



Preliminary Servicing & Stormwater Management Design Report for:

Ainley Farm Subdivision
Township of Centre Wellington (Elora)

GMBP File: 411009
Revised April 2023

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**PRELIMINARY SERVICING &
STORMWATER MANAGEMENT REPORT
AINLEY FARM SUBDIVISION
TOWNSHIP OF CENTRE WELLINGTON (ELORA)
Revised April 2023
Our File: 411009**

1.0 INTRODUCTION

In support of the Draft Plan of Subdivision Application for Part of Lots 17 and 18, Concession 12 in the Township of Centre Wellington (Geographic Township of Nichol) herein after referred to as the Ainley Farm Subdivision, GM BluePlan Engineering Limited have prepared this report to address the preliminary servicing and stormwater management requirements for the site and to address the comments received from Grand River Conservation Authority (dated July 31, 2018) and from the Township of Centre Wellington (dated March 20, 2020).

The servicing and stormwater management techniques were derived from the recommendations presented in the following reports:

- Stormwater Management Plan for The North Valley Subdivision (Cambridge Engineering and Planning Consultants Limited, January 1994),
- Design Report, Ville Lora Downs North Subdivision, Phase III (Gamsby and Mannerow Limited, July 2004),
- Design Report, Ville Lora Downs Subdivision, Stage VI (Gamsby and Mannerow Limited, April 1998),
- Preliminary Geotechnical Investigation completed CMT Engineering Inc. (March 29, 2006), and
- Environmental Impact Study completed by North-South Environmental Inc. (July 24, 2019).

Together, these reports form the overview for the development of these lands while maintaining the adjacent natural features.

2.0 SITE INFORMATION

Figure 1 shows the location of the Ainley Farm Subdivision and the surrounding area. The 21.46-hectare site is bound by existing agricultural and future development lands to the north, Gerrie Road to the east, existing residential lands to the south (Ville Lora Downs Subdivision, Phase V and Phase VI) and existing wetland and residential lands to the west (Ville Lora Downs North Subdivision, Phase III).

3.0 EXISTING CONDITIONS

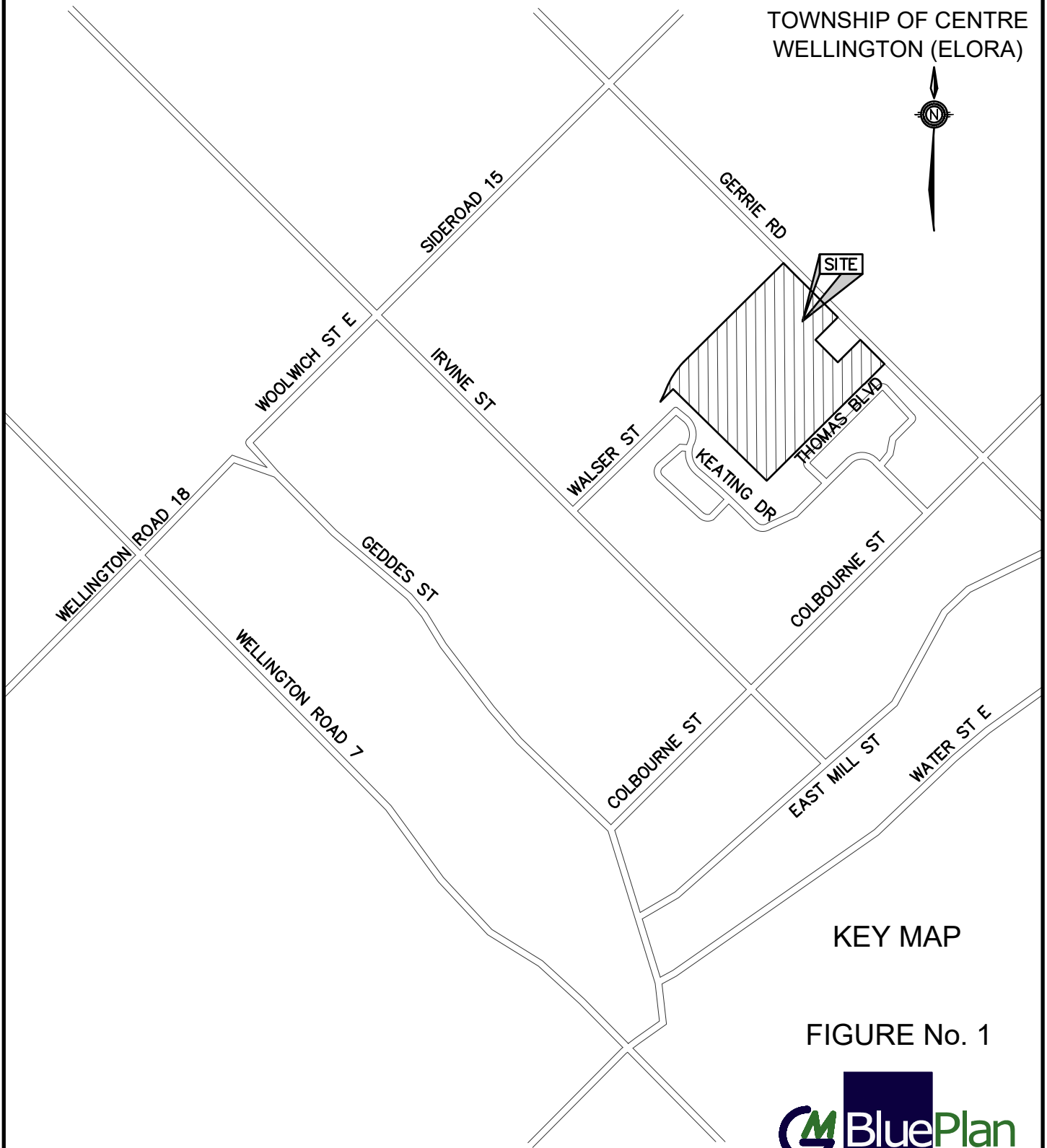
3.1 LAND USE

The site is currently comprised of agricultural fields and a natural heritage feature consisting of a woodlot and wetland. The existing site features are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 to 4).

3.2 TOPOGRAPHY

The topography throughout the Ainley Farm Subdivision is undulating and consists of rolling slopes with gradients ranging from 0.5% to 20%. Original ground elevations on site range from approximately 410.0m to approximately 416.0m. The northeastern portion of the site generally drains in a northeast direction towards Gerrie Road. The remainder of the site generally drains in a southwest direction towards the existing wetland, ultimately discharging to the existing channel located immediately south of the wetland. The northwestern

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KEY MAP

FIGURE No. 1



JOB NUMBER 411009
FEBRUARY 2019
NOT TO SCALE

portion of the site, adjacent to the existing Walser Street right-of-way, drains in a southerly direction towards Walser Street.

3.3 SOILS

The predominant surface soil type on the site is Harriston Loam (Soil Survey of Wellington County Report No. 35). Harriston Loam has a hydrologic soil classification of BC and generally has good drainage characteristics.

The Preliminary Geotechnical Investigation by CMT Engineering Inc. (March 2006) established the characteristics of the underlying soils. The boreholes identified the underlying soils as topsoil overlying organic silt, silt or sandy silt, silt till or sandy silt till, sand or silty sand and clayey silt. The results of the geotechnical investigation are included in Appendix 'A'.

4.0 PROPOSED DEVELOPMENT

The Draft Plan of Subdivision, prepared by J.D. Barnes Limited (March 23, 2023) (Figure 2), illustrates the proposed lot fabric, internal roads, park block, and open space areas and stormwater management blocks.

Access to the 21.46-hectare development will be provided via Gerrie Road and the extension of Walser Street.

Within the development, there are 101 single family lots, three (3) on-street townhouse blocks, one (1) apartment block, one (1) cluster townhouse block, one (1) open space block, one (1) park block and two (2) stormwater management blocks.

In addition, four (4) future single detached lots will be created on the north side of Walser Avenue through the extension of Walser Avenue into the Ainley Farm property (Future Development Block 120).

4.1 SITE GRADING

The site layout and internal road network for the Ainley Farm Subdivision are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 to 4). The grade and elevation of the internal streets are controlled by the existing centre line elevations of Walser Street and Gerrie Road, the major overland flow route to the stormwater management facilities, existing groundwater elevations and the elevation of the existing sanitary sewers on Walser Street and Keating Drive.

The site has been graded to match the existing elevations along the property boundary of the adjacent lands. Minor grading on the adjacent lands located along the north boundary of the site is required. The adjacent lands along the north boundary of the site are owned by the Developer (James Keating Construction (2004) Limited).

4.2 STREETS

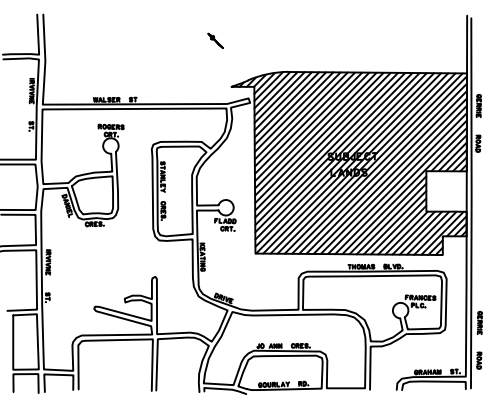
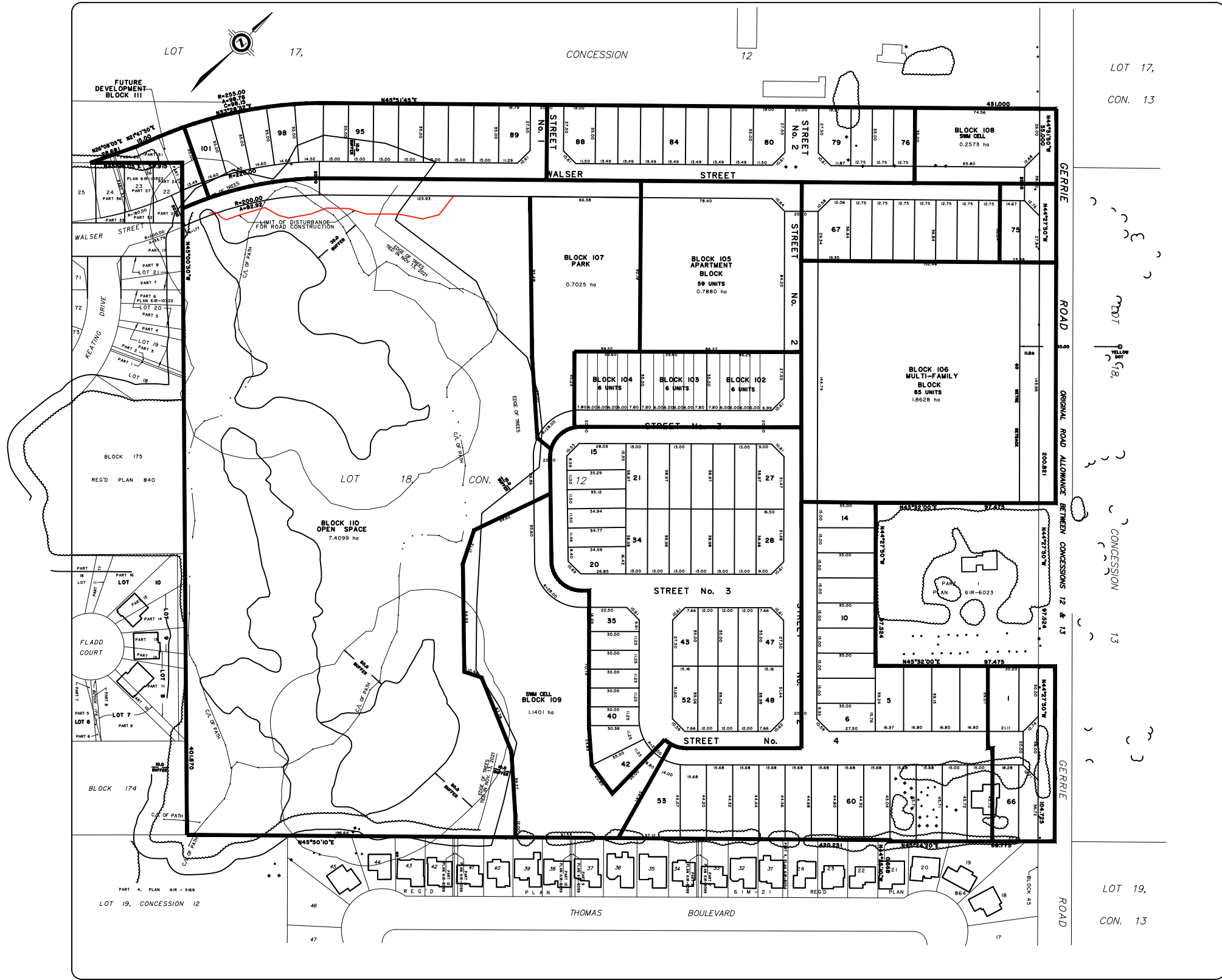
All streets will be constructed with a minimum grade of 0.5% and a maximum grade of 8.0% as per Township of Centre Wellington standards. An urban road cross-section (20 m right-of-way width), with concrete curb and gutter will be provided for Street No.1, 2, 3, 4 and the extension of Walser Street, as per Township of Centre Wellington Standard Drawing STD R1.

Concrete sidewalks (1.5 metre wide) will be constructed along both sides of the Walser Street extension and Street No. 1, 2, 3 and 4.

4.3 WATER SUPPLY

As part of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter watermain was terminated at the easterly limit of Walser Street. There is currently no watermain on Gerrie Road across the frontage of the Ainley Farm Subdivision.

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KEY PLAN N.T.S.

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51 (17) OF THE PLANNING ACT

51 (17) (a) - (g) AS SHOWN
(a) SEE SCHEDULE
(b) AS SHOWN
(c) AS SHOWN
(d) AS SHOWN
(e) AS SHOWN
(f) AS SHOWN
(g) NONE KNOWN

WE HEREBY AUTHORIZE BLACK, SHOEMAKER, ROBINSON, AND DONALDSON LIMITED, ONTARIO LAND SURVEYORS, URBAN AND RURAL PLANNERS TO SUBMIT THIS DRAFT PLAN OF PROPOSED SUBDIVISION.

DATE: JAMES KEATING CONSTRUCTION (2004) LTD.

I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LANDS TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE: KERRY F. HILLIS
ONTARIO LAND SURVEYOR

DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

RELEVANT INFORMATION

LOTS/BLOCKS	LAND USE	UNITS	AREAS (ha)
LOTS 1 TO 101	SINGLE-DETACHED RESIDENTIAL	101	5.6078
BLOCKS 102 TO 104	STREET TOWNHOUSES	18	0.4384
BLOCK 105	APARTMENT SITE	59	0.7880
BLOCK 106	CLUSTER TOWNHOUSES	65	1.8628
BLOCK 107	PARK		0.7025
BLOCKS 108 & 109	STORMWATER MANAGEMENT		1.3974
BLOCK 110	OPEN SPACE		7.3880
STREETS	ROADS		3.2004
BLOCK III	FUTURE DEVELOPMENT	4	0.0816
TOTAL		247	21.4603 ha.

NOTES:
ELEVATIONS AND RELEVANT INFORMATION TAKEN FROM

DRAFT PLAN OF
SUBDIVISION
OF PART OF
LOTS 17 & 18, CONCESSION 12
TOWNSHIP OF CENTRE WELLINGTON
(GEOGRAPHIC TOWNSHIP OF NICHOL)
COUNTY OF WELLINGTON

SCALE: 1:10,000

J.D. BARNES
LAND SURVEYOR
DATE: MARCH 23, 2023
DRAWN BY: H.S./MA
PROJECT: 04-5865-18
6-14-086-01

DRAFT PLAN OF
SUBDIVISION
FIGURE No. 2



Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200mm diameter watermain, along the Walser Street extension, Street No. 2 and a portion of Street No. 1. A 150mm diameter watermain will also be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.

4.4 SANITARY SEWER

During the municipal servicing of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter sanitary sewer was designed, approved and constructed on Walser Street. The existing 200mm diameter sanitary sewer is currently terminated at the easterly limit of Walser Street. As part of the Ville Lora Downs Subdivision, Phase VI, a 200mm diameter sanitary sewer was also designed, approved and constructed on Keating Drive. There are currently no sanitary sewers on Gerrie Road across the frontage of the Ainley Farm.

Sanitary service for the Ainley Farm Subdivision will be provided via connections to both the existing 200mm diameter sanitary sewer on Walser Street and the existing 200mm diameter sanitary sewer on Keating Drive.

The extension of a 200mm diameter sanitary sewer along the Walser Street extension will service the lots fronting on to Walser Street, as well as a portion of the lots fronting onto Street No. 2. The extension of a 200mm diameter sanitary sewer on easement through Drimmie Park to Street No. 1, from the existing 200mm diameter sanitary sewer on Keating Drive, will service the remainder of the subdivision (Street No. 2, Street No. 3 and Street No. 4).

Sanitary sewer design sheets have been included in Appendix C.

4.5 STORM SEWER

The storm sewer system for the Ainley Farm Subdivision will be sized to convey the 5-year design storm event and the storm sewer system will discharge to the two (2) proposed stormwater management facilities or to the existing storm sewer on Walser Street.

The storm sewers on Street No. 1, Street No. 3, Street No. 4, a portion of Street No. 2 and a portion of the Walser Street extension will discharge to the proposed Stormwater Management Facility No. 1 located to the east of the existing wetland.

The storm sewers on the remainder of Street No. 2, along with a portion of the Walser Street extension, will discharge to the proposed Stormwater Management Facility No. 2 located to the west of Gerrie Road.

The storm sewers on the remainder of the Walser Street extension will discharge directly to the existing storm sewer system on Walser Street, ultimately discharging to the existing storm sewers on Keating Drive.

Storm sewer design sheets have been included in Appendix C.

4.6 DEWATERING

Dewatering may be required during the installation of sanitary sewer, storm sewer and watermain. A Permit to Take Water (PTTW) or an Environmental Activity and Sector Registry (EASR) from the Ministry of Environment, Conservation and Parks (MECP) will be required if dewatering activities will involve the removal of more than 400,000 litres of groundwater per day from the site.

If dewatering activities are required during the installation of sewers and watermain, all discharge will be directed to the interim sediment control pond prior to discharge from the site.

As part of the area grading of the site, the interim stormwater management facility will be constructed and will act as an interim sedimentation control pond for the remainder of the municipal servicing and home building construction. This will prevent sediment from being discharged to the wetland. Upon build-out, accumulated

sediment will be collected and removed from the interim sediment control pond before it is constructed on Stormwater Management Facility No. 1.

4.7 FOUNDATION DRAINAGE

As per the Township of Centre Wellington municipal standards, foundation drainage will be provided via sump pits and sump pumps in each residential unit, ultimately discharging via individual storm sewer lateral connections to the storm sewer system located within the municipal right-of-way.

5.0 STORMWATER MANAGEMENT

5.1 DESIGN CRITERIA

The studies, policies and guidelines used to develop the stormwater management plan for this development were as follows:

- 1) Stormwater Management Planning and Design Manual, 2003
- 2) Design Principles for Stormwater Management Facilities, 1996
- 3) The Interim Stormwater Quality Control Guidelines, 1991
- 4) The Stormwater Quality Best Management Practices Manual, 1991
- 5) The MTO Drainage Management Technical Guidelines, 1989
- 6) The Ontario Urban Design Guidelines, 1987

The objectives of the stormwater management plan are as follows:

- a) Provide Enhanced (80% Total Suspended Solids) water quality control prior to discharge to the existing wetland and to an existing tributary of the Grand River.
- b) Provide quantity control for the full range of design storms to attenuate post-development runoff to the existing condition level.
- c) Match pre- and post-development infiltration rates.
- d) Route the Regional Storm to minimize flood damage.

A three-hour duration rainfall event was used to generate the mass rainfall data required for the 2, 5, 10, 25, 50 and 100-year design storms. The Fergus Shand Dam Chicago parameters and the total depth of rainfall for each storm are as follows:

Table No. 1: Chicago Rainfall Distribution Parameters

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
a =	695.047	1459.072	2327.596	3701.648	5089.418	6933.019
b =	6.387	13.690	19.500	25.500	30.000	34.699
c =	0.793	0.850	0.894	0.937	0.967	0.998
r =	0.38	0.38	0.38	0.38	0.38	0.38
Duration = (minutes)	180	180	180	180	180	180
Rainfall Depth = (mm)	33.014	49.792	61.359	75.581	86.737	97.921

The SCS infiltration method was used in the runoff calculations. The CN parameters used in the MIDUSS modelling are as follows:

Table No. 2: SCS Curve Number Parameters

	IMPERVIOUS AREAS	PERVIOUS AREAS
Residential	98	78
Agricultural	98	74
Wetland/Forest	98	50

The hydrologic model MIDUSS was used to create the runoff hydrographs and to route the flows through the storage structures.

5.2 STORMWATER MANAGEMENT APPROACH

In line with current practices and guidelines, the stormwater management plan for the Ainley Farm Subdivision is a “treatment train” to attenuate post-development flows and to provide Enhanced (80% total suspended solids removal) water quality control treatment prior to discharge from the site. The “treatment train” will include a combination of lot level, conveyance and end-of-pipe best management practices.

Lot level controls will simply consist of directing roof leaders to grassed areas and grassed swales.

Conveyance controls will include the use of storm sewers, grassed swales, four (4) oil/grit separator structures for Stormwater Management Facility No.1 and Stormwater Management Facility No.2.

End-of-pipe controls will be provided by two (2) extended detention stormwater management facilities designed to attenuate post-development runoff prior to discharge from the site. Runoff generated from Stormwater Management Facility No.1 will discharge to the existing wetland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive. The stormwater management facility has been designed as a wetland with 5,464m³ of storage, discharging via a multi-stage outlet consisting of a minor outlet with a 300mm diameter orifice plate and a major outlet with a 350mm diameter orifice place, as well as a 20m wide overflow weir.

Runoff generated from Stormwater Management Facility No.2 will discharge to the roadside ditch along Gerrie Road, ultimately discharging to a tributary of the Grand River. The stormwater management facility has been designed as a wetland with 1,195m³ of storage, discharging via a multi-stage outlet consisting of a 120mm knockout for minor storms and a 260mm orifice plate for major storms, as well as a 10m wide overflow weir.

A small portion of runoff from the westerly portion of Walser Street will discharge uncontrolled to the existing storm sewer system on Walser Street.

Major storm flows from the development will sheetflow overland via the municipal right-of-ways to either Stormwater Management Facility No.1 or Stormwater Management Facility No. 2.

This combination of lot-level, conveyance and end-of-pipe controls will control the release of the runoff from the site.

5.3 STORMWATER MANAGEMENT PLAN

The best management practices (BMP's) in the Stormwater Management Planning and Design Manual (2003) were screened. Those found to be applicable to this development are discussed in the following sections.

5.3.1 LOT LEVEL CONTROLS

Stormwater management practices recommended to provide lot level control on this site are as follows:

a) Roof Drainage to Ground Surface

The driveways and front yards will drain to the street. The roof and rear yard will generally drain to the rear of the lot with exception for lots with back to front drainage.

The roof runoff will be filtered across the grassed surface and some will infiltrate. The runoff for any event large enough to generate flow to the swale system will be adequately filtered by the grass en route.

b) Rear Yard Swales

The lots will be graded to current Township of Centre Wellington Standards. Where practical, the length of the rear lot swales between catch basins will be increased to extend the contact time with the grassed surfaces.

To promote infiltration on the lots and in the swales, it is recommended that the average depth of graded topsoil be 300 mm.

5.3.2 CONVEYANCE CONTROLS

The storm conveyance system for the development will consist of grassed swales, storm sewers, major overland channel and four (4) oil/grit separator structures. Conveyance controls will be achieved through the regular maintenance of the grassed swales, storm sewers, major overland channel, and oil/grit separator structures as part of the Township's annual maintenance program. Maintenance requirements will include the annual removal of accumulated sediments and debris from manholes, catch basins, and oil/grit separator structures.

5.3.3 END-OF-PIPE CONTROLS

a) Existing Conditions

Under existing conditions, the majority of the site is utilized for agricultural purposes. For hydrologic modelling purposes, the 21.42-hectare site and 1.20 hectares of external areas was modelled as seven (7) catchments. These catchments are shown on the Existing Conditions Storm Drainage Area Plan (Figure 3).

Catchment 10 (7.66 hectares, 0% impervious) consists primarily of agricultural lands and an existing residential lot.

Catchment 11 (0.13 hectares, 0% impervious) represents the external lands, which consists primarily of agricultural lands of an existing residential lot.

Runoff generated from Catchment 10 and 11 currently sheetflows overland in an east to west direction, ultimately discharging to the existing wetland.

Catchment 20 (6.66 hectares, 0% impervious) consists primarily of agricultural lands and an existing residential lot.

Catchment 21 (0.83 hectares, 10% impervious) represents external lands consisting of an existing residential dwelling.

Runoff generated from Catchment 20 and 21 currently sheetflows overland to the existing roadside ditch along Gerrie Road and ultimately to a tributary of the Grand River.

Catchment 30 (0.24 hectares, 0% impervious) represents the external lands, which consists primarily of an existing wetland and agricultural lands.

Runoff generated from Catchment 30 currently sheetflows overland, ultimately discharging to the existing wetland.

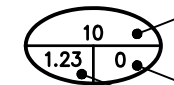
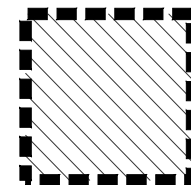
Catchment 40 (7.12 hectares, 0% impervious) represents the south-westerly portion of the site, consisting of a natural heritage feature (wetland and woodlot).

Runoff generated from Catchment 40 currently sheetflows overland in an east to west direction, discharging to an existing swale in Drimmie Park and ultimately the existing storm sewer system on Keating Drive.

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LEGEND

-  DRAINAGE AREA BOUNDARY
-  CATCHMENT NUMBER
% IMPERVIOUS
AREA IN HECTARES
-  EXTERNAL LANDS

EXISTING
CONDITIONS STORM
DRAINAGE AREA
PLAN

FIGURE No. 3



JOB NUMBER 411009
FEBRUARY 2019
Scale: 1:2500

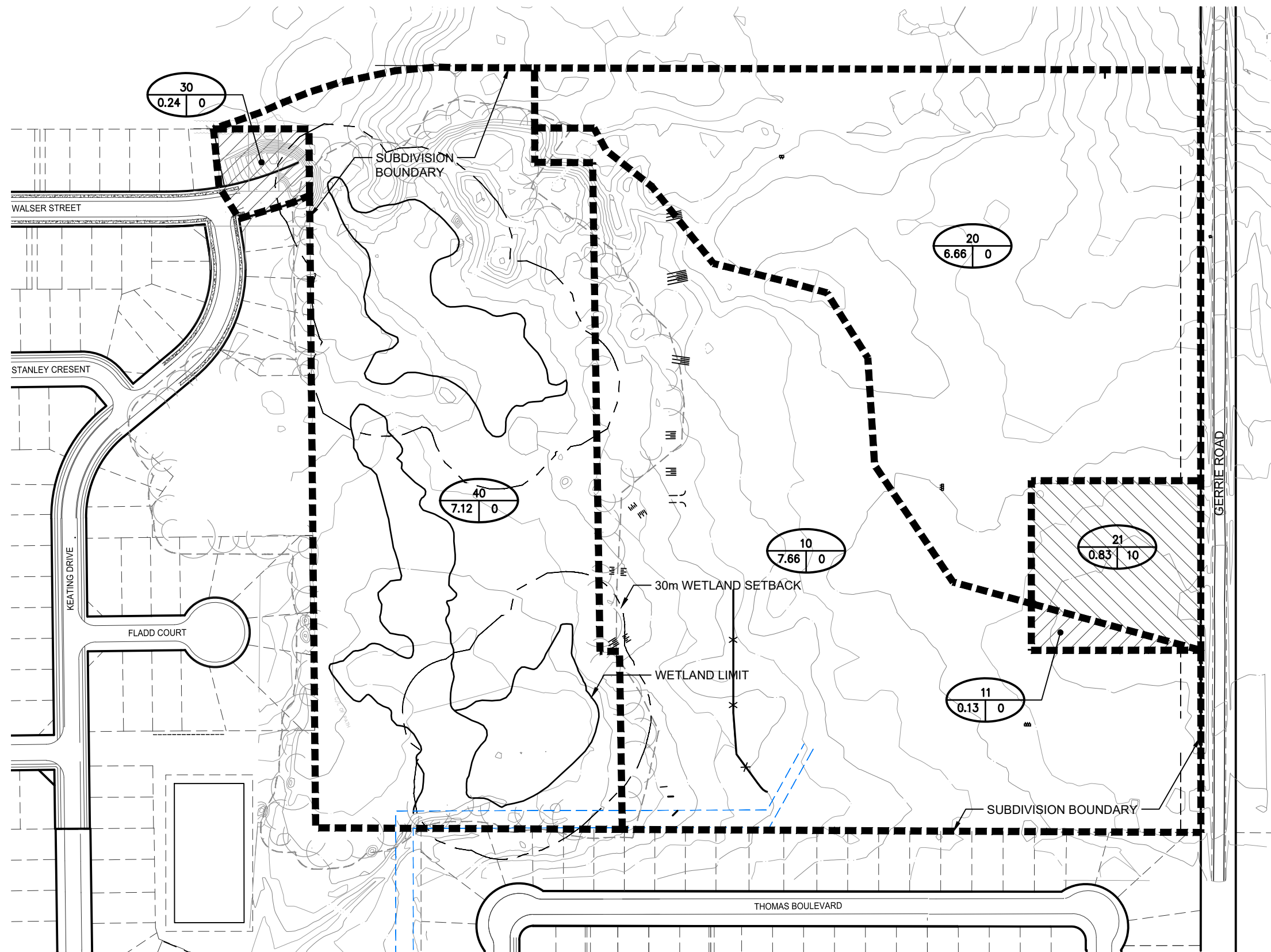


Table No. 3 summarizes the existing condition flow rates and runoff volumes from the site for the full range of design storm events.

