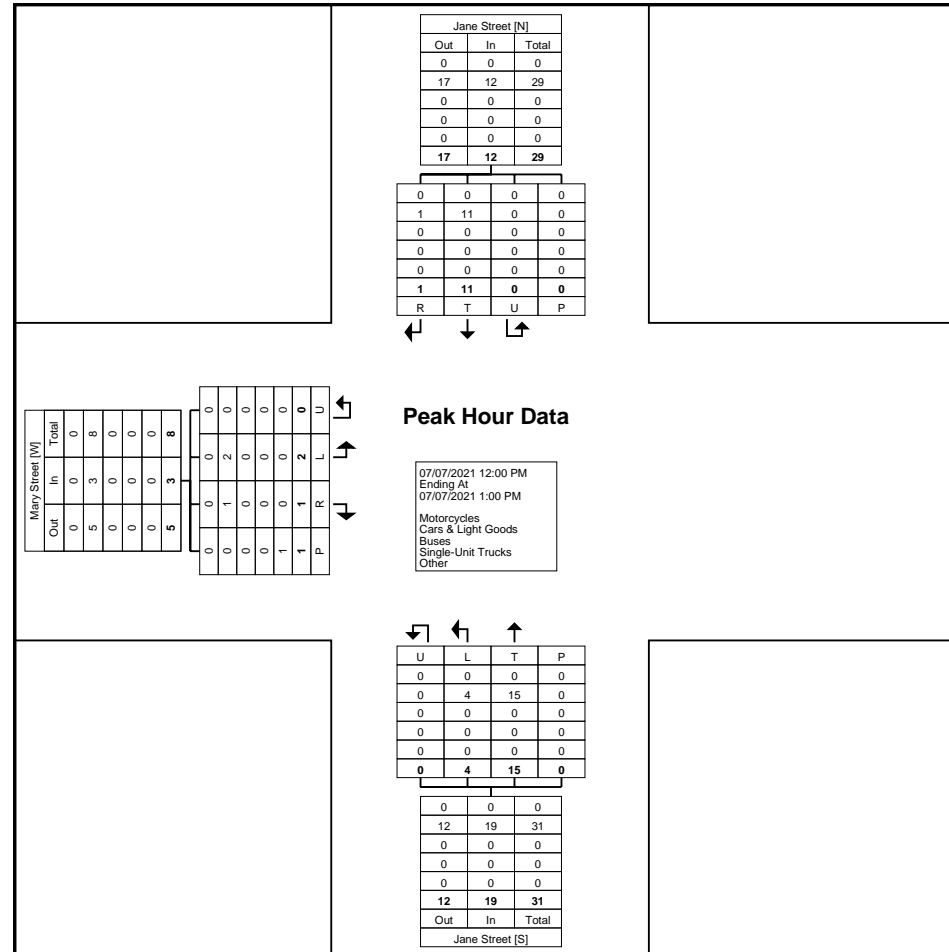




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Count Name: Mary Street & Jane Street  
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Start Date: 07/07/2021  
Page No: 7



Turning Movement Peak Hour Data Plot (12:00 PM)