Table No. 3: Existing Condition Flow Rates and Runoff Volumes

	CATCHMENTS									
	30	Total to Walser	10	11	40	Total to Ex. Wetland	20	21	Total to Tributary of Grand River	Total from Site
2-Year										
Flow Rate (m³/s)	0.003	0.003	0.044	0.001	0.060	0.099	0.038	0.016	0.043	0.138
Runoff Volume (m³)	12.3	12.3	397.3	6.7	364.5	768.4	340.5	61.1	401.6	1,170.0
5-Year										
Flow Rate (m³/s)	0.010	0.010	0.157	0.004	0.209	0.353	0.135	0.030	0.153	0.472
Runoff Volume (m³)	30.7	30.7	995.9	16.7	913.0	1,925.6	853.4	132.1	985.5	2,911.1
10-Year										
Flow Rate (m³/s)	0.017	0.017	0.273	0.008	0.359	0.608	0.234	0.050	0.264	0.806
Runoff Volume (m³)	46.5	46.5	1,505.1	25.2	1,380.7	2,911.0	1,289.8	190.6	1,480.4	4,391.4
25-Year										
Flow Rate (m³/s)	0.026	0.026	0.454	0.012	0.584	1.001	0.389	0.079	0.436	1.308
Runoff Volume (m³)	68.1	68.1	2,210.7	37.0	2,027.3	4,274.9	1,894.4	269.8	2,164.2	6,439.2
50-Year										
Flow Rate (m³/s)	0.035	0.035	0.618	0.016	0.789	1.341	0.530	0.104	0.593	1.755
Runoff Volume (m³)	86.8	86.8	2,811.2	47.1	2,577.4	5,435.7	2,409.1	336.8	2,745.9	8,181.5
100-Year										
Flow Rate (m³/s)	0.043	0.043	0.801	0.020	0.985	1.721	0.687	0.135	0.765	2.244
Runoff Volume (m³)	106.3	106.3	3,447.0	57.6	3,160.5	6,665.1	2,953.9	406.6	3,360.5	10,025.6
Regional										
Flow Rate (m³/s)	0.027	0.027	0.881	0.014	0.772	1.667	0.755	0.088	0.841	2.444
Runoff Volume (m³)	487.6	487.6	15,780.0	267.9	14,536.0	30,583.9	13,523.0	1,740.4	15,263.6	45,846.2

Table No. 4 gives the results of the ponding in the existing wetland.

Table No. 4: Wetland (Stage/Storage/Discharge)

	Available Capacity			Actual Capacity Used			Drawdown Time (hr)**
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.095	57	409.65	9.3
5-Year	---	---	---	0.322	195	409.69	5.6
10-Year	---	---	---	0.546	330	409.73	5.6
25-Year	---	---	---	0.881	534	409.77	5.5
50-Year	---	---	---	1.176	715	409.79	5.4
100-Year	---	---	---	1.499	912	409.82	5.4
Regional Storm	---	---	---	1.601	971	409.83	52.0
Overflow	18.965	15,227.7	410.75	---	---	---	---

**Drawdown time obtained from the hydrologic modelling software MIDUSS

Table No. 5 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

Table No. 5: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 1 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.095	0.162	0.541
5-Year	---	---	---	0.322	0.256	0.734
10-Year	---	---	---	0.546	0.312	0.838
25-Year	---	---	---	0.881	0.373	0.944
50-Year	---	---	---	1.176	0.416	1.015
100-Year	---	---	---	1.499	0.455	1.078
Regional Storm	---	---	---	1.601	0.467	1.096
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 6 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

Table No. 6: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 2 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.094	0.082	0.510
5-Year	---	---	---	0.321	0.167	0.772
10-Year	---	---	---	0.544	0.224	0.912
25-Year	---	---	---	0.876	0.290	1.054
50-Year	---	---	---	1.174	0.340	1.148
100-Year	---	---	---	1.490	0.385	1.230
Regional Storm	---	---	---	1.595	0.399	1.254
Top of Bank	9.246	0.95	1.966	---	---	---

b) Proposed Release Rates

In order to maintain the existing condition drainage pattern to the existing wetland and Grand River tributary, the release rates have been determined by the existing condition release rates. Under post-development conditions, runoff generated from the site will be attenuated to the existing condition levels conveyed to the existing wetland and the Grand River tributary.

The release rate to Walser Street under minor storm design events is determined by the capacity of the existing storm sewers in Ville Lora Downs North Subdivision Phase III, which were designed to incorporate a small contributing area from Walser Street. The capacity of the existing 300mm diameter storm sewer conveying a portion of the proposed development is approximately 0.110 m³/s, based on a grade of 1.34%. Excluding the existing contributing area to this storm sewer (0.05ha), the proposed development's allotment of the pipe's capacity is 0.106 m³/s. The allowable to Walser Street under major storm events has been determined based on the allotted area of 0.65 ha, as per the Villa Lora Downs North Phase III storm drainage area plans.

Therefore, the proposed release rates from the site under post-development conditions are outlined in Table No. 7.

Table No. 7: Proposed Release Rates

Release Route	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	Regional
To Ex. Wetland	0.099 m ³ /s	0.353 m ³ /s	0.608 m ³ /s	1.001 m ³ /s	1.341 m ³ /s	1.721 m ³ /s	1.667 m ³ /s
To Tributary of Grand River	0.043 m ³ /s	0.153 m ³ /s	0.264 m ³ /s	0.436 m ³ /s	0.593 m ³ /s	0.765 m ³ /s	0.841 m ³ /s
To Walser Street	0.106 m ³ /s		0.122 m ³ /s	0.150m ³ /s	0.173m ³ /s	0.196m ³ /s	0.080m ³ /s

c) Post-Development Conditions

Under post-development conditions, the existing drainage patterns of the site will be maintained. Post-development flows from the site will be attenuated to existing condition levels through the use of two (2) stormwater management facilities. Stormwater Management Facility No. 1 will outlet to the existing wetland. Stormwater Management Facility No. 2 will outlet to the existing roadside ditch along Gerrie Road and ultimately a tributary of the Grand River.

For the post-development condition analysis, the 21.42-hectare site and 1.20 hectares of external areas was modelled as fifteen (15) drainage catchments. These catchments are shown on the Post-Development Storm Drainage Area Plan (Figure 4).

Catchment 1000 (6.98-hectares, 50% Impervious) represents a central portion of the development, including Street No. 3, Street No. 4, and a portion of Street No. 2. Major and minor storm runoff generated from Catchment 1000 will be directed to Stormwater Management Facility No. 1.

Catchment 1100 (0.48-hectares, 0% Impervious) represents a portion of external lands including existing residential lot. Major and minor storm runoff generated from Catchment 1100 will be directed to Stormwater Management Facility No. 1.

Catchment 1200 (0.22-hectares, 50% Impervious) represents the rear yards of lots 35-42. Runoff generated from Catchment 1200 will be directed to Infiltration Gallery No. 1.

Catchment 1300 (0.70-hectares, 50% Impervious) represents the rear yards of lots 53-64. Runoff generated from Catchment 1300 will be directed to Infiltration Gallery No. 1.

Catchment 1400 (0.62-hectares, 20% Impervious) represents the park block. Runoff generated from Catchment 1400 will be directed to Infiltration Gallery No. 2. The clear stone infiltration gallery (80m L x 10m W x 0.70m D) with two (2) 300mm diameter perforated pipes which run the length of the gallery will provide approximately 192 m³ of storage. The clear stone infiltration gallery has been designed with sufficient capacity to infiltrate minor storm runoff generated by Catchment 1400. Flows exceeding the capacity of the clear stone infiltration gallery will be directed to Stormwater Management Facility No. 1.

Catchment 1500 (1.11-hectares, 50% Impervious) represents a portion of Walser Street and Street No. 1. Minor runoff generated from Catchment 1500 will be conveyed via storm sewers to Stormwater Management Facility No. 1, ultimately discharging to the existing wetland. Major runoff generated from Catchment 1500 will sheetflow uncontrolled to the existing wetland.

Quality control treatment (80% TSS removal) for runoff generated from Catchment 1000, 1100, 1400, and 1500 will be provided by three (3) oil/grit separator structures. The first oil/grit separator structure (Stormceptor EF8 or approved equivalent) will be located north of the northerly inlet to Stormwater Management Facility No. 1 (Street 3). The second oil/grit separator structure (Stormceptor STC EF6 or approved equivalent) will be located south of the northerly inlet to Stormwater Management Facility No. 1 (Street 4). The third oil/grit separator structure (Stormceptor STC EF6 or approved equivalent) will be located at the southerly inlet to Stormwater Management Facility No. 1 (Street 1).

Catchment 1600 (0.22-hectares, 50% Impervious) represents the rear yards of lots 43-45 and 50-52. Runoff generated from Catchment 1600 will be directed to Infiltration Gallery No. 1. The clear stone infiltration gallery (145m L x 3.5-5m W x 0.70m D), receiving flows from Catchments 1200, 1300, and 1600 with four to six (4 to 6) 450mm diameter perforated pipes which run the length of the gallery will provide approximately 192.1 m³ of storage. Flows exceeding the capacity of the clear stone infiltration gallery will be directed to Stormwater Management Facility No. 1.

411009
AINLEY FARM
SUBDIVISION
TOWNSHIP OF CENTRE
WELLINGTON (ELORA)



LEGEND

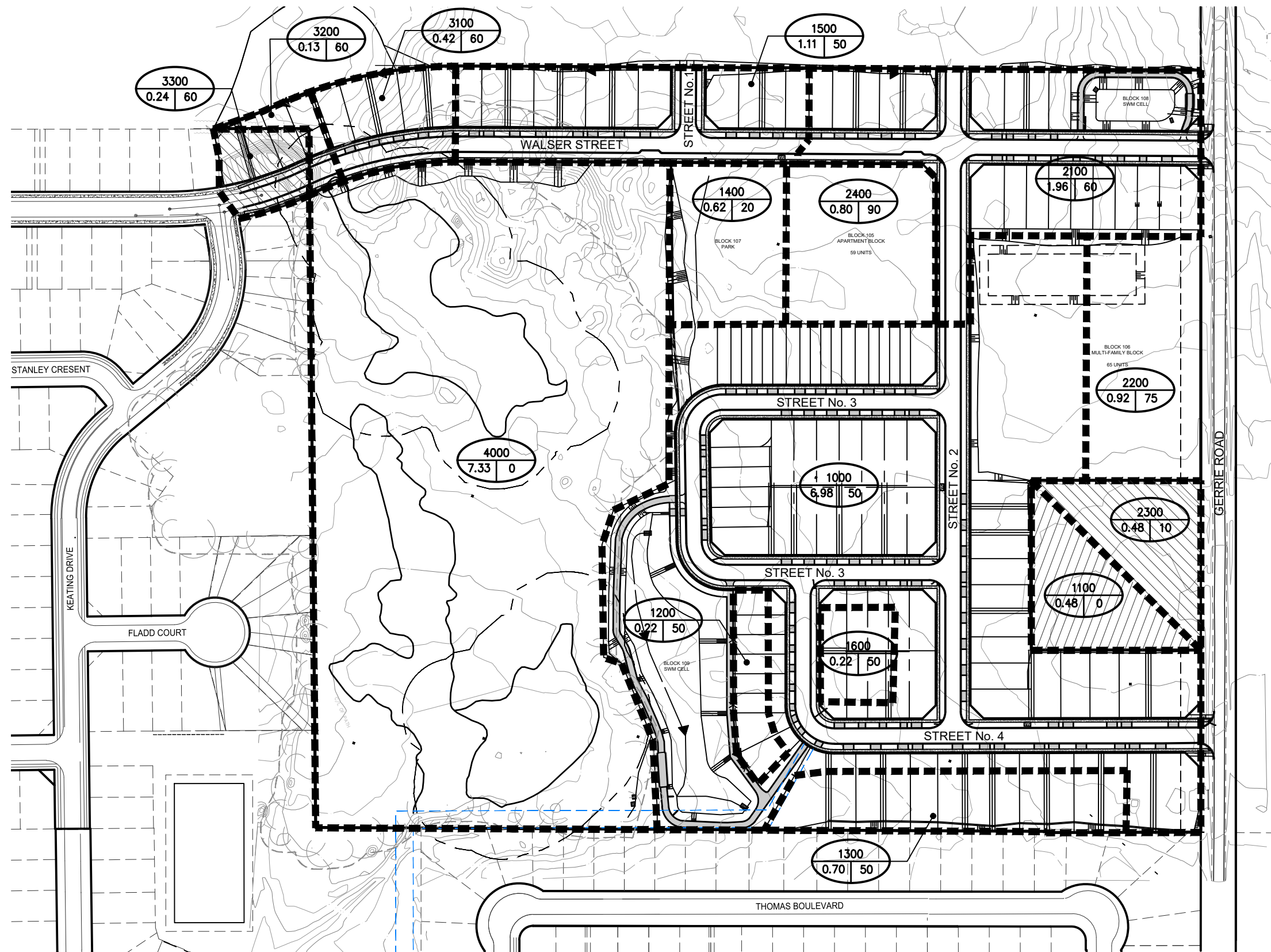
- DRAINAGE AREA BOUNDARY
- CATCHMENT NUMBER
- % IMPERVIOUS AREA IN HECTARES
- EXTERNAL LANDS

POST DEVELOPMENT
STORM DRAINAGE
AREA PLAN

FIGURE No. 4



JOB NUMBER 411009
APRIL 2023
Scale: 1:2500



Stormwater Management Facility No. 1 has been designed as a dry pond with approximately 3,030 m³ of storage. Discharging from this pond will be via a multi-stage outlet consisting of a double inlet catchbasin with a 250 mm orifice for minor storms and a double inlet catchbasin with a 350mm diameter orifice for major storms, as well as a 20 m wide overflow weir.

Catchment 2100 (1.96-hectares, 60% Impervious) represents the north portion of development, including the remainder of Street 2, and a portion of Walser Street. Major and minor storm runoff generated from Catchment 2100 will be directed to Stormwater Management Facility No. 2, ultimately discharging to a tributary of the Grand River via the existing roadside ditch along Gerrie Road.

Catchment 2200 (0.92-hectares, 75% Impervious) represents a portion of the multi-family residential block. Runoff generated from Catchment 2200 will discharge to the roadside ditch along Gerrie Road, and ultimately a tributary of the Grand River. At such time as development of Catchment 2200 proceeds, a privately owned and operated on-site quality and quantity control stormwater management facility will be required to attenuate stormwater runoff to the existing condition level, prior to discharge to the existing roadside ditch along Gerrie Road.

The privately owned and operated on-site stormwater management facility will be designed, reviewed and approved as part of the site plan approval process for the development block. The on-site stormwater management controls which may be utilized include, but are not limited to, a stormwater management facility (i.e. SWM pond), rooftop storage, parking lot ponding (to a maximum depth of 0.3m), below grade storage (i.e. clear stone storage, superpipe storage, etc.) and oil/grit separators. A preliminary stormwater management facility with approximately 2,085 m³ of storage has been modelled to provide attenuation levels for the post-development runoff generated by Catchment 2200.

Catchment 2300 (0.48-hectares, 10% Impervious) represents the remainder of the existing residential lot on Gerrie Road. Major and minor storm runoff generated from Catchment 2300 will be directed to the existing roadside ditch on Gerrie Road, ultimately discharging to a tributary of the Grand River.

Catchment 2400 (0.80-hectares, 90% Impervious) represents the apartment block. Runoff generated from Catchment 2400 will be directed to Stormwater Management Facility No. 2, ultimately discharging to a tributary of the Grand River via the existing roadside ditch along Gerrie Road.

Stormwater Management Facility No. 2 has been designed as a dry pond with approximately 1,741 m³ of storage. Discharging from this pond will be via a multi-stage outlet consisting of a 150 mm knockout for minor storms and a 300mm diameter orifice for major storms, as well as a 10 m wide overflow weir.

Quality control treatment (80% TSS removal) for runoff generated from Catchment 2100 and 2400 will be provided by one (1) oil/grit separator structure. The oil/grit separator structure (Stormceptor EF8 or approved equivalent) will be located at the inlet to Stormwater Management Facility No. 2.

Catchment 3100 (0.42-hectares, 60% Impervious) represents five (5) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3100 will be captured and attenuated via a 1350mm diameter super-pipe system which is controlled by a 120 mm diameter orifice plate prior to discharging to on-site storm sewers. The super-pipe system has approximately 75.2 m³ of storage when combined with storage generated from the storm sewer structures. Runoff from the super-pipe system will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3100 will sheetflow overland to the existing Walser Street right-of-way.

Catchment 3200 (0.13-hectares, 60% Impervious) represents five (5) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3200 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3200 will sheetflow overland to the existing Walser Street right-of-way.

Catchment 3300 (0.24-hectares, 60% Impervious) represents three (3) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3300 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3300 will discharge overland to the existing Walser Street right-of-way.

Quality and quantity control for stormwater runoff generated from Catchments 3200 and 3300 will be provided by the existing stormwater management facilities approved and constructed as part of the Villa Lora Downs North Phase II development.

Catchment 4000 (7.33 hectares, 0% impervious) represents the remainder of the site, which is a natural heritage feature consisting of a woodlot and wetland area. Runoff generated from Catchment 4000 will continue to sheetflow overland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive.

Table No. 8 lists the uncontrolled flow rate and runoff volumes generated from each catchment area shown on Figure No. 4, for the 2, 5, 10, 25, 50 and 100-year design storm events and the Regional storm.

Table No. 8: Post-Development Uncontrolled Flow Rate and Runoff Volume

	CATCHMENTS																	
	1000	1100	1200	1300	1400	1500	1600	4000	To Ex. Wetland	2100	2200	2300	2400	To Roadside Ditch	3100	3200	3300	To Walser Street
2-Year																		
Flow Rate (m³/s)	0.662	0.009	0.021	0.063	0.024	0.105	0.020	0.001	0.895	0.223	0.130	0.011	0.129	0.489	0.049	0.014	0.026	0.088
Runoff Volume (m³)	1,213.7	32.9	37.6	120.7	68.2	190.9	37.9	16.2	1,718.2	377.7	205.8	42.8	204.6	830.9	80.6	25.1	46.4	152.1
5-Year																		
Flow Rate (m³/s)	0.935	0.028	0.029	0.093	0.038	0.146	0.029	0.019	1.280	0.304	0.176	0.030	0.177	0.678	0.067	0.020	0.037	0.124
Runoff Volume (m³)	2,097.7	76.2	65.1	209.5	133.2	330.9	65.7	156.6	3,134.8	638.8	338.3	89.7	329.4	1,396.1	136.7	42.5	78.6	257.8
10-Year																		
Flow Rate (m³/s)	1.094	0.043	0.037	0.114	0.056	0.174	0.036	0.050	1.526	0.359	0.206	0.045	0.208	0.805	0.078	0.024	0.045	0.147
Runoff Volume (m³)	2,751.8	111.7	85.4	275.0	184.0	434.9	86.2	326.8	4,255.8	830.1	433.4	127.0	416.9	1,807.5	177.6	55.2	102.0	334.8
25-Year																		
Flow Rate (m³/s)	1.292	0.065	0.047	0.144	0.082	0.213	0.046	0.114	1.852	0.435	0.245	0.067	0.246	0.975	0.091	0.029	0.055	0.176
Runoff Volume (m³)	3,583.9	160.0	111.3	359.0	251.3	568.2	112.3	606.6	5,752.7	1,073.0	552.8	177.2	525.3	2,328.3	228.9	71.3	131.7	432.0
50-Year																		
Flow Rate (m³/s)	1.452	0.082	0.055	0.169	0.105	0.242	0.054	0.183	2.124	0.491	0.275	0.086	0.274	1.107	0.102	0.034	0.064	0.200
Runoff Volume (m³)	4,253.0	200.4	132.0	426.7	306.8	675.6	133.5	874.1	7,002.0	1,267.8	647.8	218.8	610.7	2,745.1	270.7	84.3	155.5	510.5
100-Year																		
Flow Rate (m³/s)	1.610	0.101	0.063	0.194	0.129	0.277	0.062	0.268	2.406	0.554	0.306	0.105	0.301	1.245	0.114	0.038	0.072	0.225
Runoff Volume (m³)	4,934.5	242.1	152.9	495.5	363.6	785.9	155.1	1,180.1	8,309.9	1,466.7	744.1	261.6	696.5	3,169.0	313.1	97.5	179.7	590.2
Regional Storm																		
Flow Rate (m³/s)	0.768	0.055	0.027	0.085	0.071	0.129	0.027	0.619	1.666	0.232	0.112	0.055	0.102	0.501	0.049	0.016	0.029	0.094
Runoff Volume (m³)	16,390.0	1031.7	490.7	1,595.8	1,379.9	2,555.4	496.2	9,470.4	33,410.0	4,559.9	2,173.8	1,044.2	1,907.3	9,685.2	981.6	301.7	553.4	1,836.7

d) Routing

Table No. 9 compares the routing results through the proposed Infiltration Gallery No. 1.

Table No. 9: Catchment 1200, 1300 & 1600 – Infiltration Gallery No. 1 Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Bottom of Stone	0.000	0.0	412.00	---	---	---
2-Year	---	---	---	0.000	104.0	412.30
Top of Stone	0.000	192.1	412.70	---	---	---
CB Lip (1)	0.000	193.0	413.23	---	---	---
5-Year	---	---	---	0.074	194.3	413.51
Regional Storm	---	---	---	0.127	195.4	413.64
10-Year	---	---	---	0.130	195.6	413.66
25-Year	---	---	---	0.213	197.2	413.79
50-Year	---	---	---	0.270	198.4	413.87
100-Year	---	---	---	0.316	199.4	413.93
CB Lip (2)	0.437	202.6	414.09	---	---	---
Overflow	0.928	214.6	414.49	---	---	---

Table No. 10 compares the routing results through the proposed Infiltration Gallery No. 2.

Table No. 10: Catchment 1400 – Infiltration Gallery No. 2 Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Bottom of Stone	0.000	0.0	413.92	---	---	---
2-Year	---	---	---	0.001	55.2	414.12
5-Year	---	---	---	0.001	123.6	414.36
10-Year	---	---	---	0.001	139.6	414.42
Top of Stone	0.001	191.8	414.62	---	---	---
Pipe Invert	0.001	191.8	414.72	---	---	---
25-Year	---	---	---	0.017	191.9	414.83
50-Year	---	---	---	0.046	192.3	415.12
Top of Grate	0.053	192.1	415.12	---	---	---
Regional Storm	---	---	---	0.070	192.9	415.13
100-Year	---	---	---	0.090	193.7	415.14
Weir	1.244	304.7	415.42	---	---	---

Table No. 11 compares the routing results through the proposed Stormwater Management Facility No. 1.

Table No. 11: Catchment 1000, 1100, 1200, 1300, 1400, 1500 (minor) & 1600 – Stormwater Management Facility No. 1 Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
CB Lip 1 Elevation	0.00	0.0	411.00	---	---	---	---
2-Year	---	---	---	0.097	726.7	411.51	3
CB Lip 2 Elevation	0.105	1,027.6	411.60	---	---	---	---
5-Year	---	---	---	0.234	1,286.5	411.66	4
10-Year	---	---	---	0.323	1,623.0	411.74	4
Weir	0.349	2,195.7	411.85	---	---	---	---
25-Year	---	---	---	0.398	2,237.9	411.86	5
50-Year	---	---	---	0.658	2,458.1	411.90	5
Regional Storm	---	---	---	0.875	2,573.8	411.92	5
100-Year	---	---	---	0.970	2,593.1	411.92	5
Top of Bank	2.018	3,030.1	412.00	---	---	---	---

Table No. 12 gives the results of the post-development condition of the existing wetland.

Table No. 12: Wetland Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.097	58.6	409.65	6.8
5-Year	---	---	---	0.237	143.1	409.67	8.0
10-Year	---	---	---	0.367	222.3	409.70	8.1
25-Year	---	---	---	0.485	293.3	409.72	8.5
50-Year	---	---	---	0.768	466.3	409.76	8.8
100-Year	---	---	---	1.083	656.4	409.79	8.8
Regional Storm	---	---	---	1.444	875.7	409.82	55.0
Overflow	18.965	15,227.7	410.75	---	---	---	---

Table No. 13 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

Table No. 13: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 1 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m³/s	Average Channel Depth m	Velocity m/s	Peak Flow m³/s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.097	0.163	0.544
5-Year	---	---	---	0.237	0.228	0.680
10-Year	---	---	---	0.367	0.269	0.759
25-Year	---	---	---	0.485	0.298	0.813
50-Year	---	---	---	0.768	0.354	0.912
100-Year	---	---	---	1.083	0.403	0.994
Regional Storm	---	---	---	1.444	0.449	1.068
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 14 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

Table No. 14: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 2 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m³/s	Average Channel Depth m	Velocity m/s	Peak Flow m³/s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.097	0.084	0.516
5-Year	---	---	---	0.236	0.140	0.699
10-Year	---	---	---	0.367	0.180	0.806
25-Year	---	---	---	0.484	0.210	0.880
50-Year	---	---	---	0.767	0.270	1.013
100-Year	---	---	---	1.078	0.325	1.120
Regional Storm	---	---	---	1.438	0.378	1.217
Top of Bank	9.246	0.950	1.966	---	---	---

Table No. 15 compares the routing results through the proposed Stormwater Management Facility No. 2.

Table No. 15: Catchment 2100, 2400 – Stormwater Management Facility No. 2
Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
150mm Knockout	0.00	0.0	410.65	---	---	---	---
2-Year	---	---	---	0.027	400.2	411.06	7
CB Lip Elevation	0.029	442.0	411.10	---	---	---	---
5-Year	---	---	---	0.121	550.4	411.20	8
10-Year	---	---	---	0.141	693.3	411.31	9
25-Year	---	---	---	0.161	911.7	411.48	12
50-Year	---	---	---	0.175	1,094.9	411.62	13
Weir	0.187	1,143.0	411.65	---	---	---	---
Regional Storm	---	---	---	0.268	1,184.5	411.68	13
100-Year	---	---	---	0.306	1,201.9	411.69	13
Top of bank	1.015	1,367.8	411.80	---	---	---	---

Table No. 16 compares the routing results through the proposed super-pipe storage system on Walser Street.

Table No. 16: Catchment 3100 – Super-Pipe Storage System
Available Stage/Storage/Discharge

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Orifice Invert	0.00	0.0	410.62	---	---	---
2-Year	---	---	---	0.022	19.0	411.24
5-Year	---	---	---	0.029	34.3	411.63
10-Year	---	---	---	0.033	45.2	411.91
25-Year	---	---	---	0.038	59.4	412.27
Regional Storm	---	---	---	0.040	67.2	412.47
50-Year	---	---	---	0.041	70.1	412.54
100-Year	---	---	---	0.050	76.2	413.47
CBMH.43 T/G Weir	0.055	77.9	413.92	---	---	---
Overflow	0.160	78.5	414.23	---	---	---

Table No. 17 summarizes the post-development flow rates from the site.

Table No. 17: Summary of Post-Development Flow Rates and Runoff Volumes from the Site

	CATCHMENTS												
	1000, 1100, 1200, 1300, 1400, 1500 (minor), 1600 (controlled)	1500 (major) (uncontrolled)	4000 (uncontrolled)	To Ex. Wetland	2100, 2400 (controlled)	2200 (controlled)	2300 (uncontrolled)	To Roadside Ditch	3100 (minor) (controlled)	3100 (major) (controlled)	3200 (uncontrolled)	3300 (uncontrolled)	To Walser Street
2-Year													
Flow Rate (m³/s)	0.097	0.000	0.001	0.097	0.027	0.008	0.011	0.043	0.022	0.000	0.014	0.026	0.060
Runoff Volume (m³)	1,437.5	0.0	16.2	1,454.3	582.2	205.8	42.8	830.9	80.6	0	25.1	46.4	152.2
5-Year													
Flow Rate (m³/s)	0.234	0.000	0.019	0.244	0.121	0.012	0.030	0.151	0.029	0.000	0.020	0.037	0.082
Runoff Volume (m³)	2,504.7	0.0	156.6	2,662.1	968.1	338.3	89.7	1,395.2	136.7	0	42.5	78.6	257.9
10-Year													
Flow Rate (m³/s)	0.323	0.028	0.050	0.372	0.141	0.014	0.045	0.190	0.033	0.011	0.024	0.045	0.107
Runoff Volume (m³)	3,287.6	10.7	326.8	3,625.2	1,247.0	433.4	127.0	1807.2	174.4	3.2	55.2	102.0	334.7
25-Year													
Flow Rate (m³/s)	0.398	0.067	0.114	0.512	0.161	0.027	0.067	0.227	0.038	0.024	0.029	0.055	0.137
Runoff Volume (m³)	4,266.0	46.1	606.6	4,920.7	1,598.3	552.8	177.2	2,329.8	216.2	12.8	71.3	131.7	431.9
50-Year													
Flow Rate (m³/s)	0.658	0.096	0.183	0.841	0.175	0.061	0.086	0.267	0.041	0.035	0.034	0.064	0.163
Runoff Volume (m³)	5,038.2	90.8	874.1	6,000.2	1,878.4	647.8	218.8	2,744.6	246.4	24.2	84.3	155.5	510.6
100-Year													
Flow Rate (m³/s)	0.970	0.131	0.268	1.235	0.306	0.099	0.105	0.454	0.050	0.047	0.038	0.072	0.190
Runoff Volume (m³)	5,817.0	145.5	1,180.1	7,146.6	2,163.2	744.1	261.6	3,165.4	273.1	40.0	97.5	179.7	590.0
Regional Storm													
Flow Rate (m³/s)	0.875	0.000	0.619	1.494	0.268	0.099	0.055	0.406	0.040	0.000	0.016	0.029	0.077
Runoff Volume (m³)	19,977.0	0.0	9,470.4	29,575.5	6,467.2	2,173.8	1,044.2	9,743.4	981.6	0.0	301.7	553.4	1,827.1

The following table compares the proposed release rates to the post-development flow rates for the site.

Table No. 18: Comparison of Release Rates and Post-Development Conditions Flow Rates

DESIGN STORM	To Ex. Wetland		To Tributary of Grand River		To Walser Street	
	Proposed Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)	Proposed Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)	Proposed Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)
2 Year	0.099	0.097	0.043	0.043	0.106	0.060
5 Year	0.353	0.244	0.153	0.151		0.082
10 Year	0.608	0.372	0.264	0.190	0.122	0.107
25 Year	1.001	0.512	0.436	0.227	0.150	0.137
50 Year	1.341	0.841	0.593	0.267	0.173	0.163
100 Year	1.721	1.235	0.765	0.454	0.196	0.190
Regional	1.667	1.494	0.841	0.406	0.080	0.077

Therefore, the post-development runoff generated from the site will be attenuated to the less than the proposed release rates to all outlets.

5.3.4 MINOR / MAJOR DRAINAGE SYSTEM

Minor storm drainage will be conveyed to the proposed stormwater management facilities and the existing storm sewers on Walser Street via storm sewers with the capacity to convey the 5-year design storm event.

The major storm runoff generated from Street 1, and a portion of Street 2, Street 3 and Street 4 will discharge to the proposed stormwater management facility located east of the existing wetland (Stormwater Management Facility No. 1), which outlets to the existing wetland, ultimately discharging to the existing storm sewers on Keating Drive.

The major storm runoff generated from the remainder of Street 2 and a portion of the Walser Street extension will discharge to the proposed stormwater management facility (Stormwater Management Facility No. 2), ultimately discharging to a tributary of the Grand River.

The major storm runoff generated from the remainder of the Walser Street extension will discharge directly to the existing Walser Street Right-of-Way, ultimately discharging to the Keating Drive Right-of-Way.

Preliminary analysis indicates that the municipal right-of-way has the capacity to convey the runoff from a major design storm event.

The major design storm drainage patterns expected for the Ainley Farm Subdivision are shown on Figure 5.

411009
AINLEY FARM
SUBDIVISION
TOWNSHIP OF CENTRE
WELLINGTON (ELORA)



LEGEND



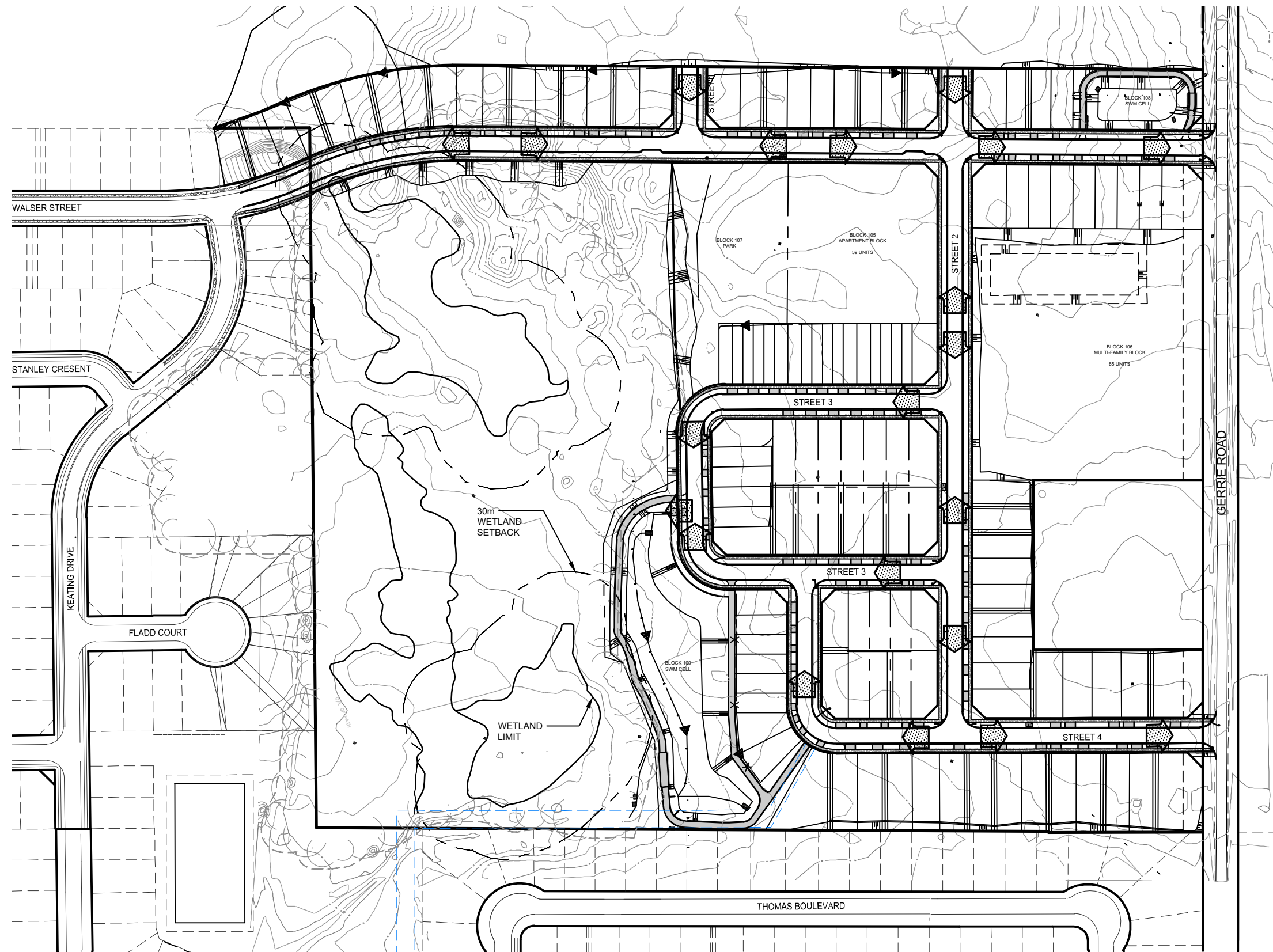
MAJOR OVERLAND
FLOW

MAJOR STORM
DRAINAGE PATTERN
PLAN

FIGURE No. 5



JOB NUMBER 411009
APRIL 2023
Scale: 1:2500



5.4 WATER BUDGET

The average annual precipitation for the site is estimated to be 945.9 mm. This amount is based on precipitation data recorded at the Fergus Shand Dam meteorological station for the period from 1981 to 2010.

From the Preliminary Geotechnical Investigation (CMT Engineering Inc., March 26, 2006), the surficial deposits across the majority of the site are described as native silt tills, with some sandy silt tills. As there are no areas of consistent sandy soils across the site, the characteristics of the silt tills will be used to develop the water budget analysis across the site.

The potential for evapotranspiration for this area is estimated to be 557.8 mm for the pervious surfaces. Therefore, 388.1 mm remain available for infiltration and runoff from the silt till.

Per Table 3.1 of the Stormwater Management Planning and Design Manual (Ministry of Environment, dated 2003), a silt till, which acts similar to clay (hence the low conductivity values), in flat cultivated land has an approximate infiltration rate of 87 mm/yr. Therefore, the runoff is estimated to be 301.1 mm/yr.

Based on the annual infiltration rates, the existing annual average groundwater recharge occurring within the 21.42-hectare site, and 1.20 hectares of external areas discharging to the site, is estimated to be 12,674 m³. Under post-development conditions, the annual average groundwater recharge occurring on-site and within the external areas naturally is estimated to be 5,473 m³. The additional annual recharge that will occur on-site via the two (2) proposed infiltration galleries is estimated to be 6,502 m³, resulting in a total post-development annual recharge rate of 11,974 m³.

Under existing conditions, the annual average runoff from the site and external areas is estimated to be 68,109 m³. As a result of the proposed development the impervious area (rooftop and paved surfaces) of the site increases, the annual potential evapotranspiration for impervious surfaces decreases to 200 mm and the runoff from the site increases. The runoff from the site and external areas under post-development conditions is estimated to be 97,828 m³ per year.

The estimated existing and post-development recharge and runoff volumes for the Ainley Farm Subdivision are detailed in Table No. 19. The estimations take into account the surficial geology, which is comprised mainly of glacial tills. The net recharge values are for the uppermost overburden aquifer. The water budget analysis has been included in Appendix D.

Table No. 19: Summary of Recharge and Runoff Volume

	Existing Condition	Post-Development Condition	Percent Change
Total Estimated Recharge	12,674 m ³	11,974 m ³	-5.5%
Total Estimated Runoff	68,109 m ³	98,828 m ³	+45.1%

The opportunities for further recharge are limited by the high groundwater elevations across the site, as noted in CMT's groundwater monitoring across the site from 2006 to 2016.

6.0 SEDIMENT AND EROSION CONTROL PLAN

A silt fence will be installed along the property boundary. The silt fence will serve to minimize the opportunity for water borne sediments to be transported from the site to the adjacent properties.

Temporary straw bale check dams will be installed in rear yard swales after the initial grading has been completed to slow the flow rates and promote the settlement of water borne sediments before they reach the silt fences and stormwater management facilities.

Upon completion of the grading, any area not subject to active construction within 30 days will be top soiled and seeded as per OPSS 572.

Once catch basins have been installed, the grates will be wrapped in filter cloth. This feature will be maintained until all building and landscaping has been completed.

Inspection and maintenance of all silt fencing and sediment and erosion controls will start after installation is complete. These features will be inspected on a weekly basis or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the controls found to need repair.

Once construction and landscaping within the limits of the subdivision has been substantially completed (75% house building construction is complete), the silt fence will be removed, any accumulated sediment will be collected, and the area will be restored.

After construction of the subdivision, erosion and sediment transport will be minimal.

7.0 MAINTENANCE PLAN

A two-phase maintenance plan is recommended. Phase I will address the short-term more intensive maintenance necessary during and immediately after construction. Once all landscaping has been completed, maintenance will shift to Phase II.

As outlined in the section on Sediment and Erosion Control, Phase I will include weekly inspection of all sediment and erosion control devices plus "as needed" inspection after significant rainfall, with the repair of any damaged works and collection of captured sediment.

Phase II will be the maintenance carried out by the Township of Centre Wellington after all construction has been completed. This work will involve a yearly visual inspection of the stormwater management facilities and catch basins to determine the amount of sediment accumulation. Sediment should be removed as required and the recommended vegetation replanted.

8.0 CONCLUSIONS

From the foregoing analysis, the following conclusions are drawn:

- Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200 mm diameter watermain along the Walser Street extension, Street No. 2 and a portion of Street No. 1. A 150 mm diameter watermain will be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.
- Sanitary service for the proposed lots along the Walser Street extension and a portion of Street No. 2 will be provided by the extension of a 200 mm diameter sanitary sewer from the existing 200 mm diameter sanitary sewer on Walser Street. Sanitary service for the remainder of the site will be provided by the extension of a 200 mm diameter sanitary sewer on easement from the existing 200 mm diameter sanitary sewer on Keating Drive.
- Storm sewers will be designed to convey the 5-year design storm event and will discharge to the two (2) stormwater management facilities and the existing storm sewer on Walser Street.
- Major storm runoff will be conveyed within the limits of the street right-of-ways to the two (2) stormwater management facilities and the existing Walser Street right-of-way.
- As per the Township of Centre Wellington municipal standards, foundation drainage will be collected in sump pits in each residential unit and pumped to the storm sewer system located within the municipal right-of-way.
- Quantity control for runoff generated from the development will be provided by two (2) stormwater management facilities,
- Quality control for runoff generated from the development will be provided by four (4) oil/grit separators (Stormceptor or approved equivalent).
- The post-development runoff generated from the site will be attenuated to the less than the proposed release rates to all outlets.
- Infiltration rates under post-development conditions are 5.5% less than existing conditions with two (2) infiltration galleries to provide additional recharge. The opportunities for further recharge are limited by the high groundwater elevations across the site, as noted in CMT's groundwater monitoring across the site from 2006 to 2016.
- During the construction phase, the erosion control measures will minimize the transport of sediment off-site during the construction period.

All of which is respectfully submitted.

GM BLUEPLAN ENGINEERING LIMITED

Per:



Patrick Grier, P.Eng.

PG/





APPENDIX A
PRELIMINARY GEOTECHNICAL INVESTIGATION
CMT ENGINEERING INC.
MARCH 29, 2006

PRELIMINARY GEOTECHNICAL INVESTIGATION

**AINLEY SUBDIVISION
TOWNSHIP OF CENTRE WELLINGTON
VILLAGE OF ELORA, ONTARIO**

CMT Project 06-004

Prepared For:

Gamsby and Mannerow Limited

March 29, 2006





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March 29, 2006

06-004.R01

Gamsby and Mannerow Limited
255 Woodlawn Road West, Suite 210
Guelph, Ontario
N1H 8J1

Attention: Mr. Glenn Anderson, C.E.T.

Dear Sir:

**Re: Preliminary Geotechnical Investigation
Ainley Subdivision
Township of Centre Wellington
Village of Elora, Ontario**

As requested, CMT Engineering Inc. conducted a subsoil investigation at the above-referenced site, and we are pleased to present the enclosed report.

We trust that this information meets your present requirements and we thank you for this opportunity to have been of service. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

Robert Koopmans, P.Eng.

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Drawing 1 - Site Plan Showing Borehole Locations

Appendix A - Borehole Logs 101 to 108

Appendix B - Cross-Sections

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Appendix D - Laboratory Proctor Test

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1.0 INTRODUCTION

The services of CMT Engineering Inc. were retained by Mr. Glenn Anderson of Gamsby and Mannerow Limited to carry out a subsurface investigation for the proposed Ainley Subdivision in the Township of Centre Wellington (Village of Elora).

It is our understanding that single-family and multi-family residences are proposed to be built on the eastern two-thirds of the property. The western one-third of the property is proposed for open space and may contain a storm water management pond.

The purpose of the investigation was to determine the subsurface soil profile, the water levels in the boreholes and provide recommendations with respect to site grading, bearing capacity for house foundations, trench excavations, bedding and backfilling for service pipes, site dewatering, road construction, pavement design recommendations and soil hydraulic conductivity for storm water management design.

2.0 SITE CONDITIONS

The geotechnical investigation was conducted on Part Lot 18, Concession 12 of the Township of Centre Wellington. In general, the eastern two-thirds of the property is currently farm land, while the western one-third is treed. The site topography undulates slightly and the ground surface elevation drops towards the southwest corner of the property.

3.0 FIELD AND LABORATORY PROCEDURES

On January 25, 2006, a track-mount CME 55 drillrig operated by Aardvark Drilling Inc. was used to drill eight (8) boreholes (referenced as Boreholes 101 to 108) to depths of between 3.5 m (11.5 ft) and 5.0 m (16.4 ft) below the existing ground surface elevation. Standard penetration tests were conducted at 0.76 m (2.5 ft) intervals to depths of 3.0 m (10.0 ft) and at 1.5 m (5.0 ft) intervals below 3.0 m (10.0 ft) in all boreholes. Monitoring wells were installed in all eight boreholes to determine the presence and depth of the groundwater table.

Technical staff from CMT Engineering Inc. observed the drilling operation and collected and logged the recovered soil samples. Soil samples taken from Borehole 102 (3.05 to 3.51 m), Borehole 103 (4.57 to 5.03 m), Borehole 105 (2.29 to 2.74 m) and Borehole 107 (2.29 to 2.74 m) were placed in marked sample bags for grain size analyses (refer to Appendix C for laboratory test results). A bulk sample from Borehole 103 (1.5 to 2.0 m) was submitted for laboratory Proctor testing (refer to Appendix D for the laboratory test results). A small portion of each sample was placed in a sealed marked jar for moisture content determinations.

Gamsby and Mannerow Limited surveyed the ground surface elevations for all boreholes, as well as the tops of the monitoring wells.

Drawing 1 shows the site plan with all of the borehole locations.

4.0 SUBSOIL CONDITIONS

The soil conditions at the borehole locations are summarized briefly below, while a more detailed stratigraphic description is provided in the borehole logs in Appendix A. Cross-section profiles through Boreholes 101-103-102-104 and Boreholes 105-106-107-108 are provided in Appendix B.

4.1 Topsoil

Dark brown silt topsoil was found at the top of all eight boreholes. The topsoil was frozen at the time of the investigation. The thickness of the topsoil ranged from 30 mm to 60 mm (average 42 mm).

4.2 Organic Silt

Organic silt was found underlying the topsoil in Borehole 103. The organic silt was saturated, loose and brown with some sand and occasional topsoil nodules. The moisture content of the organic silt was 83.5% and the N-count was 8 blows per 0.30 m.

4.3 Silt or Sandy Silt

Silt was found underlying the sandy silt in Borehole 105. The silt was very moist, loose and brown with some clay, trace sand and trace gravel. The moisture content of the silt was 18.5% and the N-count was 8 blows per 0.30 m.

Sandy silt was found underlying the topsoil in Borehole 102. In general, the sandy silt was moist, compact and brown with a trace of clay and a trace of gravel. The moisture content was 10.4% and the N-count was 12 blows per 0.30 m.

4.4 Silt Till or Sandy Silt Till

Glacial till comprising silt or sandy silt was found in all boreholes (101 to 108). In general, the silt till was moist, compact to very dense and brown with trace to some sand,

gravel and clay. The moisture content ranged from 6.8% to 20.4% (average 11.7%) and the N-count ranged from 14 to 100 blows per 0.30 m (average 37 blows per 0.30 m). The sandy silt till was generally moist, compact to very dense and brown with trace to some sand, gravel and clay. The moisture content ranged from 7.6% to 26.7% (average 13.4%) and the N-count ranged from 7 to 100 blows per 0.30 m (average 36 blows per 0.30 m).

4.5 Sand or Silty Sand

Sand was found in Boreholes 101, 102, 105 and 107. In general, the sand was very moist to wet, compact and brown with trace silt and/or trace gravel. The moisture content ranged from 14.3% to 20.0% (average 17.2%) and the N-count ranged from 7 to 22 blows per 0.30 m (average 15 blows per 0.30 m).

Silty sand was found in Boreholes 101, 103, 104, 105, 106 and 108. In general, the silty sand was wet to saturated, compact and brown with occasional trace gravel. The moisture content ranged from 10.5% to 27.7% (average 20.8%) and the N-count ranged from 1 to 25 blows per 0.30 m (average 10 blows per 0.30 m).

4.6 Clayey Silt

A localized layer of clayey silt was found in Borehole 107. The clayey silt was moist, compact and brown with trace sand and trace gravel. The moisture content was 13.6% and the N-count was 11 blows per 0.30 m.

4.7 Groundwater Conditions

Monitoring wells were installed in all boreholes. The monitoring wells were constructed utilizing 50 mm Schedule 40 PVC pipe with a 3 m long slot 10 screen surrounded by the sand filter comprising #3 industrial sand. The boreholes were backfilled with 3/8" bentonite holeplug from the top of the sand filter to the existing ground surface. For protection and security purposes, locking steel protective covers were installed on all of the monitoring wells.

A copy of the well record has been included in Appendix E. It is a requirement of Regulation 903 of the Ontario Water Resources Act that the monitoring well installations be abandoned within 180 days after they are no longer in use.

At the time of writing, the static water levels in the monitoring wells had been read on February 8, 2006, February 20, 2006, March 9, 2006, March 25, 2006 and March 29, 2006. A summary showing the ground surface, borehole bottom and water level elevations for Boreholes 101 to 108 are provided below:

Borehole No.	Ground Surface Elevation (m)	Elevation of Borehole Bottom (m)	Elevation of Water Table (m)				
			(F) - Frozen				
			Feb 8, 2006	Feb 20, 2006	Mar 9, 2006	Mar 25, 2006	Mar 29, 2006
101	413.64	408.64	413.07	413.11	412.83	412.96	--
102	414.37	409.37	411.57	411.96	411.91	412.48	--
103	414.89	409.89	412.65	412.98	412.88	412.77	--
104	410.93	407.43	410.36	410.60	410.17	410.66	--
105	414.05	409.28	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68
106	410.91	405.94	410.67	410.86 (F)	410.93 (F)	410.75	--
107	409.58	406.08	409.43	409.06 (F)	409.12 (F)	409.41	--
108	410.32	406.82	409.06	409.21	408.82	409.01	--

Due to the close proximity of the groundwater to the ground surface, some of the monitoring wells were frozen at the time of the water level readings.

The groundwater levels will be measured on a monthly basis in an effort to try and establish extreme (high and low) groundwater elevations.

5.0 DISCUSSION

It is our understanding that the property owner is proposing to develop a residential subdivision on the property investigated. The subdivision will be fully serviced with municipal sewers and water supply. A storm water management facility is proposed to be constructed in the western portion of the site.

5.1 Site Grading

Prior to the commencement of any site grading, all topsoil and organic silt soils (Borehole 103) must be removed from the proposed building envelopes (including extended zone of influence areas), road allowance and driveways.

Due to the high water table and isolated wet surface conditions, it may be necessary to utilize an excavator during topsoil stripping to minimize over-excavation as a result of soil disturbance from heavy construction traffic.

At this time, the proposed founding elevations for the residences are not available. However, it would appear that some cut and fill operations will be required to level the building site.

Prior to any placement of structural fill, the subgrade for the building envelope must be prepared large enough to accommodate a 1:1 slope commencing at a distance of 1.0 m beyond the outside edge of the proposed foundation down to approved native founding soils.

Soils approved for use as structural fill must be placed in loose lifts not exceeding 0.3 m (1 ft) in depth for granular soils and 0.2 m (8") in depth for fine grained (silt and clay) soils and compacted using adequate heavy vibratory padfoot compaction equipment to a minimum of 98% standard Proctor maximum dry density (SPMDD). The approved structural fill materials must be free of frozen materials, organics or other deleterious materials and must not contain particles exceeding 150 mm (6") in diameter. The soils must be at moisture contents suitable to achieve the specified compaction.

A laboratory Proctor moisture-density test was performed on a bulk sample of the silt till from Borehole 103 (depth 1.5 to 2.0 m). The results of the laboratory Proctor test indicate that the optimum moisture content of the sample is 8.3%. Since the insitu moisture contents of the split spoon sample of silt till ranged from 6.8% to 20.4% (average 11.7%), it should be anticipated that the majority of the silt till will require air-drying in order to achieve the specified compaction during construction.

The fine grained soils encountered in the geotechnical investigation are highly susceptible to strength losses if subjected to frequent disturbance by construction traffic. Therefore, it is recommended to minimize construction traffic on subgrade soils.

It would be recommended that the site grading and underground service installation be undertaken during drier warm weather conditions in order to minimize dewatering operations, eliminate frost problems and most importantly improve the placement and compaction of structural fill and backfill materials. Proper compaction and backfilling operations are imperative in order to provide adequate support for structures, service pipes, driveway and roadways.

If site grading and site servicing is undertaken during cold or wet weather conditions, projected overall costs would be anticipated to be higher and the project would be expected to take longer to complete.

5.2 Site Dewatering

Based on this geotechnical investigation and similar high water tables encountered during the construction of the neighbouring Ville Lora Downs Subdivision, water concerns should be anticipated for this project. Static water levels measured in the monitoring wells suggest that perched groundwater can be expected at the locations of Boreholes 101, 102, 103, 105 and 106 which were advanced within the proposed residential development area. The water appears to be surface water that has perched on top of the relatively impermeable sandy silt till, sandy silt and silt till soils. Furthermore, artesian conditions can also be expected at the locations of Boreholes 101, 102 and 103. The artesian water appears to be located between the upper sandy silt till and lower silt till in Borehole 101, between the upper and lower silt tills in Borehole 102 and below the silt till layer in Borehole 103.

Provisions for site dewatering should be part of the site development and construction process. Normally, it would be recommended that well points be installed in order to dewater the site so that site services and residential foundations could be installed. However, based on past experience, the installation of a well point dewatering system by qualified contractors can be very expensive and not necessarily guaranteed. It is probably most cost-effective to install a series of inverted drainage pipes in advance of the service (sanitary, storm and water) trench excavations and also at the locations of the manholes. Water pumps should be utilized to pump water from the inverted pipes on a continuous basis in order to keep the water table drawn down below the excavation level. Temporary drainage trenches should be constructed to remove the site water to a storm water retention pond (or reasonable alternative). The removal of considerable amounts of fine soil particles from the pumping operation can be anticipated. As such, the drainage trenches, storm water pond, pumps and hoses will most likely require regular cleanout. It might be cost-effective in regard to road construction and house construction to investigate the possibility of installing a permanent deep drainage system to lower the water table in the immediate area. Caution would be necessary with this option, since it could affect wells and building structures on adjacent properties.

The dewatering conditions may improve if work is conducted during the drier summer months as well as following the installation of the services.

5.3 Excavations

The anticipated sanitary, storm and water pipe invert elevations are all expected to be well below the water table and therefore site dewatering will be required (see Section 5.2 above). Based on observations from the neighbouring Ville Lora Downs Subdivision, the water levels in the summer are generally lower and therefore dewatering requirements

may be less. However, the anticipated effects of the artesian water conditions are still expected to be of concern.

All excavations must be carried out in accordance with Ontario Regulation 213/91 (Reg 213/91) of the Occupational Health and Safety Act and Regulations for Construction Projects.

Type 2 Soils: The native glacial till soils would be classified as Type 2 soils under Reg 213/91 and must be sloped to within 1.2 m of the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical. Where excavations expose glacial till soils underlain by wet sand or silt soils, the recommendations for Type 4 soils below must be adhered to.

Type 3 Soils: The native sand and silt soils in an unsaturated condition (above the water table) would be classified as Type 3 soils under Reg 213/91 and must be sloped from the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical.

Type 4 Soils: All native sand or silt soils in a saturated condition (below the water table) would be classified as Type 4 soils under Reg 213/91. Excavations that expose the Type 2 and Type 3 soils noted above but are underlain by saturated sand or silt soils must be treated as Type 4 soils as well. Type 4 soils must be sloped at a minimum gradient of 3 horizontal to 1 vertical. The loose wet condition of the Type 4 soils makes them very susceptible to sloughing and slope failure during excavation.

If it is not practical to excavate according to the above requirements, then a trench box system (designed in accordance with the Ontario Health and Safety Act Regulations) may be utilized.

It should be noted that some of the native glacial till soils become very dense with depth (N-values in excess of 100 blows per 0.30 m) and may prove difficult to excavate with conventional excavating equipment. It is also imperative that when the very dense soils are utilized for backfilling of service trenches, the material must be broken down (pulverized) to minimize voids and reduce the potential for settlement.

5.4 Service Pipe Bedding

The native soils are generally considered to be suitable for indirect support of the proposed service pipes. Where water inflow is a concern and the soil conditions are not suitable to support the pipe, then 80 mm to 120 mm (3" to 5") river stone (or equivalent) with a 150 mm (6") layer of 19 mm clear stone should be used to create an adequate supporting base for the pipe.

Pipe embedment and backfill for flexible pipes should be undertaken in accordance with OPSD-802.010. Pipe embedment, cover and backfill for rigid pipes should be undertaken in accordance with OPSD-802.030 or OPSD-802.031. Trenching, backfilling and compaction with respect to storm sewer pipe installations should comply with OPSS 514.

Flexible Pipes: The pipe bedding should be shaped to receive the bottom of the pipe. If necessary, pipe culvert frost treatment should be undertaken in accordance with OPSD-803.030 and OPSD-803.031. The trench excavations should be symmetrical with respect to the centreline of the pipe. The granular material placed under the haunches of the pipe must be compacted to 95% SPMDD prior to the continued placement and compaction of the embedment material. The homogeneous granular material used for embedment should be placed and compacted uniformly around the pipe. Should wet conditions be encountered at the base of the trench, then the pipe should consist of 19 mm clear stone (meeting OPS Specifications). Normally, it would be advisable to wrap the clear stone with geotextile to prevent fine soils from entering the clear stone and thereby creating voids around the pipe. In wet conditions, this is not possible to do and generally not necessary since most of the void spaces are quickly filled with fine soils as water (with suspended fine soils) rapidly enters the excavation. It is imperative that the newly installed pipe be backfilled as soon as possible in order to prevent the potential for pipe uplift. This can occur due to buoyancy, as water enters the excavation. It is also advisable to check the elevation of the installed pipe at regular intervals to ensure that uplift has not occurred. Protection against heavy construction equipment should be undertaken in accordance with OPSD-808.010.

Rigid Pipes: In general, the pipe installation recommendations for rigid pipes are the same as those for flexible pipes except that the minimum depth of bedding below a rigid pipe should be $0.15 D$ (where D is the pipe diameter). In no case should this dimension be less than 150 mm or greater than 300 mm.

5.5 Trench Backfill

Native backfill material can be used to fill the trench from 12" (30 cm) above the pipe to the subgrade elevation provided that the material is free of organics, not frozen and is not overly wet (above the optimum moisture).

Based on the existing water table, the moisture contents determined from soil samples that were taken during the geotechnical investigation, and the laboratory Proctor test (see Appendix C), it can be assumed that most soils will be too wet to enable proper compaction. As such, these soils should be allowed to drain and air-dry as long as possible before backfilling.

If wet or frozen soils are used for backfill purposes, proper compaction of the backfill will not be possible and settlement of the trenches can be expected. Site assessments will be required to determine what options can be undertaken to construct a suitable road base. These options may include subexcavating and increasing the thickness of the granular subbase, the possible use of high strength geotextiles, or a combination of both.

5.6 Sensitivity of Subsoils

The silty nature of many of the soils encountered in the boreholes can make them highly susceptible to strength losses and will prove difficult to place and compact if they become overly wet as a result of inclement weather or water seepage. If the soils become overly wet and disturbed, they may become unsuitable for reuse and require subexcavation. As such, the following is recommended:

- provide proper measures for adequate drainage during construction
- use a smooth-lipped bucket while excavating to the subgrade elevation to reduce disturbance
- minimize construction traffic traveling over the subgrade soils

5.7 Road Construction and Pavement Design

In order to achieve a suitable subgrade for the construction of the pavement structure, the following recommendations are provided:

- a) If necessary, maintain the site dewatering system during preparation of the road subgrade. Once the road subgrade is completed, the drainage pipes should be removed or cut off at the subgrade elevation and infilled with lean concrete or a bentonite slurry.
- b) The design subgrade for the road should be proof-rolled using heavy rubber-tire equipment, such as a grader. Compactive effort should be applied and compaction tests should be undertaken. Areas requiring fill to achieve the subgrade elevation should be treated as indicated above prior to placement of any additional fill. The subgrade should be evaluated to determine if subexcavation and additional Granular 'B' will be required or if the installation of a reinforcing geotextile will be necessary.
- c) The road subgrade should be cut to grade using a smooth-lipped bucket. The subgrade should be graded smooth (with no depressions) and sloped at a minimum of 2%. Construction traffic should not be allowed onto the prepared road subgrade. Construction traffic should travel only on the Granular 'B' subbase. It may be necessary to temporarily

increase the thickness of the Granular 'B' during road construction to accommodate the truck traffic.

d) It is recommended that 100 mm diameter perforated subdrains fitted with a filter sock be installed along each curb line to collect and redirect water beneath the pavement surface. It is suggested that the subdrains be installed in a 0.3 m (1 ft) by 0.3 m (1 ft) trench and placed approximately 50 mm (2") from the trench bottom. In drier conditions, the perforated subdrain with a factory-installed filter sock can be installed in Granular 'A' bedding. In wet conditions, 19 mm clear stone wrapped completely in non-woven geotextile (such as Terrafix 270R or equivalent) is recommended. Rapid drainage of the pavement structure is critical to ensure long-term performance of the road.