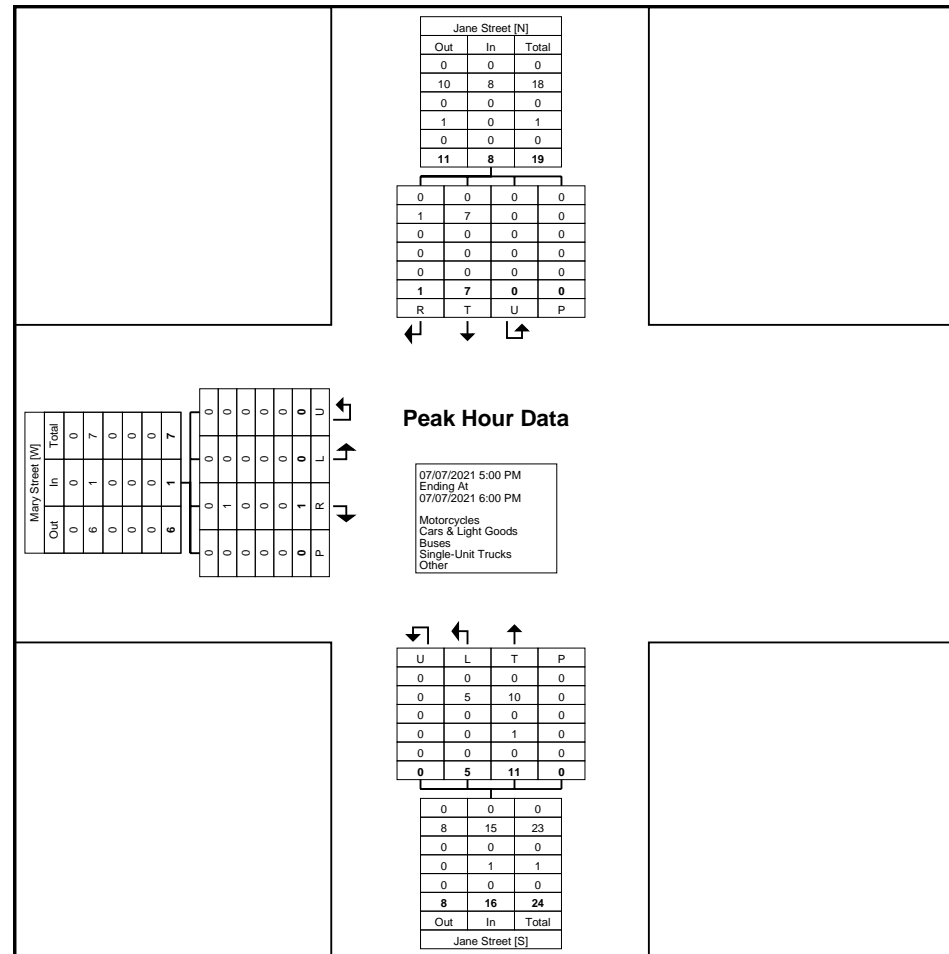




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Page No: 9



Turning Movement Peak Hour Data Plot (5:00 PM)

## Appendix B

### Existing Traffic Operations Reports



Lanes, Volumes, Timings

1: Private Driveway/Ontario Street & Main Street West

Existing AM Peak Hour

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	157	0	0	179	0	0	0	0	0	0	1
Future Volume (vph)	0	157	0	0	179	0	0	0	0	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt											0.865	
Fit Protected												
Satd. Flow (prot)	0	1727	0	0	1712	0	0	1900	0	0	1644	0
Fit Permitted												
Satd. Flow (perm)	0	1727	0	0	1712	0	0	1900	0	0	1644	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		186.9			251.4			141.3			170.4	
Travel Time (s)		13.5			18.1			10.2			12.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	10%	0%	0%	11%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	171	0	0	195	0	0	0	0	0	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	171	0	0	195	0	0	0	0	0	1	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization 19.4%	ICU Level of Service A
Analysis Period (min)	15

HCM 6th TWSC

1: Private Driveway/Ontario Street & Main Street West

Existing AM Peak Hour

210351

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	157	0	0	179	0	0	0	0	0	0	1
Future Vol, veh/h	0	157	0	0	179	0	0	0	0	0	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	10	0	0	11	0	0	0	0	0	0	0
Mvmt Flow	0	171	0	0	195	0	0	0	0	0	0	1

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	195	0	0	171
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1390	-	-	1418
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1390	-	-	1418
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	9.2
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1390	-	-	1418	-	-	851
HCM Lane V/C Ratio	-	-	-	-	-	-	-	0.001
HCM Control Delay (s)	0	0	-	-	0	-	-	9.2
HCM Lane LOS	A	A	-	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0

Lanes, Volumes, Timings

1: Private Driveway/Ontario Street & Main Street West

Existing PM Peak Hour

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	236	0	0	238	0	0	0	0	0	0	0
Future Volume (vph)	0	236	0	0	238	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fit Protected												
Satd. Flow (prot)	0	1827	0	0	1776	0	0	1900	0	0	1900	0
Fit Permitted												
Satd. Flow (perm)	0	1827	0	0	1776	0	0	1900	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		186.9			251.4			141.3			170.4	
Travel Time (s)		13.5			18.1			10.2			12.3	
Confl. Peds. (#/hr)	1					1						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	257	0	0	259	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	257	0	0	259	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.9%					ICU Level of Service A						
Analysis Period (min)	15											

HCM 6th TWSC

1: Private Driveway/Ontario Street & Main Street West

Existing PM Peak Hour

210351

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	236	0	0	238	0	0	0	0	0	0	0
Future Vol, veh/h	0	236	0	0	238	0	0	1	0	0	0	0
Conflicting Peds, #/hr	1	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	4	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	0	257	0	0	259	0	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	260	0	0	257
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1316	-	-	1320
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1315	-	-	1320
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	0
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1315	-	-	1320	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-	-	-	-
HCM Control Delay (s)	0	0	-	-	0	-	-	0
HCM Lane LOS	A	A	-	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-