Based on the anticipated loading and considering that the subsoils contain frost-susceptible soils, the following pavement design is recommended for the proposed roads:

Material	Recommended Thickness
Asphaltic Concrete	HL3 - 40 mm (1.5") HL4 or HL8 - 50 mm (2.0")
Granular 'A' Base	150 mm (6.0")
Granular 'B' Subbase	450 mm (18.0")

The granular subbase materials should be compacted to 100% SPMDD. Asphaltic concrete should be supplied, placed and compacted to 97% Marshall bulk relative density in accordance with OPSS 1150 and OPSS 310.

The pavement should be designed to ensure that water will not pond on the pavement surface. If the surface asphalt is not placed in a reasonable time following the placement of the binder asphalt, it is recommended that the catch basin lids be lowered or apertures provided to allow the surface water to drain rather than accumulating around the catch basins.

5.8 Bearing Capacity / Settlement

The proposed residential buildings may be supported on conventional spread and pier footings provided they are founded on undisturbed native soils at or below the elevations listed in the following table or structural fill prepared as detailed in Section 5.1 of this report:

Borehole No.	Existing Ground Surface Elevation (m)	Highest Recommended Footing Elevation (m)	Soil Type
101	413.64	413.01	sandy silt till
102	414.37	412.70	silt till
103	414.89	413.59	silt till
105	414.05	411.65	sandy silt till
106	410.91	408.51	silt till

It is ideally recommended that foundations be constructed above the water table. The native founding soils and structural fill in a drained condition would be considered suitable to support foundations designed with a safe net allowable bearing capacity of 150 kPa. It is anticipated that the water table may be within one footing width below the founding elevation. Therefore, a safe net allowable bearing capacity of 75 kPa should be used for design purposes.

With respect to the bearing capacities as determined above, total and differential settlements are estimated to be within the generally acceptable limits of 25 mm (1") and 19 mm (3/4") respectively.

A minimum of 1.2 m (4 ft) of soil cover above the footing grade must be provided for frost protection.

5.9 Residential Drainage Considerations

If high water conditions continue to exist during the construction of the residential foundations, and the foundations are constructed near or below the water table, then the following will be required:

- a granular drainage layer and sump pump will be required as per Section 9.14.4 of the current Ontario Building Code

- slab-on-grades constructed where groundwater levels may cause hydrostatic pressure must be designed to resist such pressures
- slab-on-grade and exterior walls must be waterproofed

If foundation construction occurs above the high water table, then conventional construction methods can be utilized.

5.10 Potential Storm Water Management Facility

Boreholes 104, 107 and 108 were all drilled in the open space area (west side of property) where a storm water management facility is proposed. In general, Borehole 104 has silt till underlain by silty sand. It would appear that artesian conditions are present in the silty sand layer below the more impermeable silt till layer. Based on the monitoring well readings, the water level fluctuates to just below the ground surface elevation.

In general, Borehole 107 has sand underlain by clayey silt and lower sandy silt till. Artesian conditions may be present in the sandy silt till below the more impermeable clayey silt layer. Based on the monitoring well readings, the water level was just below the ground surface elevation. The upper sand layer has a high moisture content due to the infiltration of surface water, which is in turn impeded by the lower clayey silt layer.

In general, Borehole 108 has silty sand underlain by sandy silt till. Artesian conditions may be present in the lower portion of the sandy silt layer below the more impermeable silty sand layer (higher density, lower moisture). Based on the monitoring well readings, the water level has fluctuated to within approximately 1.0 m of the ground surface elevation.

Based on the results of the geotechnical investigation, it can be concluded that the soil and groundwater conditions in the area of Boreholes 104, 107 and 108 are unsuitable for an inground storm water management facility.

6.0 SITE INSPECTIONS

Site grading, dewatering, trench excavations, backfilling and compaction of the service pipes should be supervised by qualified geotechnical personnel to ensure that a suitable subbase is prepared, proper backfill materials are used and that the specified compaction is achieved.

The construction of the pavement structure should also be supervised by qualified personnel to ensure that suitable materials are used and that the specified compaction is achieved. It is also

recommended that the residential foundation excavations be examined to ensure that the bearing capacity of the soil is suitable to support the structures.

CMT Engineering Inc. would be pleased to provide inspection, testing and consulting services for this project.

7.0 LIMITATIONS OF THE INVESTIGATION

This investigation was conducted to determine the subsurface conditions for this project and the comments are based on the information gathered at the borehole locations only. It is therefore assumed that the borehole information is representative of the subsoil conditions across the site. Should any conditions at the site be encountered which differ from those found at the borehole locations, we request that we be notified immediately.

This report is intended solely for the client named. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties.

We trust that this report meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Respectfully submitted,

Robert Koopmans, P.Eng.
Consulting Engineer



Tim Salter, C.E.T.



ks

Base plan provided by:



CADASTRAL AND SURVEYING LIMITED
CONSULTING ENGINEERS 20011 INDUSTRIAL
CRESCENT, UNIT 1
SCARBOROUGH, ONTARIO M1V 4Y6



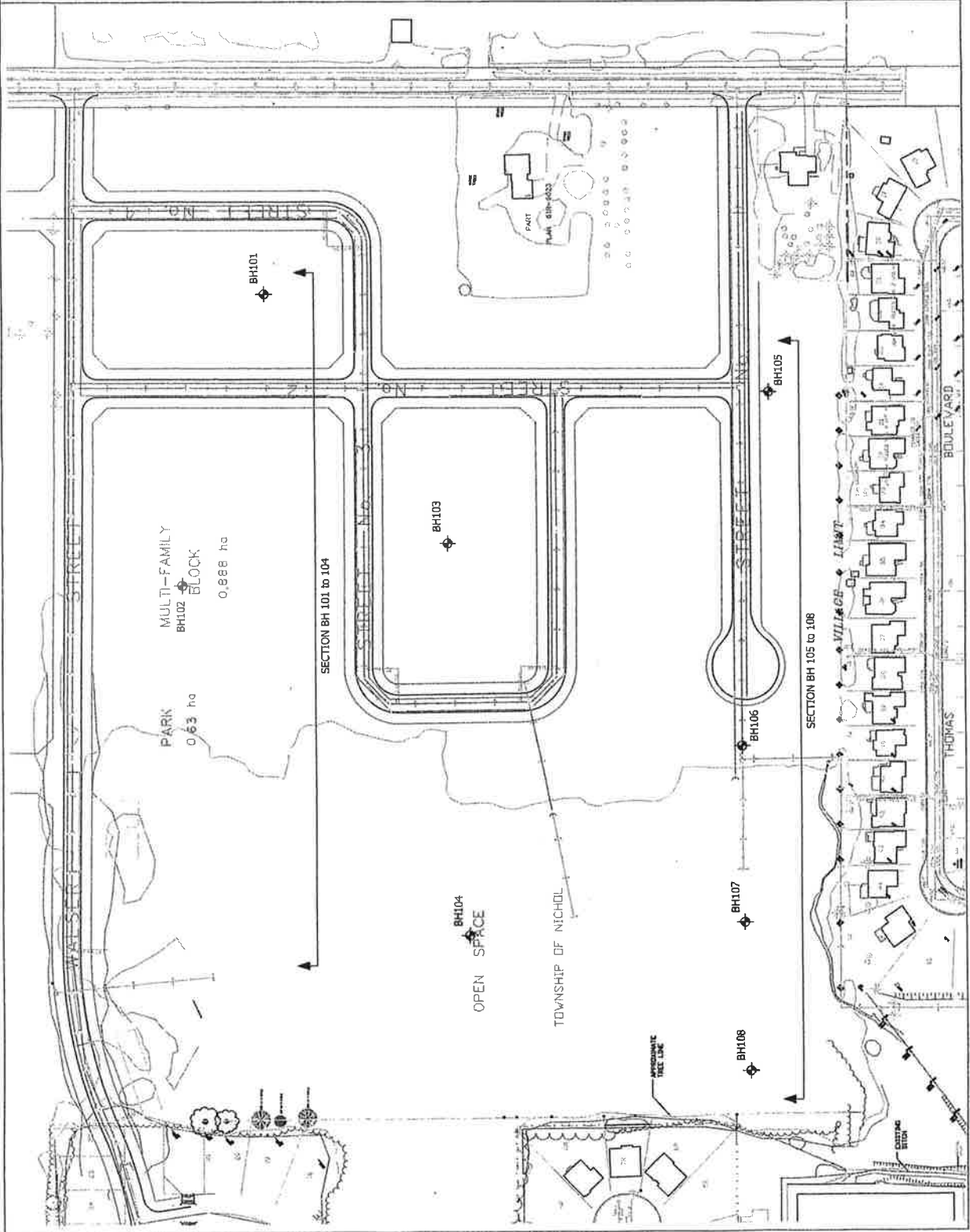
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Fax: 519-699-4664
www.cmtinc.net

BOREHOLE LOCATION PLAN

Ainley Subdivision
Township of Centre
Wellington
Village of Elora, ON

Project: 06-004	Drawing: 2
Date: Mar. 2006	Sheet: 1
Scale: 1:2000	



APPENDIX A

BOREHOLE LOGS Boreholes 101 to 108

BOREHOLE 101

Page 1 of 1

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 413.64m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 413.64			
0					Topsoil 0.00			
1					Dark brown silt, frozen 413.31			
2					Sandy Silt Till 0.33			
3	SS		1		Compact brown sandy silt till, some clay, some gravel, moist		9.7	22
4								
5							20.7	20
6	SS		2		Sand 411.84			
7					Compact brown sand, very moist to wet 411.54			
8					Silty Sand 2.10			
9	SS		3		Compact brown silty sand, wet		24.9	14
10								
11	SS		4		spoon sank through silty sand under weight of rods		27.7	1
12								
13					Sand 409.64			
14					Dense brown sand, trace silt, wet 4.00			
15							20.4	40
16	SS		5		Silt Till 408.94			
17					Dense brown silt till, some sand, gravel and clay, moist 4.70			
18					408.64			
19					5.00			
					End of Borehole			

Well Installation
 Feb 8/06
 413.07m
 Bentonite Seal
 Stick up well with protective metal casing
 #3 Sand Filter
 50mm schedule 40 PVC pipe with slot 10 screen

Please refer to section 4.7 of this report for additional water levels

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BOREHOLE 102

Page 1 of 1

Date Drilled: Jan. 25, 2006

Rig: CME 55

Contractor: Aardvark

Drilling Method: HSA

Elevation: 414.37m

Logged by: CD

Project No.: 06-004

Project: Ainley Subdivision

Township of Centre Wellington

Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl 10 20 30 40	Pocket Penetrometer kPa 100 300 SPT (N) Blows/0.3 m 10 30 50 70 90
0					Ground Surface (m) 414.37			
0.00					Topsoil			
0.30					Dark brown silt, frozen			
0.30					Sandy Silt			
1					Compact mottled brown sandy silt till, trace clay, trace gravel, moist			
1.37	SS		1				10.4	12
1.37					Silt Till			
1.37					Compact brown silt till, trace sand, trace gravel, moist		17.6	15
2	SS		2					
2.60					Sand			
2.60					Compact brown sand, trace silt, trace gravel, moist to very moist		4.8	30
2.60	SS		3					
2.60					Silt Till			
2.60					Dense grey silt till, trace sand gravel and clay, moist		20.0	22
3	SS		4					
4								
4.00								
4.00	SS		5				7.7	53
5.00					End of Borehole			

Bentonite Seal

Stick up well with protective metal casing

#3 Sand Filter

50mm schedule 40 PVC pipe with slot 10 screen

411.57m (Feb 8/06)

Please refer to section 4.7 of this report for additional water levels

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BOREHOLE 103

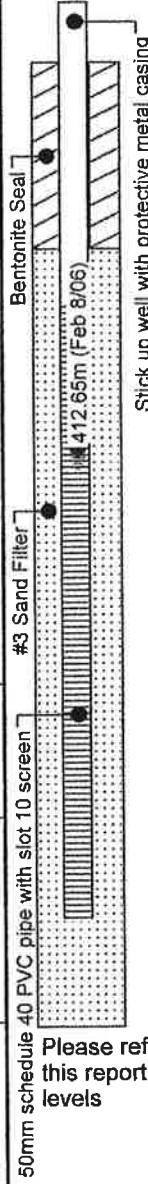
Page 1 of 1

Date Drilled: Jan. 24, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 414.89m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 414.89			
0					Topsoil 0.00 Dark brown silt, frozen			
1								
2					414.34 Organic Silt 0.55 Loose brown organic silt, some sand, occasional topsoil nodule, very moist			
3	SS		1		413.89 Silt Till 1.00 Compact brown silt till, trace sand, trace gravel, moist, becoming dense with depth			
4								
5							13.6	20
6	AS		2					
7								
8	SS		3				9.2	32
9								
10								
11	SS		4		411.54 Silty Sand 3.35 Dense brown silty sand, wet			
12								
13					410.89 Becoming loose, trace clay, saturated 4.00			
14								
15								
16	SS		5		409.89 End of Borehole 5.00		23.1	6
17								
18								
19								



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BOREHOLE 104

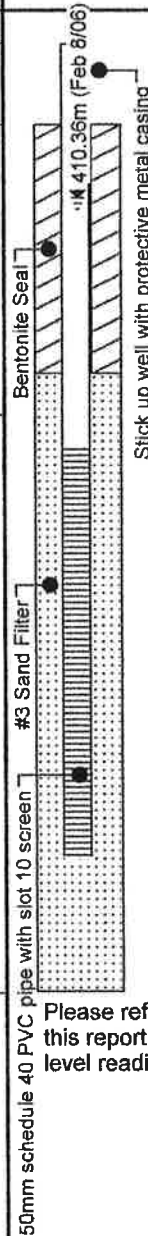
Page 1 of 1

Date Drilled: Jan. 24, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 410.93m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl 10 20 30 40	Pocket Penetrometer kPa 100 300 SPT (N) Blows/0.3 m 10 30 50 70 90
0					Ground Surface (m) 410.93			
0					Topsoil 0.00			
1					Dark brown silt, frozen 410.63			
2					Silt Till 0.30			
3	SS		1		Compact dark brown silt till, some clay, trace sand, trace gravel, moist		10.2	14
4								
5					409.56			
6	SS		2		Silty Sand 1.37		23.0	14
7					Compact brown silty sand, saturated			
8	SS		3				17.8	8
9								
10	SS		4				25.9	10
11								
12					End of Borehole 407.43			
13					3.50			
14								



Please refer to section 4.7 of this report for additional water level readings

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BOREHOLE 105

Page 1 of 1

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 414.05m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Inst ⁿ ation (Feb 8/06)	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa 100 300 SPT (N) Blows/0.3 m
0					Ground Surface (m) 414.05			
0					Topsoil 0.00			
1					Dark brown silt, frozen 413.67			
2					Silty Sand 0.38			
3	SS	1	1		Loose red-brown silty sand, moist		18.5	4
4					412.68			
5					Silt 1.37		18.5	8
6	SS	2	2		Loose brown silt, some clay, trace sand, trace gravel, very moist			
7					412.05			
8					Sand 2.00		7.7	26
9					wet sand seam at 2.0m depth			
10	SS	3	3		Sandy Silt Till		8.1	100
11					Very dense brown sandy silt till, some clay, trace gravel, moist			
12								
13								
14								
15	SS	5	5				7.6	100
16					End of Borehole 4.77			
17								
18								
19								

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BOREHOLE 106

Page 1 of 1

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 410.91m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa 100 300 SPT (N) Blows/0.3 m
0					Ground Surface (m) 410.91			
0					Topsoil 0.00			
1					Dark brown silt, frozen 410.59			
2					Silty Sand 0.32			
3	SS		1		Loose mottled grey silty sand, trace gravel, moist		24.0	9
4								
5							12.1	9
6	SS		2					
7					408.81			
8					Sandy Silt Till 2.10			
9	SS		3		Dense to very dense grey sandy silt till, trace sand, trace gravel, moist		6.8	33
10								
11	SS		4				8.3	28
12								
13								
14								
15							9.0	100
16	SS		5		405.94			
17					End of Borehole 4.97			
18								
19								

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BOREHOLE 107

Page 1 of 1

Date Drilled: Jan. 24, 2006

Rig: CME 55

Contractor: Aardvark

Drilling Method: HSA

Elevation: 409.58m

Logged by: CD

Project No.: 06-004

Project: Ainley Subdivision

Township of Centre Wellington

Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 409.58			
0					Topsoil 0.00			
1					Dark brown silt, frozen			
2					408.98			
3	SS		1		Sand 0.60			
4					Loose red-brown sand, some silt, moist to wet			
5					408.21			
6	SS		2		Clayey Silt 1.37			
7					Compact brown clayey silt, trace sand, trace gravel, moist			
8	AS		3		Sandy Silt Till 2.10			
9					Compact to loose brown sandy silt till, trace clay, saturated			
10					407.48			
11	SS		4					
12					406.08			
13					End of Borehole 3.50			
14								

Well Installation

Feb 8/06

409.43m

Bentonite Seal

Stick up well with protective metal casing

#3 Sand Filter

50mm schedule 40 PVC pipe with slot 10 screen

Please refer to section 4.7 of this report for additional water level readings

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BOREHOLE 108

Page 1 of 1

Date Drilled: Jan. 25, 2006

Rig: CME 55

Contractor: Aardvark

Drilling Method: HSA

Elevation: 410.32m

Logged by: CD

Project No.: 06-004

Project: Ainley Subdivision

Township of Centre Wellington

Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl 10 20 30 40	Pocket Penetrometer kPa 100 300 SPT (N) Blows/0.3 m 10 30 50 70 90
0					Ground Surface (m) 410.32			
0.00					Topsoil			
0.58					Dark brown silt, frozen			
2					409.74			
2.58					Silty Sand			
3	SS	1	1		Compact mottled brown silty sand, trace gravel, moist		10.5	25
4					408.95			
4.37					Sandy Silt Till			
5	SS	2	2		Dense to loose brown sandy silt till, wet		16.8	38
6								
7								
8	SS	3	3				19.9	18
9								
10								
10.37					406.82			
11	SS	4	4				26.7	7
12					End of Borehole 3.50			
13								
14								

Bentonite Seal

Stick up well with protective metal casing

409.06m (Feb 8/06)

#3 Sand Filter

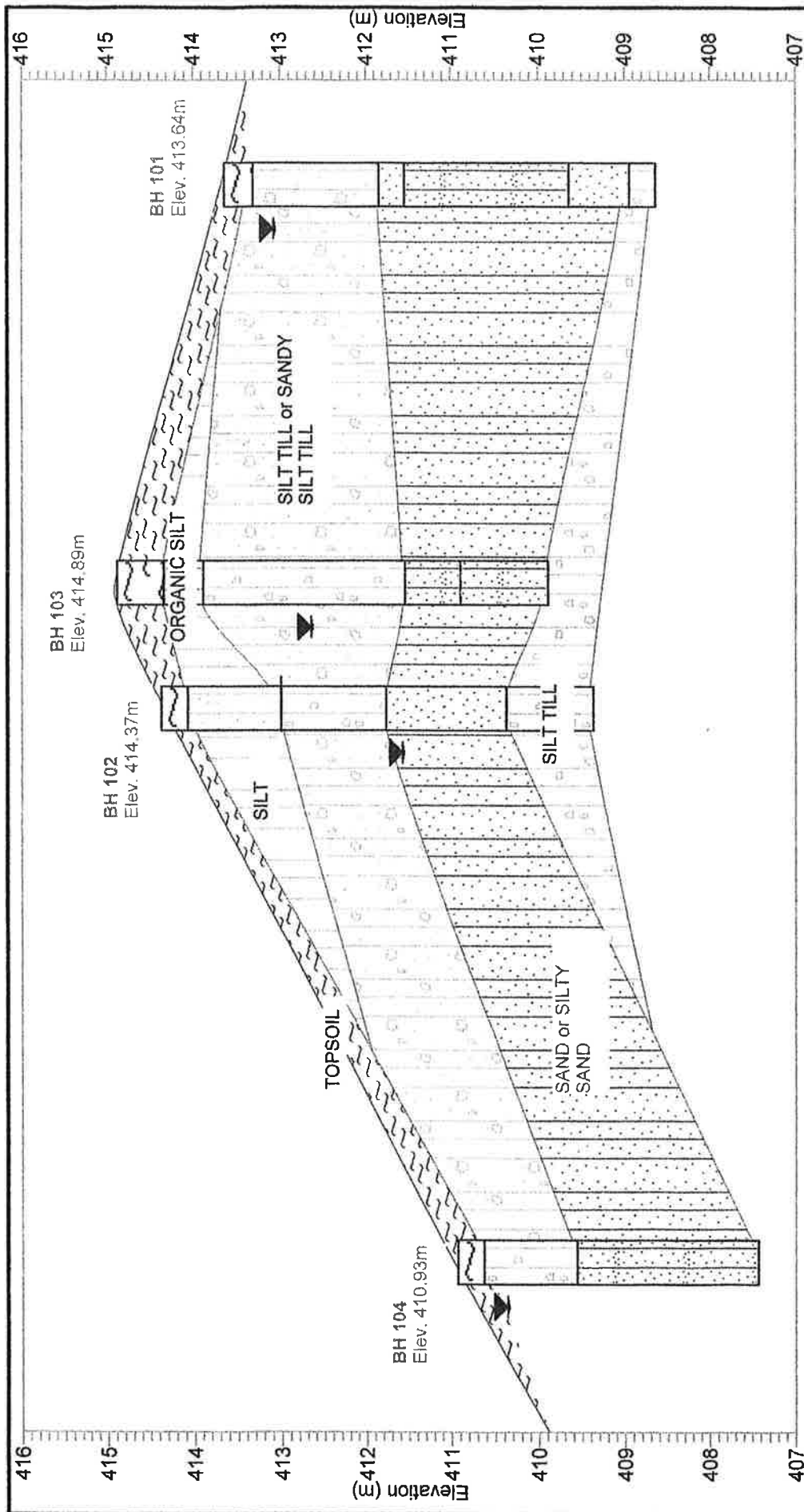
50mm schedule 40 PVC pipe with slot 10 screen

Please refer to section 4.7 of this report for additional water level readings

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APPENDIX B
CROSS-SECTIONS



NOTE: Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.

Project: AINLEY SUBDIVISION

Project Number: 06-004

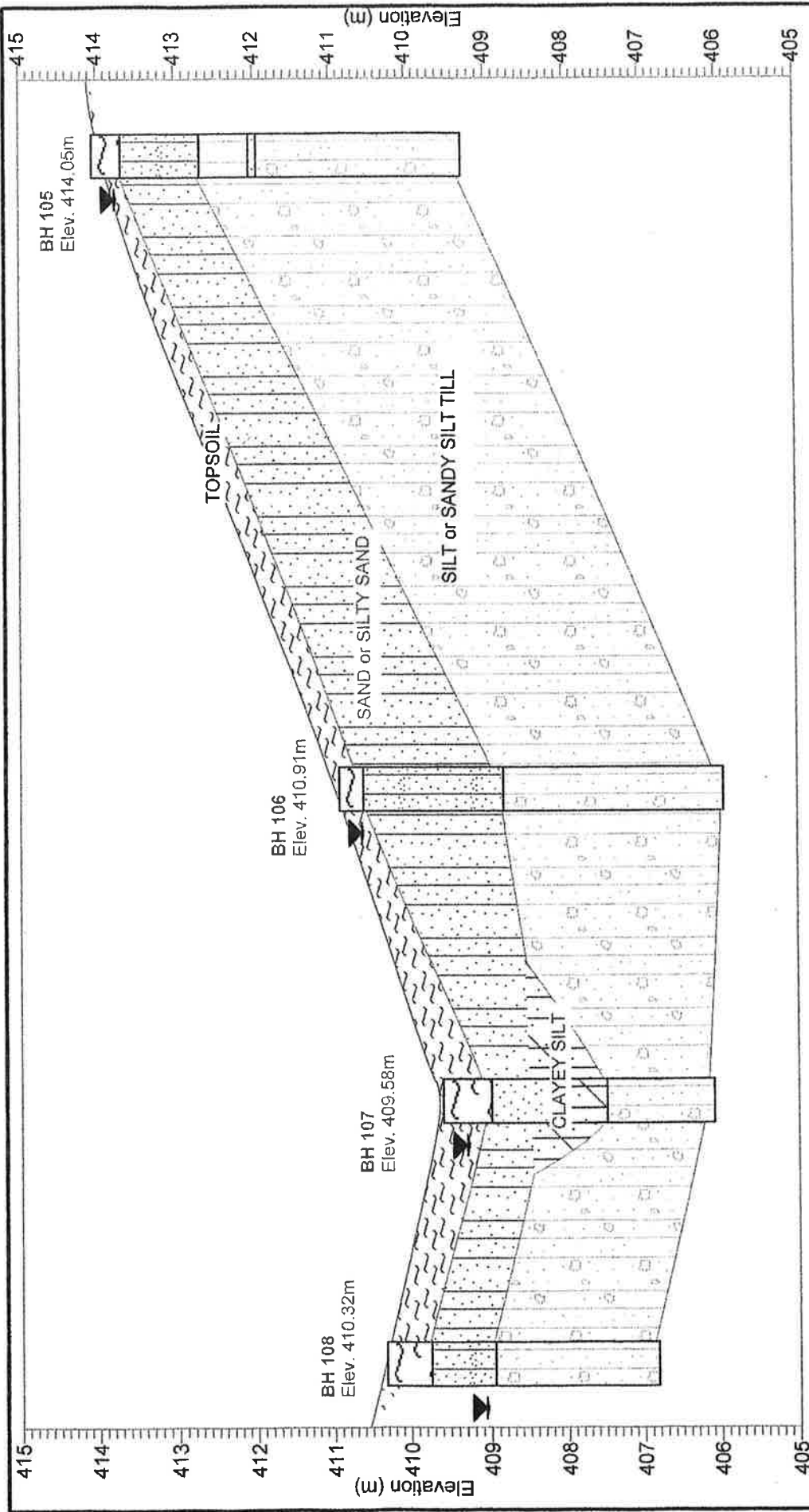
Location: Elora, Ontario

Drawn By: JS

Date: February, 2006



1011 Industrial Cres.,
Unit 1
St. Clements, Ontario



NOTE: Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.

Project: ANLEY SUBDIVISION

Project Number: 06-004

Location: Elora, Ontario

Drawn By: JS

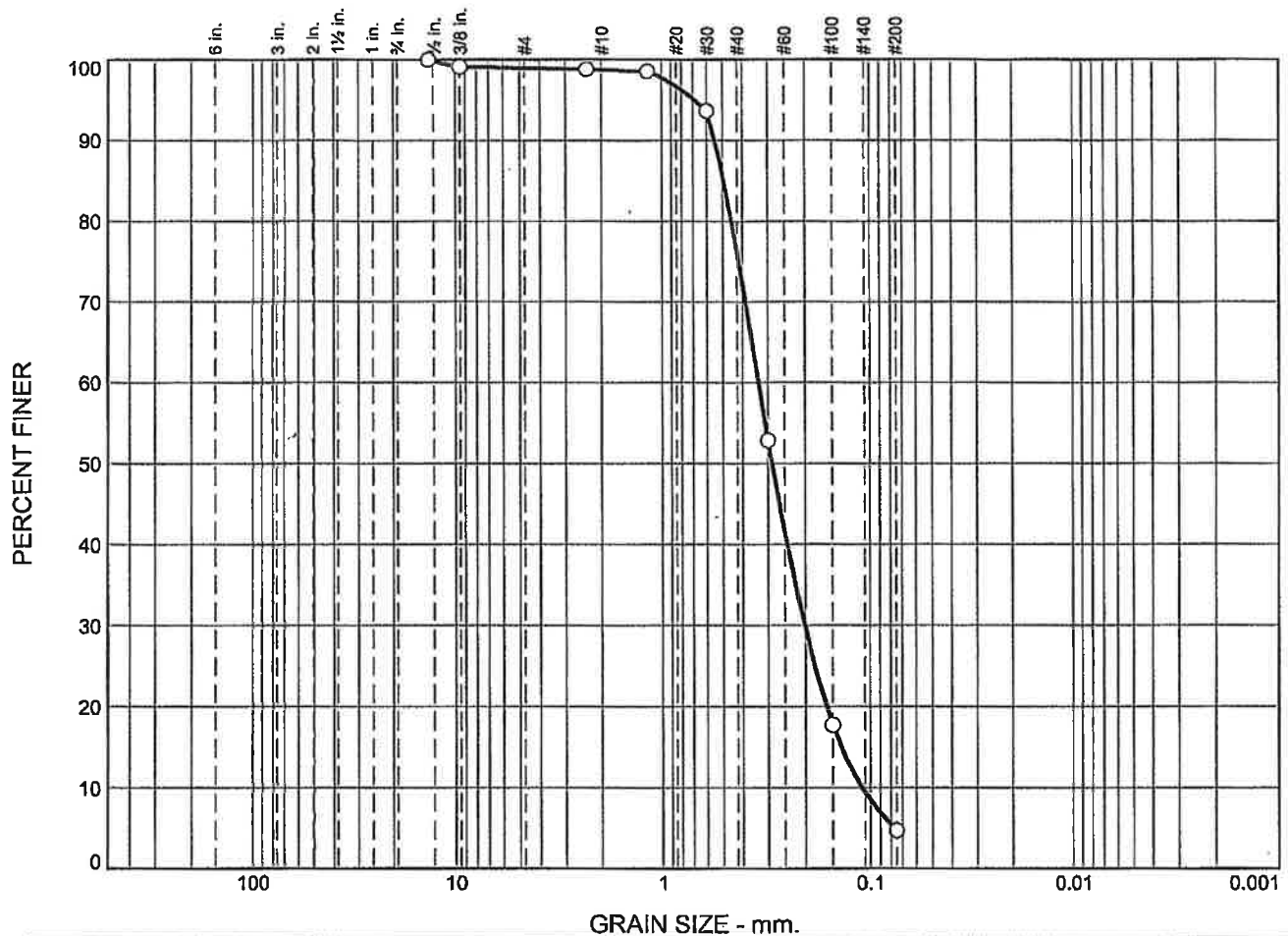
Date: February 2006



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Unit 1
St. Clements, Ontario

APPENDIX C
GRAIN SIZE ANALYSES

Particle Size Distribution Report



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	1.0	0.2	22.7	71.4	4.7	

SOIL DATA

SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH102 - SS4	1	3.05-3.51m	Sand, trace silt, trace gravel	SP
				Tested by CMT - January 27, 2006	

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St. Clements, ON

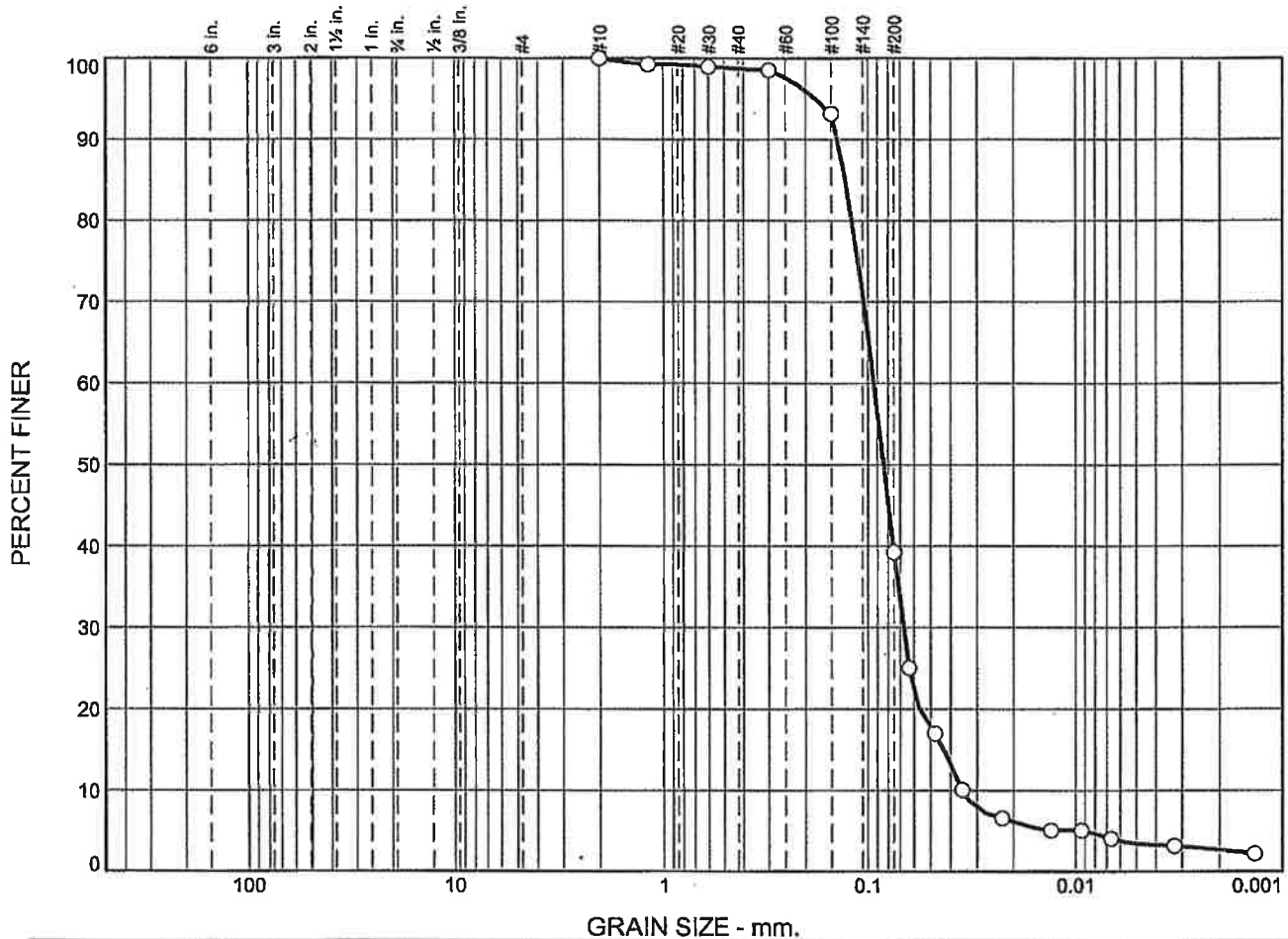
Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 1

Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	0.0	0.0	1.2	59.5	36.6	2.7

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH103 - SS5	1	4.57-5.03m	Silty sand, trace clay	SM
				Tested by CMT - January 27, 2006	

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St. Clements, ON

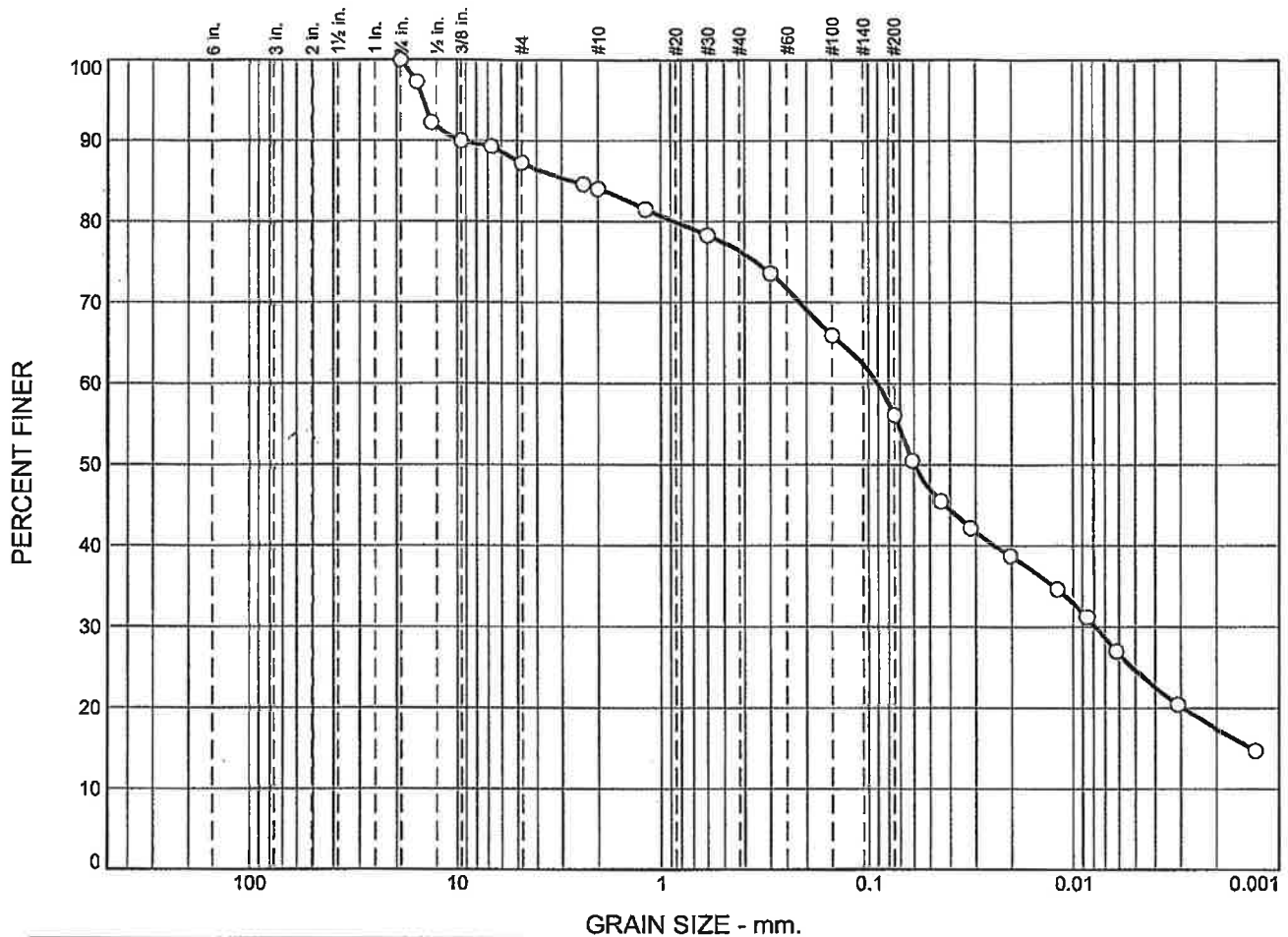
Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 2

Particle Size Distribution Report



GRAIN SIZE - mm.

	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	12.8	3.2	7.6	20.3	38.7	17.4

SOIL DATA

SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH105 - SS3	1	2.29-2.74m	Sandy silt, some clay, some gravel	ML
				Tested by CMT - January 27, 2006	

CMT Engineering Inc.

St. Clements, ON

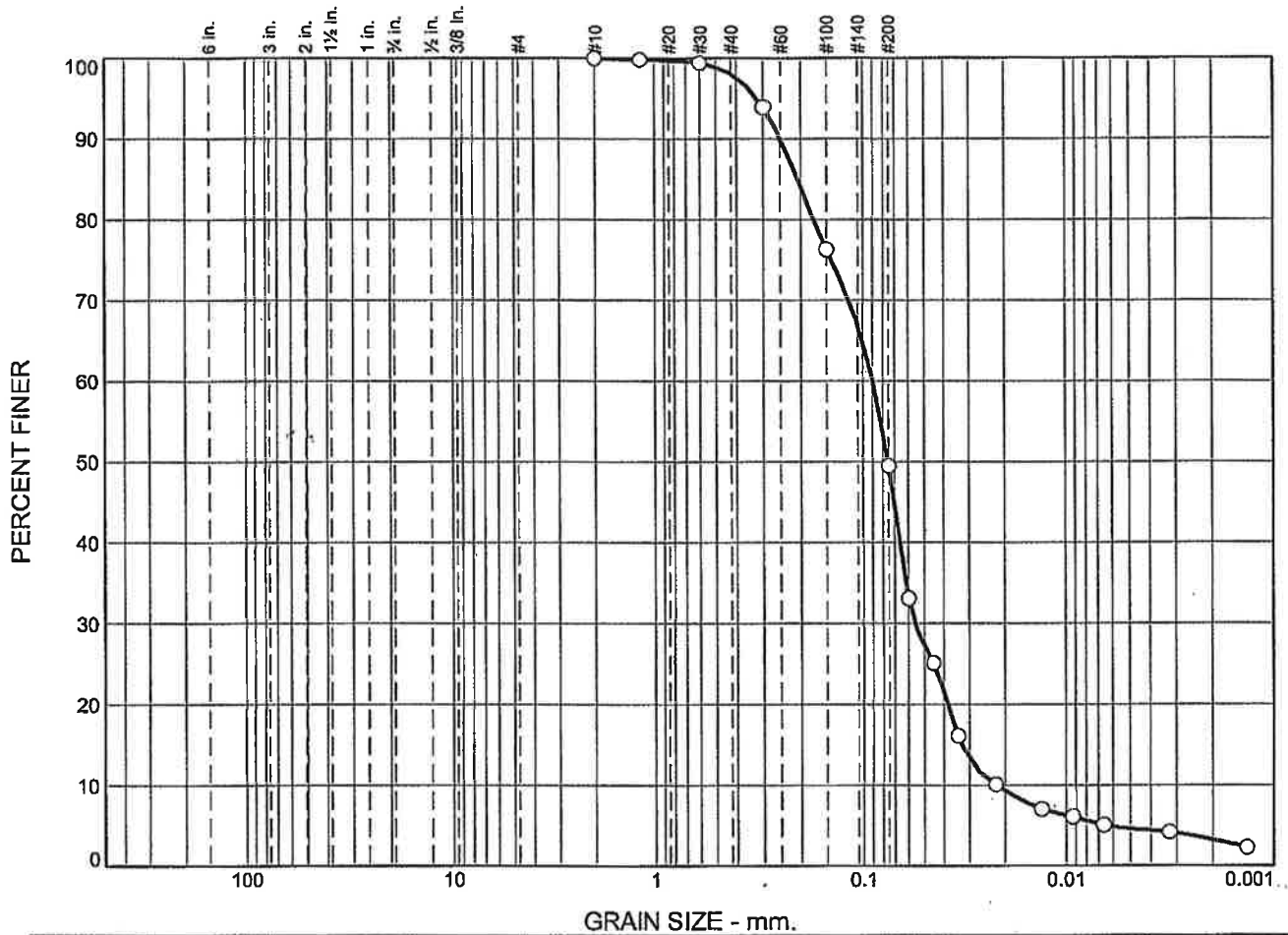
Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 3

Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	0.0	0.0	1.9	48.6	46.3	3.2

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH107 - SS3	1	2.29-2.74m	Sand and silt, trace clay	SM
				Tested by CMT - January 27, 2006	

CMT Engineering Inc.

St. Clements, ON

Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 4

APPENDIX D

LABORATORY PROCTOR TEST

CMT ENGINEERING INC.

LABORATORY PROCTOR TEST

PROJECT NO.: 06-004

PROJECT: Ainley Subdivision

PROJECT LOCATION: Township of Centre Wellington (Elora)

SAMPLED FROM: Borehole 103, BS, depth 1.5 to 2.0 m

DATE SAMPLED/BY: January 25, 2006 by C.D. of CMT Inc.

DATE TESTED/BY: January 26, 2006 by J.S. of CMT Inc.

SOIL TYPE: silt till

REMARKS:

TEST STANDARD:

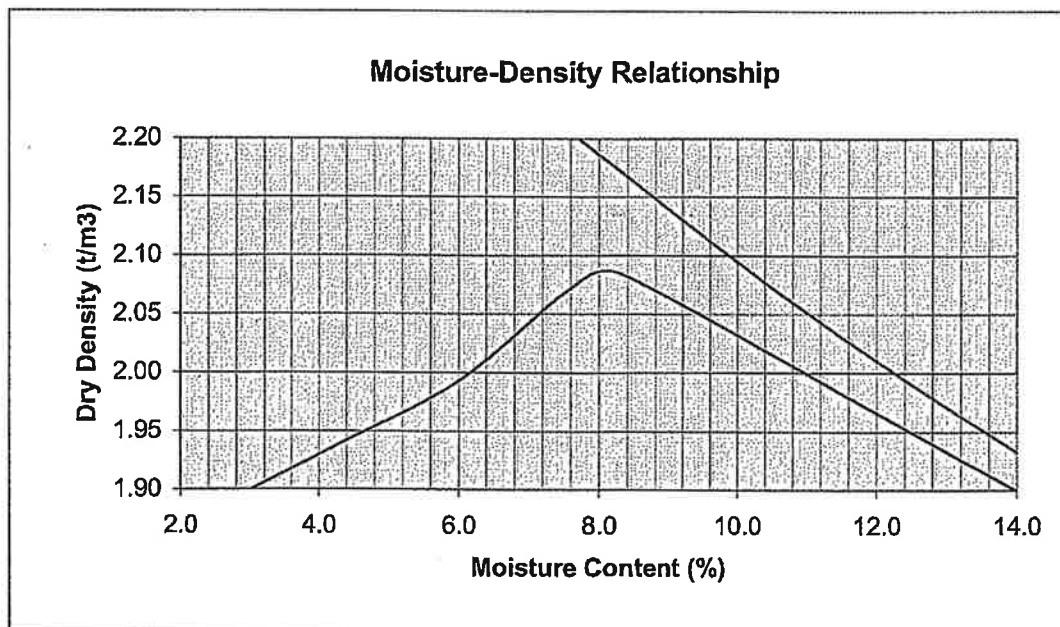
ASTM D698

PROCEDURE - ☒ A ☐ B ☐ C

ASSUMED SPECIFIC GRAVITY: 2.65

MAXIMUM DRY DENSITY: 2.085 t/m³

OPTIMUM MOISTURE CONTENT: 8.3%



APPENDIX E
WELL RECORD

Instructions for Completing Form

- For use in the Province of Ontario only. This document is a permanent legal document. Please retain for future reference.
- All Sections must be completed in full to avoid delays in processing. Further instructions and explanations are available on the back of this form.
- Questions regarding completing this application can be directed to the Water Well Management Coordinator at 416-235-8203.
- All metre measurements shall be reported to 1/10th of a metre.
- Please print clearly in blue or black ink only.

Questions regarding completing this application should be directed to the Water Permit Management Section at 905-882-2222. All metre measurements shall be reported to 1/10 th of a metre. Please print clearly in blue or black ink only.									
Well Owner's Information and Location of Well Information									
First Name James		Last Name Keenan		Mailing Address (Street Number/Name, RR, Lot, Concession) Matheson St.					
County/District/Municipality Elora		Township/City/Town/Village Elora		Province Ontario		Postal Code		Telephone Number (include area code) 519-875-1111	
Address of Well Location (County/District/Municipality) Gerric Rd.				Township Centre Wellington		Lot 17-18		Concession 12	
RR#/Street Number/Name 100				City/Town/Village Elora		Site/Compartment/Block/Tract etc. 100			
GPS Reading 43.0		NAD 83	Zone 18N	Easting 500000	Northing 4600000	Unit Make/Model 100		Mode of Operation: ☐ Differentiated ☐ Averaged ☒ Differentiated, specify	

Log of Overburden and Bedrock Materials (see instructions)

[illegible]

Hole Diameter			Construction Record				Test of Well Yield					
Depth	Metres	Diameter	Inside diam centimetres	Material	Wall thickness centimetres	Depth	Metres	Pumping test method	Draw Down		Recovery	
From	To	Centimetres				From	To		Time min	Water Level Metres	Time min	Water Level Metres
0	4.5	72										
			Casing									
			☐ Steel ☐ Fibreglass					Pump Intake set at - (metres)	Static Level			
			☒ Plastic ☐ Concrete	0 1.5				Pumping rate - (litres/min)	1		1	
			☐ Galvanized					Duration of pumping ____ hrs + ____ min	2		2	
Water Record			☐ Steel ☐ Fibreglass					Final water level and of pumping _____ metres	3		3	
Water found at	Kind of Water		☐ Plastic ☐ Concrete					Recommended pump type	4		4	
☐ m	☐ Fresh	☐ Sulphur	☐ Galvanized					☐ Shallow ☐ Deep				
☐ Gas	☐ Salty	☐ Minerals	☐ Steel ☐ Fibreglass					Recommended pump depth _____ metres	5		5	
☐ Other:			☐ Plastic ☐ Concrete					Recommended pump rate _____ (litres/min)	10		10	
☐ m	☐ Fresh	☐ Sulphur	☐ Galvanized					If flowing give rate - (litres/min)	20		20	
☐ Gas	☐ Salty	☐ Minerals							25		25	
☐ Other:								If pumping discontinued, give reason.	30		30	
☐ m	☐ Fresh	☐ Sulphur	Screen					40		40		
☐ Gas	☐ Salty	☐ Minerals	Outside diam	☐ Steel ☐ Fibreglass	Slot No.			50		50		
☐ Other:			☒ Plastic ☐ Concrete	4 1.5 4.5				60		60		
After test of well yield, water was			☐ Galvanized	10								
☐ Clear and sediment free			No Casing or Screen									
☐ Other, specify _____												
Chlorinated ☐ Yes ☐ No			☐ Open hole									

Plugging and Sealing Record			☐ Annular space	☐ Abandonment
Depth set at - Metres		Material and type (bentonite slurry, neat cement slurry) etc.	Volume Placed (cubic metres)	
From	To			
0	1	Bentonite Pellets	100 lbs	
1	1.5	# 3 Well Sand	300 lbs	
Method of Construction				
☐ Cable Tool	☐ Rotary (air)	☐ Diamond	☐ Digging	
☒ Rotary (conventional)	☐ Air percussion	☐ Jetting	☐ Other	
☐ Rotary (reverse)	☐ Boring	☐ Drilling		
Water Use				
☐ Domestic	☐ Industrial	☐ Public Supply	☐ Other	
☐ Stock	☐ Commercial	☐ Not used		
☐ Irrigation	☐ Municipal	☐ Cooling & air conditioning		
Final Status of Well				
☐ Water Supply	☐ Recharge well	☐ Unfinished	☐ Abandoned, (Other)	
☒ Observation well	☐ Abandoned, insufficient supply	☐ Dewatering		
☒ Test Hole	☐ Abandoned, poor quality	☐ Replacement well		
Well Contractor/Technician Information				
Name of Well Contractor		Well Contractor's Licence No.		
Anchor Drilling Inc		7738		
Business Address (street name, number, city etc.)				
625 Lewis Rd. Guelph On		N1H 1E9		
Name of Well Technician (last name, first name)		Well Technician's Licence No.		
Henry Mathew		7-3305		
Signature of Technician/Contractor		Date Submitted		
[Signature]		July 1981		

Location of Well

In diagram below show distances of well from road, lot line, and building. Indicate north by arrow.

Diagram showing distances of well from road, lot line, and building. The well is located near a road labeled "Gerrie Rd". Distances are marked with "X" and "Y" coordinates. The well is located near a road labeled "Gerrie Rd". Distances are marked with "X" and "Y" coordinates.

Audit No. E 39731	Date Well Completed YYY 2004 01 25
Was the well owner's information package delivered? ☐ Yes ☒ No	Date Delivered YYY MM DD 2006 02 01
Ministry Use Only	
Date Source	Contractor
Date Received YYY MM DD	Date of Inspection YYY MM DD
Remarks	Well Record Number



APPENDIX B
GROUNDWATER ELEVATION MONITORING
CMT ENGINEERING INC.
JANUARY 2016



CMT Engineering Inc.
CONSULTING ENGINEERS
1011 Industrial Crescent, Unit 1
St. Clements, Ontario N0B 2M0
Tel: 519-699-5775
Fax: 519-699-4664
www.cmtinc.net

January 20, 2016

06-004.L85

GM BluePlan Engineering Limited
330 Trillium Drive, Unit D
Kitchener, Ontario
N2E 3J2

Attention: Mr. Glenn Anderson, C.E.T.

Dear Sir:

**Re: Groundwater Monitoring
Ainley Subdivision
Elora, Ontario**

Attached is a current summary of the water level measurements for the above-referenced site. The graphs include total monthly precipitation as recorded at the Environment Canada Fergus MOE Weather Station.

I trust this information meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

Tim Salter, C.E.T.

ks

1cc: Tom Keating - James Keating Construction Ltd.

Encl - Water Level Measurements

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Feb 8/06	Water Elevation (m) Feb 20/06	Water Elevation (m) Mar 9/06	Water Elevation (m) Mar 25/06	Water Elevation (m) Mar 29/06	Water Elevation (m) Apr 28/06	Water Elevation (m) June 6/06	Water Elevation (m) July 8/06	Water Elevation (m) Aug 7/06	Water Elevation (m) Sept 7/06
101	413.64	413.07	413.11	412.83	412.96	--	412.94	412.59	411.70	411.34	411.00
102	414.37	411.57	411.96	411.91	412.48	--	412.43	412.12	411.78	411.43	411.14
103	414.89	412.65	412.98	412.88	412.77	--	412.76	411.55	410.95	410.43	410.31
104	410.93	410.36	410.60	410.17	410.66	--	410.69	410.15	409.15	408.71	408.52
105	414.05	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68	413.44	412.86	412.27	412.06	411.59
106	410.91	410.67	410.86 (F)	410.93 (F)	410.75	--	410.54	410.36	409.93	409.89	409.39
107	409.58	409.43	409.06 (F)	409.12 (F)	409.41	--	409.42	409.03	408.11	408.00	407.63
108	410.32	409.06	409.21	408.82	409.01	--	408.99	408.43	407.94	407.76	407.11

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 6/06	Water Elevation (m) Nov 11/06	Water Elevation (m) Dec 7/06	Water Elevation (m) Jan 9/07	Water Elevation (m) Feb 12/07	Water Elevation (m) Mar 8/07	Water Elevation (m) Apr 10/07	Water Elevation (m) May 12/07	Water Elevation (m) June 11/07	Water Elevation (m) July 11/07
101	413.64	410.83	412.67	412.97	413.03	412.11	411.61	413.02	412.75	411.87	411.42
102	414.37	411.00	411.25	411.71	411.99	411.69	411.45	412.14	411.96	411.67	411.38
103	414.89	410.36	411.10	411.91	412.27	411.05	410.66	412.50	411.59	410.97	410.54
104	410.93	408.71	409.13	409.45	409.65	409.12	408.96	409.78	409.39	409.24	408.92
105	414.05	411.95	413.94	413.71	413.90	412.95	412.65	413.72	413.29	412.51	412.11
106	410.91	410.13	410.74	410.57	410.59	410.28	410.22	410.55	410.50	410.07	409.59
107	409.58	408.28	408.94	409.11	409.16	408.60	408.43	409.25	409.01	408.24	407.71
108	410.32	407.62	408.21	408.48	408.57	408.07	407.93	408.69	408.36	407.96	407.57

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Aug 15/07	Water Elevation (m) Sept 13/07	Water Elevation (m) Oct 12/07	Water Elevation (m) Nov 8/07	Water Elevation (m) Dec 11/07	Water Elevation (m) Jan 15/08	Water Elevation (m) Feb 12/08	Water Elevation (m) Mar 8/08	Water Elevation (m) Apr 13/08	Water Elevation (m) May 8/08
101	413.64	411.01	410.72	410.50	410.35	410.33	412.73	412.92	413.00	413.19	413.05
102	414.37	411.13	410.98	410.87	410.81	410.77	411.26	411.53	411.80	412.92	412.46
103	414.89	410.34	410.36	410.36	410.36	410.36	411.29	411.74	412.03	413.40	412.50
104	410.93	408.73	408.61	408.57	408.58	408.76	409.70	409.66	409.81	410.70	410.35
105	414.05	411.67	411.34	411.22	411.22	411.22	414.00	414.11	414.10 frozen	413.99	413.71
106	410.91	409.22	408.99	408.89	408.89	409.44	410.63	410.61	410.64	410.77	410.69
107	409.58	407.49	407.40	407.43	407.52	407.91	409.20	409.05	409.10	409.52	409.35
108	410.32	407.36	407.37	407.37	407.37	407.52	408.52	408.50	408.57	409.19	408.89

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) June 10/08	Water Elevation (m) July 8/08	Water Elevation (m) Aug 1/08	Water Elevation (m) Sept 10/08	Water Elevation (m) Oct 8/08	Water Elevation (m) Nov 17/08	Water Elevation (m) Dec 17/08	Water Elevation (m) Jan 23/09	Water Elevation (m) Feb 20/09	Water Elevation (m) Mar 18/09
101	413.64	412.58	412.60	412.55	411.67	410.86	413.08	413.135	412.725	412.960	413.053
102	414.37	412.13	412.06	411.95	411.57	410.30	411.52	411.979	412.147	412.419	412.772
103	414.89	411.53	411.60	411.56	410.86	410.64	412.14	412.746	411.796	412.600	413.046
104	410.93	409.86	409.71	409.76	409.34	408.18	409.98	410.183	409.902	410.238	410.670
105	414.05	412.96	412.87	412.93	412.26	411.60	413.99	413.969	413.221	414.066 (F)	414.070 (F)
106	410.91	410.83	410.24	410.36	410.28	409.04	410.78	410.67	410.567	411.010 (F)	410.963 (F)
107	409.58	409.00	408.72	408.86	408.40	407.36	409.24	409.249	409.009	409.246	409.502
108	410.32	408.33	408.20	408.29	407.95	406.82	408.51	408.662	408.359	408.651	408.948

*(F) = Frozen

WATER LEVEL MEASUREMENTS

**AINLEY SUBDIVISION
ELORA, ONTARIO**

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Apr 21/09	Water Elevation (m) May 21/09	Water Elevation (m) June 26/09	Water Elevation (m) July 22/09	Water Elevation (m) Aug 27/09	Water Elevation (m) Sept 29/09	Water Elevation (m) Oct 30/09	Water Elevation (m) Dec 7/09	Water Elevation (m) Jan 5/10	Water Elevation (m) Feb 9/10
101	413.64	412.95	412.77	411.93	411.54	411.27	411.16	411.72	412.26	412.04	411.55
102	414.37	412.79	412.43	411.93	411.63	411.28	411.11	411.11	411.15	411.19	411.06
103	414.89	412.88	412.03	411.17	410.80	410.44	410.36	410.57	410.91	410.83	410.47
104	410.93	410.45	410.11	409.61	409.35	409.15	409.04	409.23	409.41	409.36	409.19
105	414.05	413.45	412.98	412.21	411.93	412.05	412.12	412.97	413.25	413.05	412.72
106	410.91	410.55	410.43	410.08	409.87	409.90	410.08	410.33	410.54	410.33	410.22
107	409.58	409.35	409.12	408.62	408.13	407.97	407.97	408.47	408.85	408.81	408.60
108	410.32	408.86	408.51	408.08	407.84	407.66	407.52	407.79	408.01	408.05	407.93

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Mar 2/10	Water Elevation (m) Apr 17/10	Water Elevation (m) May 11/10	Water Elevation (m) June 1/10	Water Elevation (m) June 29/10	Water Elevation (m) Aug 5/10	Water Elevation (m) Sept 22/10	Water Elevation (m) Oct 22/10	Water Elevation (m) Nov 9/10	Water Elevation (m) Dec 6/10
101	413.64	411.31	412.70	412.87	412.31	412.92	411.66	410.97	410.83	410.84	411.37
102	414.37	411.01	411.60	411.65	411.64	411.73	411.43	411.09	410.58	410.89	410.93
103	414.89	410.37	411.53	411.73	411.21	411.80	410.76	dry	dry	dry	dry
104	410.93	409.14	409.82	410.09	409.51	409.90	409.17	408.85	408.86	408.89	409.20
105	414.05	412.47	413.22	413.36	412.67	413.37	412.16	411.49	411.75	412.25	413.30
106	410.91	410.12	410.49	410.55	410.15	410.53	409.90	409.50	409.60	409.75	410.33
107	409.58	408.38	409.12	409.34	408.69	409.23	408.06	407.69	407.80	407.88	408.72
108	410.32	407.81	408.43	408.70	408.16	408.42	407.78	407.41	407.44	407.51	407.59