## Appendix C

### Background Traffic Operations Reports



Lanes, Volumes, Timings

2026 Background AM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	173	0	0	198	0	0	0	0	0	0	1
Future Volume (vph)	0	173	0	0	198	0	0	0	0	0	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt											0.865	
Fit Protected												
Satd. Flow (prot)	0	1727	0	0	1712	0	0	1900	0	0	1644	0
Fit Permitted												
Satd. Flow (perm)	0	1727	0	0	1712	0	0	1900	0	0	1644	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		186.9			251.4			141.3			170.4	
Travel Time (s)		13.5			18.1			10.2			12.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	10%	0%	0%	11%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	188	0	0	215	0	0	0	0	0	0	1
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	188	0	0	215	0	0	0	0	0	1	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	20.4%
ICU Level of Service	A
Analysis Period (min)	15

HCM 6th TWSC

2026 Background AM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	173	0	0	198	0	0	0	0	0	0	1
Future Vol, veh/h	0	173	0	0	198	0	0	0	0	0	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	10	0	0	11	0	0	0	0	0	0	0
Mvmt Flow	0	188	0	0	215	0	0	0	0	0	0	1

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	215	0	0	188
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1367	-	-	1398
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1367	-	-	1398
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	9.3
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1367	-	-	1398	-	-	830
HCM Lane V/C Ratio	-	-	-	-	-	-	-	0.001
HCM Control Delay (s)	0	0	-	-	0	-	-	9.3
HCM Lane LOS	A	A	-	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0

Lanes, Volumes, Timings

2026 Background PM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	261	0	0	263	0	0	0	0	0	0	0
Future Volume (vph)	0	261	0	0	263	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Fit Protected												
Satd. Flow (prot)	0	1827	0	0	1776	0	0	1900	0	0	1900	0
Fit Permitted												
Satd. Flow (perm)	0	1827	0	0	1776	0	0	1900	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		186.9			251.4			141.3			170.4	
Travel Time (s)		13.5			18.1			10.2			12.3	
Confl. Peds. (#/hr)	1					1						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	284	0	0	286	0	0	0	0	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	284	0	0	286	0	0	0	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	17.2%			ICU Level of Service A								
Analysis Period (min)	15											

HCM 6th TWSC

2026 Background PM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	261	0	0	263	0	0	0	0	0	0	0
Future Vol, veh/h	0	261	0	0	263	0	0	1	0	0	0	0
Conflicting Peds, #/hr	1	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	4	0	0	0	7	0	0	0	0	0	0
Mvmt Flow	0	284	0	0	286	0	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	287	0	0	284
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1287	-	-	1290
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1286	-	-	1290
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0	0	0
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1286	-	-	1290	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-	-	-	-
HCM Control Delay (s)	0	0	-	-	0	-	-	0
HCM Lane LOS	A	A	-	-	A	-	-	A
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	-



## Appendix D

### Total Traffic Operations Reports



Lanes, Volumes, Timings

2026 Total AM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	4	173	0	0	198	4	0	0	0	12	0	15
Future Volume (vph)	4	173	0	0	198	4	0	0	0	12	0	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.998						0.926	
Fit Protected		0.999									0.978	
Satd. Flow (prot)	0	1729	0	0	1711	0	0	1900	0	0	1721	0
Fit Permitted		0.999									0.978	
Satd. Flow (perm)	0	1729	0	0	1711	0	0	1900	0	0	1721	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		186.9			251.4			141.3			170.4	
Travel Time (s)		13.5			18.1			10.2			12.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	10%	0%	0%	11%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	188	0	0	215	4	0	0	0	13	0	16
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	192	0	0	219	0	0	0	0	0	29	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		0.0			0.0			0.0			0.0	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 22.3%

ICU Level of Service A

Analysis Period (min) 15

HCM 6th TWSC

2026 Total AM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	4	173	0	0	198	4	0	0	0	12	0	15
Future Vol, veh/h	4	173	0	0	198	4	0	0	0	12	0	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	10	0	0	11	0	0	0	0	0	0	0
Mvmt Flow	4	188	0	0	215	4	0	0	0	13	0	16

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	219	0	0	188
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1362	-	-	1398
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1362	-	-	1398
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.2	0	0	10.6
HCM LOS			A	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1362	-	-	1398	-	-	677
HCM Lane V/C Ratio	-	0.003	-	-	-	-	-	0.043
HCM Control Delay (s)	0	7.7	0	-	0	-	-	10.6
HCM Lane LOS	A	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