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Jan 11/11	Water Elevation (m) Feb 19/11	Water Elevation (m) Mar 31/11	Water Elevation (m) July 19/11	Water Elevation (m) Sept 30/11	Water Elevation (m) Dec 7/11	Water Elevation (m) Feb 10/12	Water Elevation (m) Apr 4/12	Water Elevation (m) June 27/12	Water Elevation (m) Aug 1/12
101	413.64	412.24	412.62	413.09	412.33	411.51	413.21	412.91	412.67	411.22	410.84
102	414.37	411.10	411.09	dry	412.09	411.22	412.23	412.16	412.07	411.28	411.05
103	414.89	410.76	411.02	dry	411.42	410.49	413.25	412.28	411.73	410.45	dry
104	410.93	409.27	409.20	410.05	409.54	409.24	410.45	409.95	409.86	409.06	408.73
105	414.05	413.52	413.93	413.86	412.53	412.83	413.93	413.53	412.99	411.75	411.30
106	410.91	410.37	410.61	410.66	410.09	410.23	410.73	410.52	410.46	409.64	409.17
107	409.58	408.85	408.70	409.29	408.21	408.18	409.44	409.14	409.10	407.93	407.52
108	410.32	408.02	407.89	408.66	407.92	407.73	408.92	408.53	408.38	407.69	dry

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 11/12	Water Elevation (m) Dec 11/12	Water Elevation (m) Mar 9/13	Water Elevation (m) May 3/13	Water Elevation (m) July 3/13	Water Elevation (m) Sept 30/13	Water Elevation (m) Dec 19/13	Water Elevation (m) Feb 19/14	Water Elevation (m) Apr 8/14	Water Elevation (m) June 6/14
101	413.64	410.48	412.25	412.19	412.94	412.74	412.56	412.58	412.28	413.60	412.70
102	414.37	410.81	411.18	411.42	412.52	412.08	411.59	411.98	411.74	412.51	412.47
103	414.89	dry	410.87	411.00	412.71	411.74	411.30	411.60	411.28	413.77	411.95
104	410.93	408.69	409.36	408.32	410.21	409.94	409.63	409.77	409.56	410.71	409.94
105	414.05	dry	413.48	413.12	413.38	412.89	413.15	413.09	412.90	414.02	412.89
106	410.91	409.26	410.53	410.40	410.55	410.44	410.37	410.46	410.40	411.02	410.38
107	409.58	407.61	408.91	408.84	409.32	409.21	408.97	409.02	408.83	409.61	409.00
108	410.32	dry	418.10	408.09	408.74	408.50	408.22	408.28	408.13	409.33	408.35

*(F) = Frozen

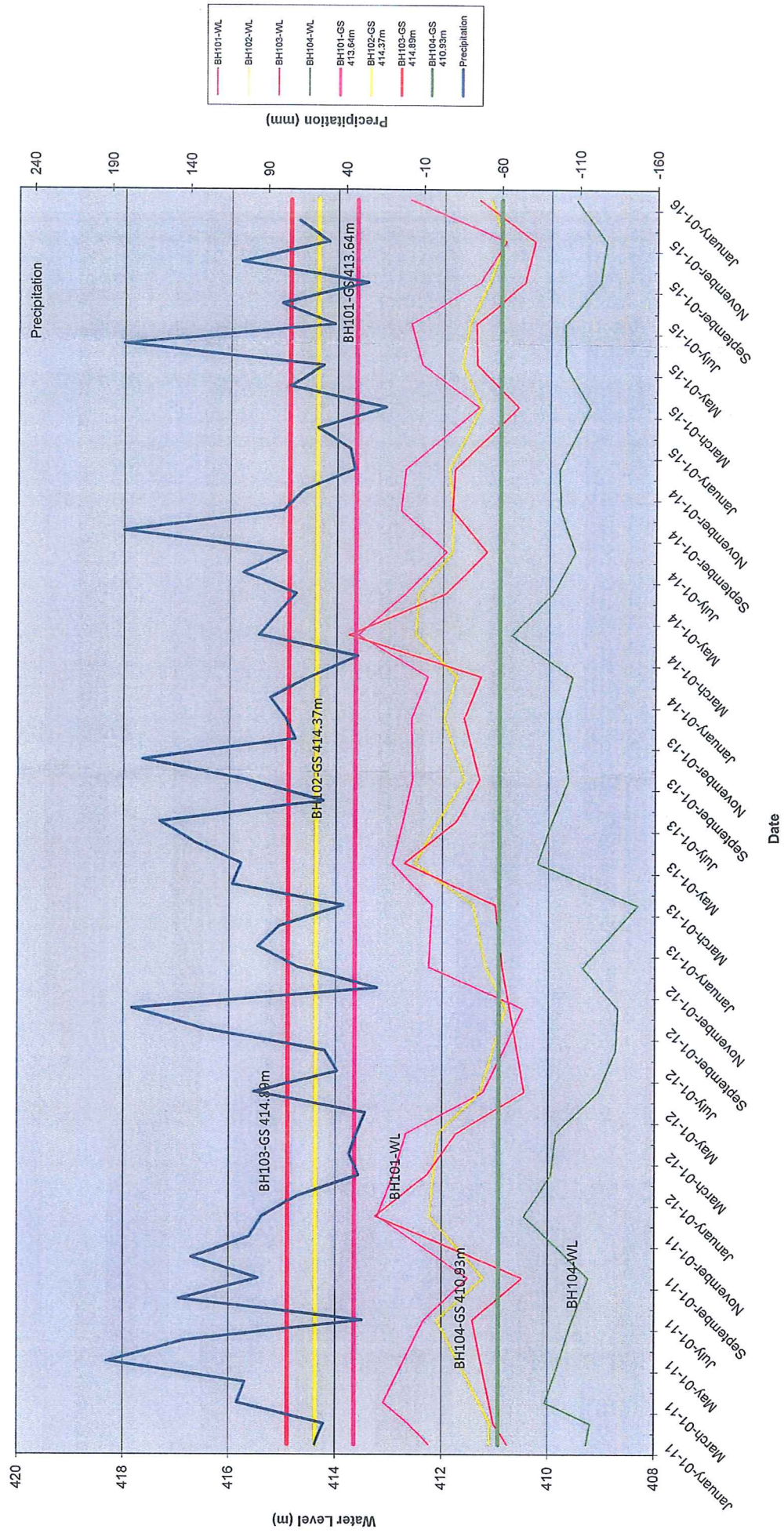
WATER LEVEL MEASUREMENTS

**AINLEY SUBDIVISION
ELORA, ONTARIO**

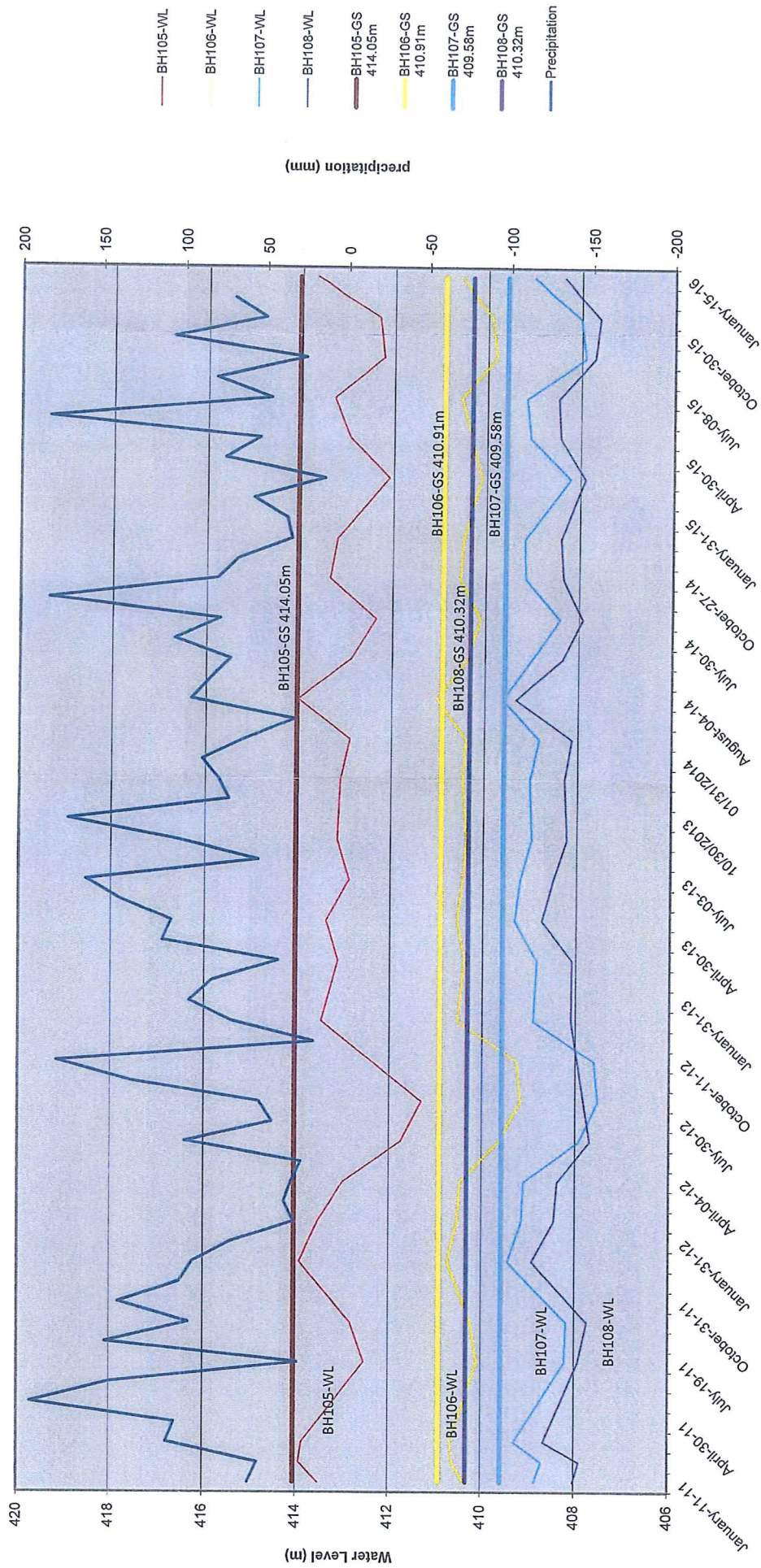
Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Aug 7/14	Water Elevation (m) Oct 27/14	Water Elevation (m) Dec 12/14	Water Elevation (m) Mar 11/15	Water Elevation (m) May 8/15	Water Elevation (m) July 8/15	Water Elevation (m) Sept 1/15	Water Elevation (m) Nov 5/15	Water Elevation (m) Jan 15/16	Water Elevation (m)
101	413.64	411.94	412.80	412.71	411.34	412.41	412.62	411.33	410.85	412.64	
102	414.37	411.83	411.81	411.90	411.30	411.66	411.53	411.16	410.88	411.14	
103	414.89	411.18	411.83	411.79	410.60	411.38	411.39	410.49	410.31	411.35	
104	410.93	409.52	409.80	409.83	409.25	409.72	409.73	409.07	408.96	409.52	
105	414.05	412.36	413.36	413.20	412.10	412.95	413.29	412.23	412.33	413.67	
106	410.91	410.11	410.55	410.48	410.11	410.33	410.55	409.82	409.91	410.57	
107	409.58	408.41	409.15	409.17	408.21	409.07	409.16	407.91	408.02	409.01	
108	410.32	407.92	408.34	408.40	407.90	408.43	408.48	407.71	407.60	408.36	

*(F) = Frozen

Water Level Measurements - Ainley Subdivision, Elora



Water Level Measurements - Ainley Subdivision, Elora





APPENDIX C
SANITARY AND STORM SEWER DESIGN SHEETS

q = average daily per capita flow (450 L/cap.d)

I = unit of peak extraneous flow (0.15 L/ha/s)

A = Tributary area in gross hectares

M = Peaking factor

Q(p) = peak population flow (L/s)

Q(i) = peak extraneous flow (L/s)

Q(d) = peak design flow

SANITARY SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

$M = 1 + \frac{14}{4 + (P)^{1/2}}$ where P is population in 1000's

$Q(p) = \frac{PqM}{86.4}$ (L/s)

Q(i) = IA

$Q(d) = Q(p) + Q(i)$ (L/s)

Ainley Farm Subdivision

Street	From	To	Individual Population	Cumulative Population	Individual Area (ha)	Cumulative Area (ha)	Peaking Factor (M)	Pop. Flow Q(p) (L/s)	Peak Extraneous Flow Q(i) (L/s)	Peak Design Flow Q(d) (m ³ /s)	Proposed Sewer						
											Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m ³ /s)	Full Flow Velocity (m/s)	Actual velocity at Q(d)
Street 2	Stub	MH.I	149	149	1.86	1.86	4.000	3.10	0.279	0.0034	10.0	200	0.013	2.00	0.0464	1.476	0.856
Street 2	MH.I	MH.F	0	149	0.21	2.07	4.000	3.10	0.311	0.0034	97.0	200	0.013	1.00	0.0328	1.044	0.668
Street 2	Stub	MH.F	135	135	2.76	2.76	4.000	2.81	0.414	0.0032	43.3	200	0.013	1.00	0.0328	1.044	0.668
Walser Street	MH.H	MH.G	18	18	0.71	0.71	4.000	0.38	0.107	0.0005	55.0	200	0.013	1.00	0.0328	1.044	0.240
Walser Street	MH.G	MH F	28	195	0.56	1.27	4.000	4.06	0.191	0.0043	70.0	200	0.013	0.50	0.0232	0.738	0.568
Walser Street	MH.F	MH.E	18	362	0.42	3.76	4.000	7.54	0.564	0.0081	82.3	200	0.013	0.50	0.0232	0.738	0.664
Walser Street	Stub	MH E	87	87	0.79	0.79	4.000	1.81	0.119	0.0019	10.0	200	0.013	1.00	0.0328	1.044	0.606
Walser Street	MH.E	MH.D	14	463	0.33	4.88	4.000	9.65	0.732	0.0104	69.8	200	0.013	0.50	0.0232	0.738	0.716
Street 1	Stub	MH D	150	0	3.61	3.61	4.000	0.00	0.542	0.0005	43.6	200	0.013	1.01	0.0330	1.049	0.346
Walser Street	MH D	MH C	7	470	0.20	8.69	4.000	9.79	1.304	0.0111	42.6	200	0.013	0.49	0.0230	0.731	0.716
Walser Street	MH C	MH B	21	491	0.51	9.20	4.000	10.23	1.380	0.0116	100.0	200	0.013	0.50	0.0232	0.738	0.738
Walser Street	MH B	MH A	14	505	0.32	9.52	4.000	10.52	1.428	0.0119	44.2	200	0.013	0.50	0.0232	0.738	0.746
Walser Street	MH A	EX.MH OUTLET A	14	519	0.34	9.86	4.000	10.81	1.479	0.0123	63.3	200	0.013	0.50	0.0232	0.738	0.753
Street 2	MH.J	MH.K	14	14	0.24	0.24	4.000	0.29	0.036	0.0003	42.2	200	0.013	0.80	0.0293	0.934	0.215
Street 2	MH.K	MH.L	28	42	0.54	0.78	4.000	0.88	0.117	0.0010	100.0	200	0.013	0.50	0.0232	0.738	0.362
Street 3	MH.M	MH.N	60	60	1.12	1.06	4.000	1.25	0.159	0.0014	100.0	200	0.013	1.00	0.0328	1.044	0.512
Street 3	MH.N	MH.O	7	67	0.11	1.17	4.000	1.40	0.176	0.0016	18.1	200	0.013	0.50	0.0232	0.738	0.428
Street 3	MH.O	MH.P	0	67	0.04	1.21	4.000	1.40	0.182	0.0016	19.2	200	0.013	1.00	0.0328	1.044	0.595
Street 3	MH.P	MH.Q	21	88	0.40	1.61	4.000	1.83	0.242	0.0021	68.0	200	0.013	0.50	0.0232	0.738	0.465

Project: Ainley Farm Subdivision

q = average daily per capita flow (450 L/cap.d)
I = unit of peak extraneous flow (0.15 L/ha/s)
A = Tributary area in gross hectares
M = Peaking factor
Q(p) = peak population flow (L/s)
Q(i) = peak extraneous flow (L/s)
Q(d) = peak design flow

SANITARY SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

$M = 1 + \frac{14}{4 + (P)^{1/2}}$ where P is population in 1000's
 $Q(p) = \frac{PqM}{86.4}$ (L/s)
 $Q(i) = IA$
 $Q(d) = Q(p) + Q(i)$ (L/s)

Ainley Farm Subdivision

			Individual Population	Cumulative Population	Individual Area (ha)	Cumulative Area (ha)	Peaking Factor (M)	Pop. Flow Q(p) (L/s)	Peak Extraneous Flow Q(i) (L/s)	Peak Design Flow Q(d) (m3/s)	Proposed Sewer						
Street	From	To									Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m³/s)	Full Flow Velocity (m/s)	Actual velocity at Q(d)
Street 3	MH.Q	MH.R	0	88	0.05	1.66	4.000	1.83	0.249	0.0021	20.8	200	0.013	0.50	0.0232	0.738	0.465
Street 3	MH.R	MH.S	4	92	0.13	1.79	4.000	1.92	0.269	0.0022	47.6	200	0.013	0.50	0.0232	0.738	0.465
Street 3	MH.T	MH.S	39	39	0.68	0.68	4.000	0.81	0.102	0.0009	67.6	200	0.013	1.00	0.0328	1.044	0.418
Street 4	MH.S	MH.U	28	159	0.50	2.97	4.000	3.31	0.446	0.0038	77.9	200	0.013	0.50	0.0232	0.738	0.539
Street 4	MH.U	MH.V	0	247	0.00	2.97	4.000	5.15	0.446	0.0056	17.6	200	0.013	0.50	0.0232	0.738	0.613
Street 4	MH.X	MH.W	32	32	0.87	0.87	4.000	0.67	0.131	0.0008	65.0	200	0.013	1.50	0.0402	1.279	0.422
Street 4	MH.W	MH.L	14	46	0.40	1.27	4.000	0.96	0.191	0.0011	65.0	200	0.013	1.50	0.0402	1.279	0.511
Street 2	MH.L	MH.V	39	127	0.87	2.92	4.000	2.65	0.438	0.0031	79.8	200	0.013	1.50	0.0402	1.279	0.780
Sanitary Easement	MH.V	MH.Y	0	374	0.06	5.95	4.000	7.79	0.893	0.0087	54.2	200	0.013	0.50	0.0232	0.738	0.679
Sanitary Easement	MH.Y	MH.Z	0	374	0.06	6.01	4.000	7.79	0.902	0.0087	82.8	200	0.013	0.50	0.0232	0.738	0.679
Sanitary Easement	MH.Z	MH.AA	0	374	0.04	9.02	4.000	7.79	1.353	0.0091	64.3	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.AA	MH.BB	0	374	0.15	9.17	4.000	7.79	1.376	0.0092	62.0	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.BB	MH.CC	0	374	0.04	9.21	4.000	7.79	1.382	0.0092	72.6	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.CC	MH.DD OUTLET B	0	374	0.04	9.25	4.000	7.79	1.388	0.0092	69.8	200	0.013	0.50	0.0232	0.738	0.694

Project: Ainley Farm Subdivision

Chicago Storm Parameters

Township of Centre Wellington

A = 1459.072

B = 13.69

C = 0.85

Intensity = $A / (t + B)^C$

STORM SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

5 Year Design

Ainley Farm Subdivision

Q = CiA (m³/s)

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m³/s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m³/s)	Full Flow Velocity (m/s)	Time of Flow (min.)
Outlet 1 - Walser Street Outlet																
WALSER STREET	MH.4	MH.3	0.21	0.45	0.09	0.09	10.00	99.01	0.026	27.4	1200	0.013	1.00	4.051	3.58	0.13
WALSER STREET	MH.3	CBMH.1	0.24	0.45	0.11	0.20	10.13	98.56	0.055	28.3	1200	0.013	1.00	4.051	3.58	0.13
WALSER STREET	CBMH.1	EX CBMH 200	0.35	0.45	0.16	0.36	10.26	98.10	0.098	48.7	300	0.013	1.70	0.131	1.85	0.44
Outlet 2 - Stormwater Management Facility #1, Storm Outlet # 1																
WALSER STREET	MH.5	MH.6	0.00	0.45	0.00	0.00	10.00	99.01	0.000	30.2	300	0.013	0.50	0.071	1.01	0.50
WALSER STREET	MH.6	CBMH.7	0.04	0.45	0.02	0.02	10.50	97.27	0.005	30.0	300	0.013	0.50	0.071	1.01	0.50
WALSER STREET	CBMH.7	MH.9	0.17	0.45	0.08	0.09	11.00	95.60	0.025	29.0	300	0.013	0.50	0.071	1.01	0.48
WALSER STREET	MH.9	CBMH.10	0.34	0.45	0.15	0.25	11.48	94.05	0.065	30.6	300	0.013	0.50	0.071	1.01	0.51
WALSER STREET	CBMH.10	DCBMH.12	0.18	0.45	0.08	0.33	11.99	92.46	0.084	10.0	375	0.013	0.50	0.129	1.17	0.14
STREET NO. 1	MH.39	CBMH.103	0.05	0.45	0.02	0.02	10.00	99.01	0.006	26.6	300	0.013	0.50	0.071	1.01	0.44
STREET NO. 1	CBMH.103	DCBMH.12	0.07	0.45	0.03	0.05	10.44	97.47	0.015	18.0	300	0.013	0.50	0.071	1.01	0.30
WALSER STREET	MH.15	CBMH.13	0.10	0.45	0.05	0.05	10.00	99.01	0.012	27.2	300	0.013	0.50	0.071	1.01	0.45
WALSER STREET	CBMH.13	DCBMH.12	0.16	0.45	0.07	0.12	10.45	97.44	0.032	18.5	300	0.013	0.50	0.071	1.01	0.31
STM EASEMENT	DCBMH.12	MH.64	0.06	0.45	0.03	0.50	12.13	92.03	0.127	74.9	450	0.013	0.50	0.209	1.32	0.95
STM EASEMENT	MH.64	MH.63	0.62	0.32	0.20	0.69	13.08	89.25	0.172	64.8	525	0.013	0.36	0.268	1.24	0.87

Minimum diameter = 300 mm

Minimum acceptable velocity = 0.75 m/s

Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision

File: 411009

Chicago Storm Parameters

Township of Centre Wellington

A = 1459.072

B = 13.69

C = 0.85

Intensity = $A / (t + B)^C$

STORM SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

5 Year Design

Ainley Farm Subdivision

Q = CiA (m³/s)

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m ³ /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m ³ /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
STREET NO. 3	STUB	MH.44	0.94	0.45	0.42	0.42	10.00	99.01	0.116	14.3	450	0.013	0.50	0.209	1.32	0.18
STREET NO. 3	MH.44	CBMH.59	0.22	0.45	0.10	0.52	10.18	98.38	0.143	60.9	450	0.013	0.50	0.209	1.32	0.77
STREET NO. 3	CBMH.59	DCBMH.61	0.36	0.45	0.16	0.68	10.95	95.75	0.182	66.8	525	0.013	0.50	0.316	1.46	0.76
STREET NO. 3	DCBMH.61	MH.63	0.44	0.45	0.20	0.88	11.71	93.31	0.229	22.3	525	0.013	0.50	0.316	1.46	0.25
STREET NO. 3	MH.63	OGS.3 STORMCEPTOR EF8	0.00	0.45	0.00	1.58	13.95	86.85	0.380	59.6	600	0.013	0.50	0.451	1.60	0.62
STREET NO. 3	OGS.3 STORMCEPTOR EF8	DCBMH.68	0.00	0.45	0.00	1.58	14.57	85.22	0.373	7.2	600	0.013	0.50	0.451	1.60	0.08
STREET NO. 3	DCBMH.68	MH.70	0.50	0.45	0.23	1.80	14.65	85.03	0.425	11.9	675	0.013	0.50	0.618	1.73	0.11
STREET NO. 2	CBMH.47	MH.49	0.17	0.45	0.08	0.08	10.00	99.01	0.021	16.0	300	0.013	0.50	0.071	1.01	0.27
STREET NO. 2	MH.49	MH.50	0.21	0.45	0.09	0.17	10.27	98.08	0.047	18.6	300	0.013	0.50	0.071	1.01	0.31
STREET NO. 2	MH.51	MH.50	0.49	0.45	0.22	0.22	10.00	99.01	0.061	25.5	375	0.013	0.50	0.129	1.17	0.36
STREET NO. 3	MH.50	MH.78	0.00	0.45	0.00	0.39	10.57	97.02	0.106	34.8	375	0.013	0.49	0.128	1.15	0.50
STREET NO. 3	MH.78	CBMH.76	0.17	0.45	0.08	0.47	10.36	97.74	0.127	26.0	450	0.013	0.50	0.209	1.32	0.33
STREET NO. 3	CBMH.76	MH.75	0.40	0.45	0.18	0.65	10.69	96.62	0.174	26.0	450	0.013	0.50	0.209	1.32	0.33
STREET NO. 3	MH.75	MH.74	0.15	0.45	0.07	0.72	11.02	95.52	0.190	14.0	450	0.013	0.50	0.209	1.32	0.18
STREET NO. 3	MH.74	DCBMH.72	0.21	0.45	0.09	0.81	11.20	94.94	0.214	26.9	525	0.013	0.50	0.316	1.46	0.31

Minimum diameter = 300 mm

Minimum acceptable velocity = 0.75 m/s

Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision

File: 411009

Chicago Storm Parameters

Township of Centre Wellington

A = 1459.072

B = 13.69

C = 0.85

Intensity = $A / (t + B)^C$

STORM SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

5 Year Design

Ainley Farm Subdivision

 $Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m ³ /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m ³ /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
STREET NO. 3	DCBMH.72	OGS.2 STORMCEPTOR EF6	0.19	0.45	0.09	0.90	11.51	93.96	0.234	11.1	525	0.013	0.50	0.316	1.46	0.13
STREET NO. 3	OGS.2 STORMCEPTOR EF6	MH.70	0.00	0.45	0.00	0.90	11.63	93.56	0.233	18.3	525	0.013	0.50	0.316	1.46	0.21
STREET NO. 3	MH.70	HEADWALL	0.00	0.45	0.00	2.70	14.76	84.74	0.635	18.7	750	0.013	0.50	0.818	1.85	0.17
Outlet 3 - Stormwater Management Facility #1, Storm Outlet # 2																
STREET NO. 4	DCBMH.104	MH.103	0.18	0.45	0.08	0.08	10.00	99.01	0.022	24.3	300	0.013	0.50	0.071	1.01	0.40
STREET NO. 4	MH.103	CBMH.102	0.15	0.45	0.07	0.15	10.40	97.60	0.040	2.8	300	0.013	0.50	0.071	1.01	0.05
STREET NO. 4	CBMH.102	CBMH.99	0.19	0.45	0.09	0.23	10.45	97.45	0.063	67.4	375	0.013	0.50	0.129	1.17	0.96
STREET NO. 4	CBMH.99	CBMH.96	0.20	0.45	0.09	0.32	11.41	94.26	0.085	31.5	450	0.013	0.50	0.209	1.32	0.40
STREET NO. 4	CBMH.96	MH.55	0.34	0.45	0.15	0.48	11.81	93.00	0.123	18.2	450	0.013	0.50	0.209	1.32	0.23
STREET NO. 2	CBMH.53	MH.55	0.16	0.45	0.07	0.07	10.00	99.01	0.020	8.9	300	0.013	0.50	0.071	1.01	0.15
STREET NO. 4	MH.55	MH.94	0.00	0.45	0.00	0.55	12.04	92.30	0.141	32.4	525	0.013	0.50	0.316	1.46	0.37
STREET NO. 4	MH.94	CBMH.91	0.15	0.45	0.07	0.62	12.41	91.18	0.156	24.5	525	0.013	0.50	0.316	1.46	0.28
STREET NO. 4	CBMH.91	MH.88	0.28	0.45	0.13	0.74	12.69	90.36	0.186	17.2	525	0.013	0.50	0.316	1.46	0.20
STREET NO. 4	CBMH.83	CBMH.86	0.19	0.45	0.09	0.09	10.00	99.01	0.024	62.1	300	0.013	0.50	0.071	1.01	1.03
STREET NO. 4	CBMH.86	MH.88	0.19	0.45	0.09	0.17	11.03	95.50	0.045	10.9	300	0.013	0.50	0.071	1.01	0.18

Minimum diameter = 300 mm

Minimum acceptable velocity = 0.75 m/s

Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision

File: 411009

Chicago Storm Parameters

Township of Centre Wellington

A = 1459.072

B = 13.69

C = 0.85

Intensity = $A / (t + B)^C$

STORM SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

5 Year Design

Ainley Farm Subdivision

Q = CiA (m³/s)

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m ³ /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m ³ /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
SWM EASEMENT	MH.88	OGS.4	0.00	0.45	0.00	0.91	12.89	89.79	0.228	63.9	525	0.013	1.05	0.458	2.12	0.50
SWM EASEMENT	OGS.4	HEADWALL	0.00	0.45	0.00	0.91	13.39	88.37	0.224	6.0	525	0.013	1.00	0.447	2.06	0.05
Outlet 4 - Stormwater Management Facility #2																
WALSER STREET	MH.16	MH.17	0.07	0.45	0.03	0.03	10.00	99.01	0.009	27.0	300	0.013	0.50	0.071	1.01	0.45
WALSER STREET	MH.17	MH.18	0.07	0.45	0.03	0.06	10.45	97.45	0.017	13.3	300	0.013	0.50	0.071	1.01	0.22
WALSER STREET	MH.18	MH.19	1.00	0.68	0.68	0.74	10.67	96.70	0.200	12.2	525	0.013	1.80	0.600	2.77	0.07
STREET NO. 2	MH.40	CBMH.105	0.05	0.45	0.02	0.02	10.00	99.01	0.006	25.7	300	0.013	0.50	0.071	1.01	0.43
STREET NO. 2	CBMH.105	MH.19	0.06	0.45	0.03	0.05	10.00	99.01	0.014	19.9	300	0.013	0.50	0.071	1.01	0.33
STREET NO. 2	DCBMH.41	MH.19	0.15	0.45	0.07	0.07	10.00	99.01	0.019	18.1	300	0.013	0.50	0.071	1.01	0.30
WALSER STREET	MH.19	MH.20	0.00	0.45	0.00	0.83	10.74	96.45	0.223	45.5	525	0.013	1.40	0.529	2.44	0.31
WALSER STREET	MH.20	CBMH.21	0.08	0.45	0.04	0.87	11.05	95.42	0.230	25.0	525	0.013	1.40	0.529	2.44	0.17
WALSER STREET	CBMH.21	DCBMH.23	0.39	0.45	0.18	1.04	11.22	94.87	0.275	63.2	525	0.013	0.80	0.799	1.81	0.58
WALSER STREET	DCBMH.23	DCBMH.24	0.41	0.45	0.18	1.23	11.80	93.02	0.318	8.5	2 - 450	0.013	0.54	0.435	0.99	0.14
WALSER STREET	DCBMH.24	OGS.1 Stormceptor EF8	0.00	0.45	0.00	1.23	11.95	92.58	0.316	5.9	2 - 450	0.013	0.49	0.415	0.94	0.10
WALSER STREET	OGS.1 Stormceptor EF8	POND 2	0.00	0.45	0.00	1.23	12.05	92.26	0.315	5.1	2 - 450	0.013	0.50	0.419	0.95	0.09
RYCB 08	POND 2		0.06	0.45	0.03	0.03	10.00	99.01	0.007	15.8	300	0.013	0.50	0.071	1.01	0.26

Minimum diameter = 300 mm

Minimum acceptable velocity = 0.75 m/s

Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision

File: 411009

Chicago Storm Parameters

Township of Centre Wellington

A = 1459.072

B = 13.69

C = 0.85

Intensity = $A / (t + B)^C$

STORM SEWER DESIGN

TOWNSHIP OF CENTRE WELLINGTON (ELORA)

5 Year Design

Ainley Farm Subdivision

 $Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m³/s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m³/s)	Full Flow Velocity (m/s)	Time of Flow (min.)
Outlet 5 - Infiltration Gallery #1																
STREET NO. 4	MH.101	MH.100	0.11	0.45	0.05	0.05	10.00	99.01	0.014	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.100	MH.96	0.11	0.45	0.05	0.10	10.52	97.20	0.027	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.96	MH.95	0.22	0.45	0.10	0.20	11.04	95.46	0.053	15.0	300	0.013	0.50	0.071	1.01	0.25
STREET NO. 4	MH.95	MH.93	0.05	0.45	0.02	0.22	11.29	94.65	0.058	47.0	300	0.013	0.50	0.071	1.01	0.78
STREET NO. 4	MH.93	MH.90	0.11	0.45	0.05	0.27	12.07	92.21	0.069	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.90	MH.89	0.24	0.45	0.11	0.38	12.59	90.66	0.095	12.3	375	0.013	0.50	0.129	1.17	0.18
STREET NO. 4	MH.85	MH.89	0.21	0.45	0.09	0.09	10.00	99.01	0.026	53.9	300	0.013	0.50	0.071	1.01	0.89
STM EASEMENT	MH.89	MH.119	0.00	0.45	0.00	0.47	12.77	90.14	0.118	37.5	375	0.013	0.50	0.129	1.17	0.54
STM EASEMENT	MH.119	MH.120	0.22	0.45	0.10	0.57	13.30	88.62	0.141	32.6	375	0.013	0.50	0.129	1.17	0.47
STM EASEMENT	MH.120	INFILTRATION GALLERY #1	0.00	0.45	0.00	0.57	13.77	87.34	0.139	8.0	375	0.013	0.50	0.129	1.17	0.11

Minimum diameter = 300 mm

Minimum acceptable velocity = 0.75 m/s

Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision

File: 411009



APPENDIX D

STORMWATER MANAGEMENT ANALYSIS

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__2yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:42:39 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.050 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                        33.014  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.240 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.155 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          8.924 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"

```

	0.003	0.000	0.000	0.000 c.m/sec"	
"	Catchment 30	Pervious	Impervious	Total Area	"
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	23.304	1.868	23.304	minutes"
"	Time to Centroid	130.781	88.659	130.781	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	79.23	0.00	79.23	c.m"
"	Rainfall losses	27.898	5.363	27.898	mm"
"	Runoff depth	5.116	27.651	5.116	mm"
"	Runoff volume	12.28	0.00	12.28	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.003	0.000	0.003	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.003	0.003	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"			
"	0.003	0.003	0.003	0.000"	
" 40	HYDROGRAPH Combine 2"				
"	6	Combine "			
"	2	Node #"			
"	To Walser Street"				
"	Maximum flow	0.003	c.m/sec"		
"	Hydrograph volume	12.279	c.m"		
"	0.003	0.003	0.003	0.003"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"	0.003	0.000	0.003	0.003"	
" 33	CATCHMENT 10"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	10	Catchment 10"			
"	0.000	% Impervious"			
"	7.760	Total Area"			
"	150.000	Flow length"			
"	2.000	Overland Slope"			
"	7.760	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.155	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.044	0.000	0.003	0.003 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	78.068	6.258	78.068	minutes"
"	Time to Centroid	195.540	95.197	195.540	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	2561.88	0.00	2561.88	c.m"
"	Rainfall losses	27.894	5.228	27.894	mm"
"	Runoff depth	5.120	27.786	5.120	mm"
"	Runoff volume	397.31	0.00	397.31	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.044	0.000	0.044	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.044	0.044	0.003	0.003"	
" 33	CATCHMENT 11"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.155	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.001	0.044	0.003	0.003 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	35.323	2.832	35.323	minutes"
"	Time to Centroid	144.986	90.217	144.986	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	42.92	0.00	42.92	c.m"

"	Rainfall losses	27.897	5.467	27.897	mm"
"	Runoff depth	5.117	27.547	5.117	mm"
"	Runoff volume	6.65	0.00	6.65	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.001	0.000	0.001	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.001	0.045	0.003	0.003"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No. "				
"	0.155 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.060	0.045	0.003	0.003 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	45.052	3.611	45.051	minutes"
"	Time to Centroid	156.495	91.497	156.495	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	2350.59	0.00	2350.59	c.m"
"	Rainfall losses	27.895	5.642	27.895	mm"
"	Runoff depth	5.119	27.372	5.119	mm"
"	Runoff volume	364.45	0.00	364.45	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.060	0.000	0.060	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.060	0.099	0.003	0.003"
" 54	POND DESIGN"				
"	0.099 Current peak flow	c.m/sec"			

```

"      0.050 Target outflow      c.m/sec"
"      768.4 Hydrograph volume    c.m"
"      6. Number of stages"
" 409.630 Minimum water level      metre"
" 410.750 Maximum water level      metre"
" 409.630 Starting water level      metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650     402.200"
"          410.000      3.601     2187.900"
"          410.250      7.811     5318.900"
"          410.500     12.984     9642.300"
"          410.750     18.965     15227.70"
"          Peak outflow              0.095      c.m/sec"
"          Maximum level              409.647     metre"
"          Maximum storage              57.616      c.m"
"          Centroidal lag              3.111     hours"
"          0.060      0.099      0.095      0.003 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.060      0.095      0.095      0.003"
" 52 CHANNEL DESIGN"
"      0.095 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.162     metre"
"          Velocity              0.541     m/sec"
"          Channel capacity              10.655     c.m/sec"
"          Critical depth              0.133     metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.460 X-factor <= 0.5"
" 100.360 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
" 100.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              0.095      c.m/sec"
"          0.060      0.095      0.095      0.003 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.060      0.095      0.095      0.003"

```

```

" 52      CHANNEL DESIGN"
"      0.095    Current peak flow      c.m/sec"
"      0.035    Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000    Basewidth      metre"
"      2.950    Left bank slope"
"      3.000    Right bank slope"
"      0.950    Channel depth      metre"
"      1.040    Gradient      %"
"      Depth of flow      0.083      metre"
"      Velocity      0.512      m/sec"
"      Channel capacity      9.246      c.m/sec"
"      Critical depth      0.059      metre"
" 53      ROUTE      Channel Route 40"
"      39.80      Channel Route 40 Reach length      ( metre)"
"      0.442      X-factor <= 0.5"
"      58.297      K-lag      ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag      ( seconds)"
"      0.500      Beta weighting factor"
"      60.000      Routing time step      ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow      0.095      c.m/sec"
"      0.060      0.095      0.095      0.003 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.060      0.095      0.095      0.003"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.060      0.095      0.095      0.003"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      0.095      c.m/sec"
"      Hydrograph volume      768.416      c.m"
"      0.060      0.095      0.095      0.095"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.060      0.000      0.095      0.095"
" 33      CATCHMENT 20"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      20      Catchment 20"
"      0.000      % Impervious"
"      6.650      Total Area"
"      150.000      Flow length"
"      2.000      Overland Slope"

```

"	6.650	Pervious Area"
"	150.000	Pervious length"
"	2.000	Pervious slope"
"	0.000	Impervious Area"
"	150.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	74.000	Pervious SCS Curve No."
"	0.155	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	8.924	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.000	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"
"	0.038	0.000 0.095 0.095 c.m/sec"
"	Catchment 20	Pervious Impervious Total Area "
"	Surface Area	6.650 0.000 6.650 hectare"
"	Time of concentration	78.068 6.258 78.068 minutes"
"	Time to Centroid	195.540 95.197 195.539 minutes"
"	Rainfall depth	33.014 33.014 33.014 mm"
"	Rainfall volume	2195.43 0.00 2195.43 c.m"
"	Rainfall losses	27.894 5.228 27.894 mm"
"	Runoff depth	5.120 27.786 5.120 mm"
"	Runoff volume	340.48 0.00 340.48 c.m"
"	Runoff coefficient	0.155 0.000 0.155 "
"	Maximum flow	0.038 0.000 0.038 c.m/sec"
" 40	HYDROGRAPH Add Runoff "	
"	4 Add Runoff "	
"	0.038 0.038 0.095 0.095"	
" 33	CATCHMENT 21"	
"	1 Triangular SCS"	
"	1 Equal length"	
"	1 SCS method"	
"	21 Catchment 20"	
"	10.000 % Impervious"	
"	0.830 Total Area"	
"	40.000 Flow length"	
"	2.000 Overland Slope"	
"	0.747 Pervious Area"	
"	40.000 Pervious length"	
"	2.000 Pervious slope"	
"	0.083 Impervious Area"	
"	40.000 Impervious length"	
"	2.000 Impervious slope"	
"	0.250 Pervious Manning 'n'"	
"	74.000 Pervious SCS Curve No."	
"	0.155 Pervious Runoff coefficient"	
"	0.100 Pervious Ia/S coefficient"	

"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.834	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.016	0.038	0.095	0.095 c.m/sec"	
"		Catchment 21	Pervious	Impervious	Total Area	"
"		Surface Area	0.747	0.083	0.830	hectare"
"		Time of concentration	35.323	2.832	23.162	minutes"
"		Time to Centroid	144.986	90.217	124.487	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	246.61	27.40	274.02	c.m"
"		Rainfall losses	27.897	5.467	25.654	mm"
"		Runoff depth	5.117	27.547	7.360	mm"
"		Runoff volume	38.22	22.86	61.09	c.m"
"		Runoff coefficient	0.155	0.834	0.223	"
"		Maximum flow	0.007	0.016	0.016	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.016	0.043	0.095	0.095"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.016	0.043	0.043	0.095"	
" 64		SHOW TABLE"				
"	2	Flow hydrograph"				
"	4	Inflow Hydrograph"				
"		Maximum flow		0.043	c.m/sec"	
"		Hydrograph volume		401.567	c.m"	
" 40		HYDROGRAPH Combine 1"				
"	6	Combine "				
"	1	Node #"				
"		Total"				
"		Maximum flow		0.138	c.m/sec"	
"		Hydrograph volume		1169.982	c.m"	
"		0.016	0.043	0.043	0.138"	
" 38		START/RE-START TOTALS 21"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.730	hectare"	
"		Total Impervious area		0.083	hectare"	
"		Total % impervious		0.365"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__5yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:44:05 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          113.586  mm/hr"
"          Total depth                49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000 Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000 Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.257  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.010	0.000	0.000	0.000 c.m/sec"
"	Catchment 30	Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240 hectare"
"	Time of concentration	16.417	1.691	16.417 minutes"
"	Time to Centroid	118.292	87.210	118.292 minutes"
"	Rainfall depth	49.792	49.792	49.792 mm"
"	Rainfall volume	119.50	0.00	119.50 c.m"
"	Rainfall losses	36.983	5.811	36.983 mm"
"	Runoff depth	12.809	43.981	12.809 mm"
"	Runoff volume	30.74	0.00	30.74 c.m"
"	Runoff coefficient	0.257	0.000	0.257 "
"	Maximum flow	0.010	0.000	0.010 c.m/sec"
" 40	HYDROGRAPH Add Runoff "			
"	4 Add Runoff "			
"	0.010	0.010	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"			
"	8 Copy to Outflow"			
"	0.010	0.010	0.010	0.000"
" 40	HYDROGRAPH Combine 2"			
"	6 Combine "			
"	2 Node #"			
"	To Walser Street"			
"	Maximum flow	0.010		c.m/sec"
"	Hydrograph volume	30.741		c.m"
"	0.010	0.010	0.010	0.010"
" 40	HYDROGRAPH Start - New Tributary"			
"	2 Start - New Tributary"			
"	0.010	0.000	0.010	0.010"
" 33	CATCHMENT 10"			
"	1 Triangular SCS"			
"	1 Equal length"			
"	1 SCS method"			
"	10 Catchment 10"			
"	0.000 % Impervious"			
"	7.760 Total Area"			
"	150.000 Flow length"			
"	2.000 Overland Slope"			
"	7.760 Pervious Area"			
"	150.000 Pervious length"			
"	2.000 Pervious slope"			
"	0.000 Impervious Area"			
"	150.000 Impervious length"			
"	2.000 Impervious slope"			
"	0.250 Pervious Manning 'n' "			
"	74.000 Pervious SCS Curve No."			
"	0.258 Pervious Runoff coefficient"			
"	0.100 Pervious Ia/S coefficient"			
"	8.924 Pervious Initial abstraction"			
"	0.015 Impervious Manning 'n' "			
"	98.000 Impervious SCS Curve No."			

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.157	0.000	0.010	0.010 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	54.995	5.665	54.994	minutes"
"	Time to Centroid	162.955	92.780	162.955	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	3863.83	0.00	3863.84	c.m"
"	Rainfall losses	36.958	5.466	36.958	mm"
"	Runoff depth	12.834	44.325	12.834	mm"
"	Runoff volume	995.89	0.00	995.90	c.m"
"	Runoff coefficient	0.258	0.000	0.258	"
"	Maximum flow	0.157	0.000	0.157	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.157	0.157	0.010	0.010"	
" 33	CATCHMENT 11"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.258	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.004	0.157	0.010	0.010 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	24.883	2.563	24.883	minutes"
"	Time to Centroid	128.082	88.517	128.082	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	64.73	0.00	64.73	c.m"

"	Rainfall losses	36.970	6.066	36.969	mm"
"	Runoff depth	12.822	43.726	12.822	mm"
"	Runoff volume	16.67	0.00	16.67	c.m"
"	Runoff coefficient	0.258	0.000	0.258	"
"	Maximum flow	0.004	0.000	0.004	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.004 0.159 0.010 0.010"				
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No. "				
"	0.258 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.209 0.159 0.010 0.010 c.m/sec"				
"	Catchment 40 Pervious Impervious Total Area "				
"	Surface Area 7.120 0.000 7.120 hectare"				
"	Time of concentration 31.736 3.269 31.736 minutes"				
"	Time to Centroid 136.024 89.581 136.024 minutes"				
"	Rainfall depth 49.792 49.792 49.792 mm"				
"	Rainfall volume 3545.16 0.00 3545.17 c.m"				
"	Rainfall losses 36.968 6.236 36.968 mm"				
"	Runoff depth 12.824 43.556 12.824 mm"				
"	Runoff volume 913.04 0.00 913.04 c.m"				
"	Runoff coefficient 0.258 0.000 0.258 "				
"	Maximum flow 0.209 0.000 0.209 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.209 0.353 0.010 0.010"				
" 54	POND DESIGN"				
"	0.353 Current peak flow c.m/sec"				

```

"      0.050 Target outflow      c.m/sec"
"      1925.6 Hydrograph volume    c.m"
"      6. Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650     402.200"
"          410.000      3.601     2187.900"
"          410.250      7.811     5318.900"
"          410.500     12.984     9642.300"
"          410.750     18.965     15227.70"
"          Peak outflow              0.324      c.m/sec"
"          Maximum level              409.688     metre"
"          Maximum storage            195.955     c.m"
"          Centroidal lag              2.666     hours"
"          0.209      0.353      0.324      0.010 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.209      0.324      0.324      0.010"
" 52 CHANNEL DESIGN"
"      0.324 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.256     metre"
"          Velocity                  0.735     m/sec"
"          Channel capacity            10.655     c.m/sec"
"          Critical depth              0.217     metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.436 X-factor <= 0.5"
"      73.851 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      75.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              0.323      c.m/sec"
"          0.209      0.324      0.323      0.010 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.209      0.323      0.323      0.010"

```

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" 52      CHANNEL DESIGN"
"      0.323  Current peak flow      c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth      metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth      metre"
"      1.040  Gradient      %"
"          Depth of flow              0.167      metre"
"          Velocity                    0.774      m/sec"
"          Channel capacity            9.246      c.m/sec"
"          Critical depth              0.129      metre"
" 53      ROUTE      Channel Route 40"
"      39.80    Channel Route 40 Reach length      ( metre)"
"      0.386    X-factor <= 0.5"
"      38.571    K-lag      ( seconds)"
"      0.000    Default(0) or user spec.(1) values used"
"      0.500    X-factor <= 0.5"
"      30.000    K-lag      ( seconds)"
"      0.500    Beta weighting factor"
"      42.857    Routing time step      ( seconds)"
"          1    No. of sub-reaches"
"          Peak outflow              0.322      c.m/sec"
"          0.209      0.323      0.322      0.010 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5    Next link "
"          0.209      0.322      0.322      0.010"
" 40      HYDROGRAPH Copy to Outflow"
"          8    Copy to Outflow"
"          0.209      0.322      0.322      0.010"
" 40      HYDROGRAPH Combine      1"
"          6    Combine "
"          1    Node #"
"          Total"
"          Maximum flow              0.322      c.m/sec"
"          Hydrograph volume          1925.607      c.m"
"          0.209      0.322      0.322      0.322"
" 40      HYDROGRAPH Start - New Tributary"
"          2    Start - New Tributary"
"          0.209      0.000      0.322      0.322"
" 33      CATCHMENT 20"
"          1    Triangular SCS"
"          1    Equal length"
"          1    SCS method"
"          20    Catchment 20"
"          0.000    % Impervious"
"          6.650    Total Area"
"          150.000    Flow length"
"          2.000    Overland Slope"

```

"	6.650	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.258	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.135	0.000	0.322	0.322 c.m/sec"	
"	Catchment 20	Pervious	Impervious	Total Area	"
"	Surface Area	6.650	0.000	6.650	hectare"
"	Time of concentration	54.995	5.665	54.994	minutes"
"	Time to Centroid	162.956	92.780	162.955	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	3311.14	0.00	3311.15	c.m"
"	Rainfall losses	36.958	5.466	36.958	mm"
"	Runoff depth	12.834	44.325	12.834	mm"
"	Runoff volume	853.44	0.00	853.44	c.m"
"	Runoff coefficient	0.258	0.000	0.258	"
"	Maximum flow	0.135	0.000	0.135	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.135	0.135	0.322	0.322"	
" 33	CATCHMENT 21"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	21 Catchment 20"				
"	10.000 % Impervious"				
"	0.830 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.747 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.083 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.258 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				

"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.878	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.030	0.135	0.322	0.322 c.m/sec"
"		Catchment 21	Pervious	Impervious	Total Area "
"		Surface Area	0.747	0.083	0.830 hectare"
"		Time of concentration	24.883	2.563	18.750 minutes"
"		Time to Centroid	128.082	88.517	117.210 minutes"
"		Rainfall depth	49.792	49.792	49.792 mm"
"		Rainfall volume	371.94	41.33	413.27 c.m"
"		Rainfall losses	36.970	6.066	33.879 mm"
"		Runoff depth	12.822	43.726	15.913 mm"
"		Runoff volume	95.78	36.29	132.07 c.m"
"		Runoff coefficient	0.258	0.878	0.320 "
"		Maximum flow	0.025	0.021	0.030 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.030	0.153	0.322	0.322"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.030	0.153	0.153	0.322"
" 64		SHOW TABLE"			
"	2	Flow hydrograph"			
"	4	Inflow Hydrograph"			
"		Maximum flow	0.153	c.m/sec"	
"		Hydrograph volume	985.517	c.m"	
" 40		HYDROGRAPH Combine 1"			
"	6	Combine "			
"	1	Node #"			
"		Total"			
"		Maximum flow	0.472	c.m/sec"	
"		Hydrograph volume	2911.122	c.m"	
"		0.030	0.153	0.153	0.472"
" 38		START/RE-START TOTALS 21"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area		22.730	hectare"
"		Total Impervious area		0.083	hectare"
"		Total % impervious		0.365"	
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__10yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:46:41 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          2327.596 Coefficient A"
"          19.500  Constant B"
"          0.894  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          126.171  mm/hr"
"          Total depth                61.359  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000 Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000 Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.316  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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"		0.017	0.000	0.000	0.000 c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	14.182	1.611	14.182	minutes"
"	Time to Centroid	113.893	86.563	113.893	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	147.26	0.00	147.26	c.m"
"	Rainfall losses	41.992	6.044	41.992	mm"
"	Runoff depth	19.367	55.315	19.367	mm"
"	Runoff volume	46.48	0.00	46.48	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.017	0.000	0.017	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.017	0.017	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.017	0.017	0.017	0.000"
" 40	HYDROGRAPH Combine 2"				
"	6 Combine "				
"	2 Node #"				
"	To Walser Street"				
"	Maximum flow		0.017		c.m/sec"
"	Hydrograph volume		46.481		c.m"
"		0.017	0.017	0.017	0.017"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.017	0.000	0.017	0.017"
" 33	CATCHMENT 10"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	10 Catchment 10"				
"	0.000 % Impervious"				
"	7.760 Total Area"				
"	150.000 Flow length"				
"	2.000 Overland Slope"				
"	7.760 Pervious Area"				
"	150.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	150.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.316 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.273	0.000	0.017	0.017 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	47.507	5.395	47.507	minutes"
"	Time to Centroid	151.963	91.698	151.963	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	4761.47	0.00	4761.48	c.m"
"	Rainfall losses	41.963	5.633	41.963	mm"
"	Runoff depth	19.396	55.726	19.396	mm"
"	Runoff volume	1505.11	0.00	1505.12	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.273	0.000	0.273	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.273	0.273	0.017	0.017"	
" 33	CATCHMENT 11"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.316	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.008	0.273	0.017	0.017 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	21.495	2.441	21.495	minutes"
"	Time to Centroid	122.241	87.742	122.240	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	79.77	0.00	79.77	c.m"

"	Rainfall losses	41.967	6.310	41.967	mm"
"	Runoff depth	19.392	55.050	19.393	mm"
"	Runoff volume	25.21	0.00	25.21	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.008	0.000	0.008	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.008	0.277	0.017	0.017"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No. "				
"	0.316 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.359	0.277	0.017	0.017 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	27.416	3.114	27.416	minutes"
"	Time to Centroid	128.990	88.727	128.990	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	4368.77	0.00	4368.78	c.m"
"	Rainfall losses	41.968	6.469	41.968	mm"
"	Runoff depth	19.392	54.890	19.392	mm"
"	Runoff volume	1380.67	0.00	1380.68	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.359	0.000	0.359	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.359	0.608	0.017	0.017"
" 54	POND DESIGN"				
"	0.608 Current peak flow	c.m/sec"			

```

"      0.050 Target outflow      c.m/sec"
"      2911.0 Hydrograph volume    c.m"
"      6. Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650     402.200"
"          410.000      3.601     2187.900"
"          410.250      7.811     5318.900"
"          410.500     12.984     9642.300"
"          410.750     18.965     15227.70"
"          Peak outflow              0.549      c.m/sec"
"          Maximum level              409.729     metre"
"          Maximum storage            332.166      c.m"
"          Centroidal lag              2.515     hours"
"          0.359      0.608      0.549      0.017 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.359      0.549      0.549      0.017"
" 52 CHANNEL DESIGN"
"      0.549 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.312     metre"
"          Velocity                  0.839     m/sec"
"          Channel capacity            10.655     c.m/sec"
"          Critical depth              0.267     metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.422 X-factor <= 0.5"
"      64.729 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              0.547      c.m/sec"
"          0.359      0.549      0.547      0.017 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.359      0.547      0.547      0.017"

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" 52      CHANNEL DESIGN"
"      0.547    Current peak flow      c.m/sec"
"      0.035    Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000    Basewidth      metre"
"      2.950    Left bank slope"
"      3.000    Right bank slope"
"      0.950    Channel depth      metre"
"      1.040    Gradient      %"
"      Depth of flow      0.224      metre"
"      Velocity      0.914      m/sec"
"      Channel capacity      9.246      c.m/sec"
"      Critical depth      0.179      metre"
" 53      ROUTE      Channel Route 40"
"      39.80      Channel Route 40 Reach length      ( metre)"
"      0.350      X-factor <= 0.5"
"      32.667      K-lag      ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag      ( seconds)"
"      0.500      Beta weighting factor"
"      37.500      Routing time step      ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow      0.546      c.m/sec"
"      0.359      0.547      0.546      0.017 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.359      0.546      0.546      0.017"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.359      0.546      0.546      0.017"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      0.546      c.m/sec"
"      Hydrograph volume      2911.006      c.m"
"      0.359      0.546      0.546      0.546"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.359      0.000      0.546      0.546"
" 33      CATCHMENT 20"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      20      Catchment 20"
"      0.000      % Impervious"
"      6.650      Total Area"
"      150.000      Flow length"
"      2.000      Overland Slope"

```

"	6.650	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.316	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.234	0.000	0.546	0.546 c.m/sec"	
"	Catchment 20	Pervious	Impervious	Total Area	"
"	Surface Area	6.650	0.000	6.650	hectare"
"	Time of concentration	47.507	5.395	47.507	minutes"
"	Time to Centroid	151.963	91.698	151.963	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	4080.39	0.00	4080.39	c.m"
"	Rainfall losses	41.963	5.633	41.963	mm"
"	Runoff depth	19.396	55.726	19.396	mm"
"	Runoff volume	1289.82	0.00	1289.82	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.234	0.000	0.234	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.234	0.234	0.546	0.546"	
" 33	CATCHMENT 21"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	21 Catchment 20"				
"	10.000 % Impervious"				
"	0.830 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.747 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.083 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.316 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				

"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.897	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.050	0.234	0.546	0.546 c.m/sec"
"		Catchment 21	Pervious	Impervious	Total Area "
"		Surface Area	0.747	0.083	0.830 hectare"
"		Time of concentration	21.495	2.441	16.926 minutes"
"		Time to Centroid	122.241	87.742	113.968 minutes"
"		Rainfall depth	61.359	61.359	61.359 mm"
"		Rainfall volume	458.35	50.93	509.28 c.m"
"		Rainfall losses	41.967	6.310	38.401 mm"
"		Runoff depth	19.392	55.050	22.958 mm"
"		Runoff volume	144.86	45.69	190.55 c.m"
"		Runoff coefficient	0.316	0.897	0.374 "
"		Maximum flow	0.044	0.024	0.050 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.050	0.264	0.546	0.546"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.050	0.264	0.264	0.546"
" 64		SHOW TABLE"			
"	2	Flow hydrograph"			
"	4	Inflow Hydrograph"			
"		Maximum flow	0.264	c.m/sec"	
"		Hydrograph volume	1480.378	c.m"	
" 40		HYDROGRAPH Combine 1"			
"	6	Combine "			
"	1	Node #"			
"		Total"			
"		Maximum flow	0.806	c.m/sec"	
"		Hydrograph volume	4391.384	c.m"	
"		0.050	0.264	0.264	0.806"
" 38		START/RE-START TOTALS 21"			
"	3	Runoff Totals on EXIT"			
"		Total Catchment area		22.730	hectare"
"		Total Impervious area		0.083	hectare"
"		Total % impervious		0.365"	
" 19		EXIT"			

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__25yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:47:44 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          3701.648 Coefficient A"
"          25.500  Constant B"
"          0.937  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          143.371  mm/hr"
"          Total depth                75.581  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.376  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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"		0.026	0.000	0.000	0.000 c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	12.370	1.523	12.370	minutes"
"	Time to Centroid	110.314	85.984	110.314	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	181.39	0.00	181.39	c.m"
"	Rainfall losses	47.190	6.330	47.190	mm"
"	Runoff depth	28.391	69.250	28.391	mm"
"	Runoff volume	68.14	0.00	68.14	c.m"
"	Runoff coefficient	0.376	0.000	0.376	"
"	Maximum flow	0.026	0.000	0.026	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.026	0.026	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.026	0.026	0.026	0.000"
" 40	HYDROGRAPH Combine 2"				
"	6 Combine "				
"	2 Node #"				
"	To Walser Street"				
"	Maximum flow		0.026	c.m/sec"	
"	Hydrograph volume		68.139	c.m"	
"		0.026	0.026	0.026	0.026"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.026	0.000	0.026	0.026"
" 33	CATCHMENT 10"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	10 Catchment 10"				
"	0.000 % Impervious"				
"	7.760 Total Area"				
"	150.000 Flow length"				
"	2.000 Overland Slope"				
"	7.760 Pervious Area"				
"	150.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	150.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.377 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.454	0.000	0.026	0.026 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	41.437	5.102	41.437	minutes"
"	Time to Centroid	143.191	90.751	143.190	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	5865.07	0.01	5865.07	c.m"
"	Rainfall losses	47.093	5.908	47.093	mm"
"	Runoff depth	28.488	69.673	28.488	mm"
"	Runoff volume	2210.64	0.01	2210.65	c.m"
"	Runoff coefficient	0.377	0.000	0.377	"
"	Maximum flow	0.454	0.000	0.454	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.454	0.454	0.026	0.026"	
" 33	CATCHMENT 11"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	11 Catchment 11"				
"	0.000 % Impervious"				
"	0.130 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.130 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.376 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.012	0.454	0.026	0.026 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	18.749	2.308	18.749	minutes"
"	Time to Centroid	117.510	87.059	117.510	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	98.25	0.00	98.26	c.m"