Lanes, Volumes, Timings

2026 Total PM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	14	261	0	0	263	14	0	0	0	8	0	8
Future Volume (vph)	14	261	0	0	263	14	0	0	0	8	0	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt												
Fit Protected	0.997											
Satd. Flow (prot)	0	1825	0	0	1769	0	0	1900	0	0	1728	0
Fit Permitted	0.997											
Satd. Flow (perm)	0	1825	0	0	1769	0	0	1900	0	0	1728	0
Link Speed (k/h)	50											
Link Distance (m)	186.9			251.4			141.3			170.4		
Travel Time (s)	13.5		18.1		10.2		12.3					
Confl. Peds. (#/hr)	1											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	7%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	15	284	0	0	286	15	0	0	0	9	0	9
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	299	0	0	301	0	0	0	0	0	18	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0											
Link Offset(m)	0.0											
Crosswalk Width(m)	4.8			4.8			4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25	25	15	25	25	15	25	15	25	25	15	25
Sign Control	Free		Free		Stop		Stop					

Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	35.2%			ICU Level of Service A								
Analysis Period (min)	15											

HCM 6th TWSC

2026 Total PM Peak Hour

1: Private Driveway/Ontario Street & Main Street West

210351

Intersection												
Int Delay, s/veh	0.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	14	261	0	0	263	14	0	0	0	8	0	8
Future Vol, veh/h	14	261	0	0	263	14	0	0	0	8	0	8
Conflicting Peds, #/hr	1	0	0	0	0	1	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	4	0	0	7	0	0	0	0	0	0	0
Mvmt Flow	15	284	0	0	286	15	0	0	0	9	0	9

Major/Minor	Major1	Major2	Minor1	Minor2
Conflicting Flow All	302	0	0	284
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	4.1	-	-	4.1
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	2.2	-	-	2.2
Pot Cap-1 Maneuver	1270	-	-	1290
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	1269	-	-	1290
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.4	0	0	12.1
HCM LOS			A	B

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	1269	-	-	1290	-	-	525
HCM Lane V/C Ratio	-	0.012	-	-	-	-	-	0.033
HCM Control Delay (s)	0	7.9	0	-	0	-	-	12.1
HCM Lane LOS	A	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	-	0	-	-	0	-	-	0.1

## Appendix E

### OTM Signal Warrants



## Signal Justification Calculation for Forecasted Volumes (OTM Book 12 - Justification 7)



Horizon Year: 2026  
Region/City/Township: Wellington County

Major Street: Main Street North/South?: N  
Minor Street: Ontario Street

Number of Approach Lanes: 1  
Tee Intersection?: N  
Flow Conditions: Free  
  
PM Forecast Only? N

Warrant Results			
150% Satisfied	No	Justification for new intersections with forecast traffic	
120% Satisfied	No	Justification for existing intersections with forecast traffic	

Time Period	Major Street Main Street						Minor Street Ontario Street						Peds Crossing Main Road
	Eastbound			Westbound			Northbound			Southbound			
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
	AM Peak Hour	4	173	0	0	198	4	0	0	0	12	0	
PM Peak Hour	14	261	0	0	263	14	0	0	0	8	0	8	0
Average Hourly Volume	5	109	0	0	115	5	0	0	0	5	0	6	0

Warrant	AHV
1A - All	244
1B - Minor	11
2A - Major	233
2B - Cross	5

### Warrant 1 - Minimum Vehicular Volume

1A	Approach Lanes	1		2 or more		Average Hourly Volume
	Flow Conditions	Free	Restricted	Free	Restricted	
	All Approaches	X				
		480	720	600	900	244
		% Fulfilled				50.7%

1B	Approach Lanes	1		2 or more		Average Hourly Volume
	Flow Conditions	Free	Restricted	Free	Restricted	
	Minor Street Approaches	X				
		120	170	120	170	11
		% Fulfilled				9.0%

### Warrant 2 - Delay To Cross Traffic

2A	Approach Lanes	1		2 or more		Average Hourly Volume
	Flow Conditions	Free	Restricted	Free	Restricted	
	Major Street Approaches	X				
		480	720	600	900	233
		% Fulfilled				48.5%

2B	Approach Lanes	1		2 or more		Average Hourly Volume
	Flow Conditions	Free	Restricted	Free	Restricted	
	Traffic Crossing Major Street	X				
		50	75	50	75	5
		% Fulfilled				10.0%