"	Rainfall losses	47.127	6.593	47.127	mm"
"	Runoff depth	28.453	68.988	28.453	mm"
"	Runoff volume	36.99	0.00	36.99	c.m"
"	Runoff coefficient	0.376	0.000	0.376	"
"	Maximum flow	0.012	0.000	0.012	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.012	0.461	0.026	0.026"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.377 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.584	0.461	0.026	0.026 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	23.913	2.944	23.913	minutes"
"	Time to Centroid	123.357	87.974	123.357	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	5381.35	0.01	5381.36	c.m"
"	Rainfall losses	47.108	6.942	47.107	mm"
"	Runoff depth	28.473	68.639	28.473	mm"
"	Runoff volume	2027.30	0.00	2027.30	c.m"
"	Runoff coefficient	0.377	0.000	0.377	"
"	Maximum flow	0.584	0.000	0.584	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.584	1.001	0.026	0.026"
" 54	POND DESIGN"				
"	1.001 Current peak flow	c.m/sec"			

```

"      0.050 Target outflow      c.m/sec"
"      4274.9 Hydrograph volume    c.m"
"      6. Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650     402.200"
"          410.000      3.601     2187.900"
"          410.250      7.811     5318.900"
"          410.500     12.984     9642.300"
"          410.750     18.965     15227.70"
"          Peak outflow              0.886      c.m/sec"
"          Maximum level              409.769     metre"
"          Maximum storage            536.694      c.m"
"          Centroidal lag              2.394     hours"
"          0.584      1.001      0.886      0.026 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.584      0.886      0.886      0.026"
" 52 CHANNEL DESIGN"
"      0.886 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.374     metre"
"          Velocity                    0.946     m/sec"
"          Channel capacity            10.655     c.m/sec"
"          Critical depth              0.324     metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.407 X-factor <= 0.5"
"      57.429 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              0.880      c.m/sec"
"          0.584      0.886      0.880      0.026 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.584      0.880      0.880      0.026"

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" 52      CHANNEL DESIGN"
"      0.880    Current peak flow      c.m/sec"
"      0.035    Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000    Basewidth      metre"
"      2.950    Left bank slope"
"      3.000    Right bank slope"
"      0.950    Channel depth      metre"
"      1.040    Gradient      %"
"      Depth of flow      0.291      metre"
"      Velocity      1.055      m/sec"
"      Channel capacity      9.246      c.m/sec"
"      Critical depth      0.239      metre"
" 53      ROUTE      Channel Route 40"
"      39.80      Channel Route 40 Reach length      ( metre)"
"      0.310      X-factor <= 0.5"
"      28.289      K-lag      ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag      ( seconds)"
"      0.500      Beta weighting factor"
"      37.500      Routing time step      ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow      0.877      c.m/sec"
"      0.584      0.880      0.877      0.026 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.584      0.877      0.877      0.026"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.584      0.877      0.877      0.026"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      0.877      c.m/sec"
"      Hydrograph volume      4274.952      c.m"
"      0.584      0.877      0.877      0.877"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.584      0.000      0.877      0.877"
" 33      CATCHMENT 20"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      20      Catchment 20"
"      0.000      % Impervious"
"      6.650      Total Area"
"      150.000      Flow length"
"      2.000      Overland Slope"

```

"	6.650	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.377	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.389	0.000	0.877	0.877 c.m/sec"	
"	Catchment 20	Pervious	Impervious	Total Area	"
"	Surface Area	6.650	0.000	6.650	hectare"
"	Time of concentration	41.437	5.102	41.437	minutes"
"	Time to Centroid	143.191	90.751	143.190	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	5026.12	0.01	5026.13	c.m"
"	Rainfall losses	47.093	5.908	47.093	mm"
"	Runoff depth	28.488	69.673	28.488	mm"
"	Runoff volume	1894.43	0.00	1894.44	c.m"
"	Runoff coefficient	0.377	0.000	0.377	"
"	Maximum flow	0.389	0.000	0.389	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.389	0.389	0.877	0.877"	
" 33	CATCHMENT 21"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	21 Catchment 20"				
"	10.000 % Impervious"				
"	0.830 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.747 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.083 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.376 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				

"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.913	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.079	0.389	0.877	0.877 c.m/sec"	
"		Catchment 21	Pervious	Impervious	Total Area	"
"		Surface Area	0.747	0.083	0.830	hectare"
"		Time of concentration	18.749	2.308	15.260	minutes"
"		Time to Centroid	117.510	87.059	111.048	minutes"
"		Rainfall depth	75.581	75.581	75.581	mm"
"		Rainfall volume	564.59	62.73	627.32	c.m"
"		Rainfall losses	47.127	6.593	43.074	mm"
"		Runoff depth	28.453	68.988	32.507	mm"
"		Runoff volume	212.55	57.26	269.81	c.m"
"		Runoff coefficient	0.376	0.913	0.430	"
"		Maximum flow	0.069	0.028	0.079	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.079	0.436	0.877	0.877"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.079	0.436	0.436	0.877"	
" 64		SHOW TABLE"				
"	2	Flow hydrograph"				
"	4	Inflow Hydrograph"				
"		Maximum flow		0.436	c.m/sec"	
"		Hydrograph volume		2164.242	c.m"	
" 40		HYDROGRAPH Combine 1"				
"	6	Combine "				
"	1	Node #"				
"		Total"				
"		Maximum flow		1.308	c.m/sec"	
"		Hydrograph volume		6439.194	c.m"	
"		0.079	0.436	0.436	1.308"	
" 38		START/RE-START TOTALS 21"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.730	hectare"	
"		Total Impervious area		0.083	hectare"	
"		Total % impervious		0.365"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__50yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:48:38 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          5089.418 Coefficient A"
"          30.000  Constant B"
"          0.967  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          156.350  mm/hr"
"          Total depth                86.737  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000 Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000 Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.417  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"		0.035	0.000	0.000	0.000 c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	11.375	1.467	11.375	minutes"
"	Time to Centroid	108.305	85.675	108.305	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	208.17	0.00	208.17	c.m"
"	Rainfall losses	50.570	6.561	50.570	mm"
"	Runoff depth	36.167	80.176	36.167	mm"
"	Runoff volume	86.80	0.00	86.80	c.m"
"	Runoff coefficient	0.417	0.000	0.417	"
"	Maximum flow	0.035	0.000	0.035	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.035	0.035	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"		0.035	0.035	0.035	0.000"
" 40	HYDROGRAPH Combine 2"				
"	6 Combine "				
"	2 Node #"				
"	To Walser Street"				
"	Maximum flow		0.035		c.m/sec"
"	Hydrograph volume		86.800		c.m"
"		0.035	0.035	0.035	0.035"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.035	0.000	0.035	0.035"
" 33	CATCHMENT 10"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	10 Catchment 10"				
"	0.000 % Impervious"				
"	7.760 Total Area"				
"	150.000 Flow length"				
"	2.000 Overland Slope"				
"	7.760 Pervious Area"				
"	150.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	150.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.418 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.618	0.000	0.035	0.035 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	38.106	4.916	38.106	minutes"
"	Time to Centroid	138.366	90.175	138.366	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	6730.77	0.01	6730.77	c.m"
"	Rainfall losses	50.510	5.941	50.510	mm"
"	Runoff depth	36.227	80.796	36.227	mm"
"	Runoff volume	2811.21	0.01	2811.22	c.m"
"	Runoff coefficient	0.418	0.000	0.418	"
"	Maximum flow	0.618	0.000	0.618	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.618	0.618	0.035	0.035"	
" 33	CATCHMENT 11"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.417	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.016	0.618	0.035	0.035 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	17.241	2.224	17.241	minutes"
"	Time to Centroid	114.897	86.667	114.896	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	112.76	0.00	112.76	c.m"

"	Rainfall losses	50.540	6.773	50.540	mm"
"	Runoff depth	36.197	79.963	36.197	mm"
"	Runoff volume	47.06	0.00	47.06	c.m"
"	Runoff coefficient	0.417	0.000	0.417	"
"	Maximum flow	0.016	0.000	0.016	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.016 0.628 0.035 0.035"				
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No. "				
"	0.417 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.789 0.628 0.035 0.035 c.m/sec"				
"	Catchment 40 Pervious Impervious Total Area "				
"	Surface Area 7.120 0.000 7.120 hectare"				
"	Time of concentration 21.990 2.837 21.990 minutes"				
"	Time to Centroid 120.254 87.552 120.254 minutes"				
"	Rainfall depth 86.737 86.737 86.737 mm"				
"	Rainfall volume 6175.65 0.01 6175.66 c.m"				
"	Rainfall losses 50.538 7.307 50.538 mm"				
"	Runoff depth 36.199 79.429 36.199 mm"				
"	Runoff volume 2577.38 0.01 2577.39 c.m"				
"	Runoff coefficient 0.417 0.000 0.417 "				
"	Maximum flow 0.789 0.000 0.789 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.789 1.341 0.035 0.035"				
" 54	POND DESIGN"				
"	1.341 Current peak flow c.m/sec"				

```

"      0.050 Target outflow      c.m/sec"
"      5435.7 Hydrograph volume    c.m"
"      6. Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650      402.200"
"          410.000      3.601      2187.900"
"          410.250      7.811      5318.900"
"          410.500      12.984      9642.300"
"          410.750      18.965      15227.70"
"          Peak outflow              1.183      c.m/sec"
"          Maximum level              409.794      metre"
"          Maximum storage              718.586      c.m"
"          Centroidal lag              2.328      hours"
"          0.789      1.341      1.183      0.035 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.789      1.183      1.183      0.035"
" 52 CHANNEL DESIGN"
"      1.183 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.417      metre"
"          Velocity              1.016      m/sec"
"          Channel capacity              10.655      c.m/sec"
"          Critical depth              0.364      metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.396 X-factor <= 0.5"
"      53.425 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              1.180      c.m/sec"
"          0.789      1.183      1.180      0.035 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.789      1.180      1.180      0.035"

```

```

" 52      CHANNEL DESIGN"
"      1.180    Current peak flow      c.m/sec"
"      0.035    Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000    Basewidth      metre"
"      2.950    Left bank slope"
"      3.000    Right bank slope"
"      0.950    Channel depth      metre"
"      1.040    Gradient      %"
"      Depth of flow      0.341      metre"
"      Velocity      1.150      m/sec"
"      Channel capacity      9.246      c.m/sec"
"      Critical depth      0.283      metre"
" 53      ROUTE      Channel Route 40"
"      39.80      Channel Route 40 Reach length      ( metre)"
"      0.282      X-factor <= 0.5"
"      25.955      K-lag      ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag      ( seconds)"
"      0.500      Beta weighting factor"
"      33.333      Routing time step      ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow      1.178      c.m/sec"
"      0.789      1.180      1.178      0.035 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.789      1.178      1.178      0.035"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.789      1.178      1.178      0.035"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      1.178      c.m/sec"
"      Hydrograph volume      5435.673      c.m"
"      0.789      1.178      1.178      1.178"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.789      0.000      1.178      1.178"
" 33      CATCHMENT 20"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      20      Catchment 20"
"      0.000      % Impervious"
"      6.650      Total Area"
"      150.000      Flow length"
"      2.000      Overland Slope"

```

"	6.650	Pervious Area"
"	150.000	Pervious length"
"	2.000	Pervious slope"
"	0.000	Impervious Area"
"	150.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	74.000	Pervious SCS Curve No."
"	0.418	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	8.924	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.000	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"
"	0.530	0.000 1.178 1.178 c.m/sec"
"	Catchment 20	Pervious Impervious Total Area "
"	Surface Area	6.650 0.000 6.650 hectare"
"	Time of concentration	38.106 4.916 38.106 minutes"
"	Time to Centroid	138.366 90.175 138.366 minutes"
"	Rainfall depth	86.737 86.737 86.737 mm"
"	Rainfall volume	5767.99 0.01 5768.00 c.m"
"	Rainfall losses	50.510 5.941 50.510 mm"
"	Runoff depth	36.227 80.796 36.227 mm"
"	Runoff volume	2409.09 0.01 2409.10 c.m"
"	Runoff coefficient	0.418 0.000 0.418 "
"	Maximum flow	0.530 0.000 0.530 c.m/sec"
" 40	HYDROGRAPH Add Runoff "	
"	4 Add Runoff "	
"	0.530 0.530 1.178 1.178"	
" 33	CATCHMENT 21"	
"	1 Triangular SCS"	
"	1 Equal length"	
"	1 SCS method"	
"	21 Catchment 20"	
"	10.000 % Impervious"	
"	0.830 Total Area"	
"	40.000 Flow length"	
"	2.000 Overland Slope"	
"	0.747 Pervious Area"	
"	40.000 Pervious length"	
"	2.000 Pervious slope"	
"	0.083 Impervious Area"	
"	40.000 Impervious length"	
"	2.000 Impervious slope"	
"	0.250 Pervious Manning 'n'"	
"	74.000 Pervious SCS Curve No."	
"	0.417 Pervious Runoff coefficient"	
"	0.100 Pervious Ia/S coefficient"	

"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.922	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.104	0.530	1.178	1.178 c.m/sec"	
"		Catchment 21	Pervious	Impervious	Total Area	"
"		Surface Area	0.747	0.083	0.830	hectare"
"		Time of concentration	17.241	2.224	14.282	minutes"
"		Time to Centroid	114.897	86.667	109.333	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	647.92	71.99	719.92	c.m"
"		Rainfall losses	50.540	6.774	46.163	mm"
"		Runoff depth	36.197	79.963	40.574	mm"
"		Runoff volume	270.39	66.37	336.76	c.m"
"		Runoff coefficient	0.417	0.922	0.468	"
"		Maximum flow	0.092	0.031	0.104	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.104	0.593	1.178	1.178"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.104	0.593	0.593	1.178"	
" 64		SHOW TABLE"				
"	2	Flow hydrograph"				
"	4	Inflow Hydrograph"				
"		Maximum flow		0.593	c.m/sec"	
"		Hydrograph volume		2745.860	c.m"	
" 40		HYDROGRAPH Combine 1"				
"	6	Combine "				
"	1	Node #"				
"		Total"				
"		Maximum flow		1.755	c.m/sec"	
"		Hydrograph volume		8181.533	c.m"	
"		0.104	0.593	0.593	1.755"	
" 38		START/RE-START TOTALS 21"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.730	hectare"	
"		Total Impervious area		0.083	hectare"	
"		Total % impervious		0.365"		
" 19		EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__100yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:49:40 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.019 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          168.777  mm/hr"
"          Total depth                97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.452  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.043	0.000	0.000	0.000 c.m/sec"
"	Catchment 30	Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240 hectare"
"	Time of concentration	10.606	1.421	10.606 minutes"
"	Time to Centroid	106.728	85.423	106.728 minutes"
"	Rainfall depth	97.921	97.921	97.921 mm"
"	Rainfall volume	235.01	0.00	235.01 c.m"
"	Rainfall losses	53.628	6.787	53.628 mm"
"	Runoff depth	44.294	91.134	44.294 mm"
"	Runoff volume	106.30	0.00	106.30 c.m"
"	Runoff coefficient	0.452	0.000	0.452 "
"	Maximum flow	0.043	0.000	0.043 c.m/sec"
" 40	HYDROGRAPH Add Runoff "			
"	4	Add Runoff "		
"	0.043	0.043	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"		
"	0.043	0.043	0.043	0.000"
" 40	HYDROGRAPH Combine 2"			
"	6	Combine "		
"	2	Node #"		
"	To Walser Street"			
"	Maximum flow	0.043	c.m/sec"	
"	Hydrograph volume	106.305	c.m"	
"	0.043	0.043	0.043	0.043"
" 40	HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"		
"	0.043	0.000	0.043	0.043"
" 33	CATCHMENT 10"			
"	1	Triangular SCS"		
"	1	Equal length"		
"	1	SCS method"		
"	10	Catchment 10"		
"	0.000	% Impervious"		
"	7.760	Total Area"		
"	150.000	Flow length"		
"	2.000	Overland Slope"		
"	7.760	Pervious Area"		
"	150.000	Pervious length"		
"	2.000	Pervious slope"		
"	0.000	Impervious Area"		
"	150.000	Impervious length"		
"	2.000	Impervious slope"		
"	0.250	Pervious Manning 'n'"		
"	74.000	Pervious SCS Curve No."		
"	0.454	Pervious Runoff coefficient"		
"	0.100	Pervious Ia/S coefficient"		
"	8.924	Pervious Initial abstraction"		
"	0.015	Impervious Manning 'n'"		
"	98.000	Impervious SCS Curve No."		

"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.801	0.000	0.043	0.043 c.m/sec"	
"	Catchment 10	Pervious	Impervious	Total Area	"
"	Surface Area	7.760	0.000	7.760	hectare"
"	Time of concentration	35.531	4.759	35.530	minutes"
"	Time to Centroid	134.554	89.737	134.554	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	7598.69	0.01	7598.69	c.m"
"	Rainfall losses	53.501	6.084	53.501	mm"
"	Runoff depth	44.420	91.837	44.420	mm"
"	Runoff volume	3447.00	0.01	3447.01	c.m"
"	Runoff coefficient	0.454	0.000	0.454	"
"	Maximum flow	0.801	0.000	0.801	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.801	0.801	0.043	0.043"	
" 33	CATCHMENT 11"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.453	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.020	0.801	0.043	0.043 c.m/sec"	
"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	16.076	2.153	16.076	minutes"
"	Time to Centroid	112.853	86.345	112.853	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	127.30	0.00	127.30	c.m"

"	Rainfall losses	53.605	6.948	53.605	mm"
"	Runoff depth	44.316	90.973	44.316	mm"
"	Runoff volume	57.61	0.00	57.61	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.020	0.000	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.020	0.813	0.043	0.043"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	74.000 Pervious SCS Curve No. "				
"	0.453 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.985	0.813	0.043	0.043 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	20.504	2.747	20.504	minutes"
"	Time to Centroid	117.777	87.189	117.777	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	6971.99	0.01	6972.00	c.m"
"	Rainfall losses	53.532	7.496	53.532	mm"
"	Runoff depth	44.389	90.426	44.389	mm"
"	Runoff volume	3160.50	0.01	3160.50	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.985	0.000	0.985	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.985	1.721	0.043	0.043"
" 54	POND DESIGN"				
"	1.721 Current peak flow	c.m/sec"			

```

"      0.050 Target outflow      c.m/sec"
"      6665.1 Hydrograph volume    c.m"
"      6. Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          409.630      0.000      0.000"
"          409.750      0.6650      402.200"
"          410.000      3.601      2187.900"
"          410.250      7.811      5318.900"
"          410.500      12.984      9642.300"
"          410.750      18.965      15227.70"
"          Peak outflow              1.507      c.m/sec"
"          Maximum level              409.822      metre"
"          Maximum storage              917.762      c.m"
"          Centroidal lag              2.275      hours"
"          0.985      1.721      1.507      0.043 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.985      1.507      1.507      0.043"
" 52 CHANNEL DESIGN"
"      1.507 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"          Depth of flow              0.456      metre"
"          Velocity              1.080      m/sec"
"          Channel capacity              10.655      c.m/sec"
"          Critical depth              0.400      metre"
" 53 ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length ( metre)"
"      0.386 X-factor <= 0.5"
"      50.288 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"          Peak outflow              1.499      c.m/sec"
"          0.985      1.507      1.499      0.043 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.985      1.499      1.499      0.043"

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" 52      CHANNEL DESIGN"
"      1.499    Current peak flow      c.m/sec"
"      0.035    Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000    Basewidth      metre"
"      2.950    Left bank slope"
"      3.000    Right bank slope"
"      0.950    Channel depth      metre"
"      1.040    Gradient      %"
"      Depth of flow      0.386      metre"
"      Velocity      1.232      m/sec"
"      Channel capacity      9.246      c.m/sec"
"      Critical depth      0.325      metre"
" 53      ROUTE      Channel Route 40"
"      39.80      Channel Route 40 Reach length      ( metre)"
"      0.256      X-factor <= 0.5"
"      24.228      K-lag      ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag      ( seconds)"
"      0.500      Beta weighting factor"
"      33.333      Routing time step      ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow      1.499      c.m/sec"
"      0.985      1.499      1.499      0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.985      1.499      1.499      0.043"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.985      1.499      1.499      0.043"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      1.499      c.m/sec"
"      Hydrograph volume      6665.152      c.m"
"      0.985      1.499      1.499      1.499"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.985      0.000      1.499      1.499"
" 33      CATCHMENT 20"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      20      Catchment 20"
"      0.000      % Impervious"
"      6.650      Total Area"
"      150.000      Flow length"
"      2.000      Overland Slope"

```

"	6.650	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.454	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.687	0.000	1.499	1.499 c.m/sec"	
"	Catchment 20	Pervious	Impervious	Total Area	"
"	Surface Area	6.650	0.000	6.650	hectare"
"	Time of concentration	35.531	4.759	35.530	minutes"
"	Time to Centroid	134.554	89.737	134.554	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	6511.76	0.01	6511.77	c.m"
"	Rainfall losses	53.501	6.084	53.501	mm"
"	Runoff depth	44.420	91.837	44.420	mm"
"	Runoff volume	2953.94	0.01	2953.94	c.m"
"	Runoff coefficient	0.454	0.000	0.454	"
"	Maximum flow	0.687	0.000	0.687	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.687	0.687	1.499	1.499"	
" 33	CATCHMENT 21"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	21 Catchment 20"				
"	10.000 % Impervious"				
"	0.830 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.747 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.083 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.453 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				

"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.929	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.135	0.687	1.499	1.499 c.m/sec"	
"		Catchment 21	Pervious	Impervious	Total Area	"
"		Surface Area	0.747	0.083	0.830	hectare"
"		Time of concentration	16.076	2.153	13.490	minutes"
"		Time to Centroid	112.853	86.345	107.930	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	731.47	81.27	812.75	c.m"
"		Rainfall losses	53.605	6.948	48.939	mm"
"		Runoff depth	44.316	90.973	48.982	mm"
"		Runoff volume	331.04	75.51	406.55	c.m"
"		Runoff coefficient	0.453	0.929	0.500	"
"		Maximum flow	0.117	0.034	0.135	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.135	0.765	1.499	1.499"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.135	0.765	0.765	1.499"	
" 64		SHOW TABLE"				
"	2	Flow hydrograph"				
"	4	Inflow Hydrograph"				
"		Maximum flow		0.765	c.m/sec"	
"		Hydrograph volume		3360.493	c.m"	
" 40		HYDROGRAPH Combine 1"				
"	6	Combine "				
"	1	Node #"				
"		Total"				
"		Maximum flow		2.244	c.m/sec"	
"		Hydrograph volume		10025.642	c.m"	
"		0.135	0.765	0.765	2.244"	
" 38		START/RE-START TOTALS 21"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.730	hectare"	
"		Total Impervious area		0.083	hectare"	
"		Total % impervious		0.365"		
" 19		EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Ex__REG.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:51:35 PM"
" 31      TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Historic"
"          5      Historic"
"          2880.000 Duration"
"          48.000  Rainfall intensity values"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.026      2.026      2.026      2.028"
"                  2.026      6.000      4.000      6.000      13.000"
"                  17.000      13.000      23.000      13.000      13.000"
"                  53.000      38.000      13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
"          6 000hyd Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          30 Catchment 30"
"          0.000 % Impervious"
"          0.240 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.240 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.713 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"

```

"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.027	0.000	0.000	0.000 c.m/sec"
"		Catchment 30	Pervious	Impervious	Total Area "
"		Surface Area	0.240	0.000	0.240 hectare"
"		Time of concentration	12.633	2.243	12.633 minutes"
"		Time to Centroid	2530.545	2290.972	2530.545 minutes"
"		Rainfall depth	285.000	285.000	285.000 mm"
"		Rainfall volume	684.00	0.00	684.00 c.m"
"		Rainfall losses	81.839	43.972	81.839 mm"
"		Runoff depth	203.161	241.028	203.161 mm"
"		Runoff volume	487.58	0.00	487.59 c.m"
"		Runoff coefficient	0.713	0.000	0.713 "
"		Maximum flow	0.027	0.000	0.027 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.027	0.027	0.000	0.000"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.027	0.027	0.027	0.000"
" 40		HYDROGRAPH Combine 2"			
"	6	Combine "			
"	2	Node #"			
"		To Walser Street"			
"		Maximum flow	0.027		c.m/sec"
"		Hydrograph volume	487.585		c.m"
"		0.027	0.027	0.027	0.027"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.027	0.000	0.027	0.027"
" 33		CATCHMENT 10"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	10	Catchment 10"			
"	0.000	% Impervious"			
"	7.760	Total Area"			
"	150.000	Flow length"			
"	2.000	Overland Slope"			
"	7.760	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			

"	74.000	Pervious SCS Curve No."			
"	0.714	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.881	0.000	0.027	0.027 c.m/sec"
"		Catchment 10	Pervious	Impervious	Total Area "
"		Surface Area	7.760	0.000	7.760 hectare"
"		Time of concentration	42.319	7.513	42.319 minutes"
"		Time to Centroid	2572.242	2276.224	2572.241 minutes"
"		Rainfall depth	285.000	285.000	285.000 mm"
"		Rainfall volume	2.2116	0.0000	2.2116 ha-m"
"		Rainfall losses	81.644	25.621	81.644 mm"
"		Runoff depth	203.356	259.379	203.356 mm"
"		Runoff volume	1.5780	0.0000	1.5780 ha-m"
"		Runoff coefficient	0.714	0.000	0.714 "
"		Maximum flow	0.881	0.000	0.881 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.881	0.881	0.027	0.027"
" 33		CATCHMENT 11"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	11	Catchment 11"			
"	0.000	% Impervious"			
"	0.130	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.130	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.723	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.014	0.881	0.027	0.027 c.m/sec"

"	Catchment 11	Pervious	Impervious	Total Area	"
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	19.148	3.399	19.148	minutes"
"	Time to Centroid	2545.193	2266.333	2545.193	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	370.50	0.00	370.50	c.m"
"	Rainfall losses	78.940	42.646	78.940	mm"
"	Runoff depth	206.060	242.354	206.060	mm"
"	Runoff volume	267.88	0.00	267.88	c.m"
"	Runoff coefficient	0.723	0.000	0.723	"
"	Maximum flow	0.014	0.000	0.014	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

"	4	Add Runoff "			
"		0.014	0.894	0.027	0.027"

" 33 CATCHMENT 40"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	40	Catchment 40"
"	0.000	% Impervious"
"	7.120	Total Area"
"	60.000	Flow length"
"	2.000	Overland Slope"
"	7.120	Pervious Area"
"	60.000	Pervious length"
"	2.000	Pervious slope"
"	0.000	Impervious Area"
"	60.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	74.000	Pervious SCS Curve No."
"	0.716	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	8.924	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.000	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"		0.772	0.894	0.027	0.027 c.m/sec"
---	--	-------	-------	-------	----------------

"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	24.421	4.336	24.421	minutes"
"	Time to Centroid	2549.942	2258.969	2549.942	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	2.0292	0.0000	2.0292	ha-m"
"	Rainfall losses	80.848	39.404	80.848	mm"
"	Runoff depth	204.152	245.596	204.152	mm"
"	Runoff volume	1.4536	0.0000	1.4536	ha-m"
"	Runoff coefficient	0.716	0.000	0.716	"

"		Maximum flow	0.772	0.000	0.772	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.772 1.667 0.027 0.027"				
" 54		POND DESIGN"				
"	1.667	Current peak flow	c.m/sec"			
"	0.050	Target outflow	c.m/sec"			
"	30583.9	Hydrograph volume	c.m"			
"	6.	Number of stages"				
"	409.630	Minimum water level	metre"			
"	410.750	Maximum water level	metre"			
"	409.630	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	409.630	0.000	0.000"			
"	409.750	0.6650	402.200"			
"	410.000	3.601	2187.900"			
"	410.250	7.811	5318.900"			
"	410.500	12.984	9642.300"			
"	410.750	18.965	15227.70"			
"		Peak outflow	1.612	c.m/sec"		
"		Maximum level	409.831	metre"		
"		Maximum storage	977.909	c.m"		
"		Centroidal lag	42.858	hours"		
"		0.772 1.667 1.612 0.027	c.m/sec"			
" 40		HYDROGRAPH Next link "				
"	5	Next link "				
"		0.772 1.612 1.612 0.027"				
" 52		CHANNEL DESIGN"				
"	1.612	Current peak flow	c.m/sec"			
"	0.035	Manning 'n'"				
"	0.	Cross-section type: 0=trapezoidal; 1=general"				
"	0.000	Basewidth	metre"			
"	7.410	Left bank slope"				
"	6.000	Right bank slope"				
"	0.950	Channel depth	metre"			
"	1.040	Gradient	%"			
"		Depth of flow	0.468	metre"		
"		Velocity	1.098	m/sec"		
"		Channel capacity	10.655	c.m/sec"		
"		Critical depth	0.411	metre"		
" 53		ROUTE Channel Route 72"				
"	72.40	Channel Route 72 Reach length	(metre)"			
"	0.383	X-factor <= 0.5"				
"	49.448	K-lag (seconds)"				
"	0.000	Default(0) or user spec.(1) values used"				
"	0.500	X-factor <= 0.5"				
"	30.000	K-lag (seconds)"				
"	0.500	Beta weighting factor"				
"	60.000	Routing time step (seconds)"				

```

"          1  No. of sub-reaches"
"          Peak outflow          1.606    c.m/sec"
"          0.772    1.612    1.606    0.027 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.772    1.606    1.606    0.027"
" 52      CHANNEL DESIGN"
"          1.606    Current peak flow    c.m/sec"
"          0.035    Manning 'n'"
"          0.      Cross-section type: 0=trapezoidal; 1=general"
"          2.000    Basewidth    metre"
"          2.950    Left bank slope"
"          3.000    Right bank slope"
"          0.950    Channel depth    metre"
"          1.040    Gradient    %"
"          Depth of flow          0.401    metre"
"          Velocity          1.256    m/sec"
"          Channel capacity          9.246    c.m/sec"
"          Critical depth          0.339    metre"
" 53      ROUTE    Channel Route 40"
"          39.80    Channel Route 40 Reach length    ( metre)"
"          0.248    X-factor <= 0.5"
"          23.758    K-lag    ( seconds)"
"          0.000    Default(0) or user spec.(1) values used"
"          0.500    X-factor <= 0.5"
"          30.000    K-lag    ( seconds)"
"          0.500    Beta weighting factor"
"          35.644    Routing time step    ( seconds)"
"          1  No. of sub-reaches"
"          Peak outflow          1.603    c.m/sec"
"          0.772    1.606    1.603    0.027 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.772    1.603    1.603    0.027"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          0.772    1.603    1.603    0.027"
" 40      HYDROGRAPH Combine    1"
"          6  Combine "
"          1  Node #"
"          Total"
"          Maximum flow          1.603    c.m/sec"
"          Hydrograph volume          30582.604    c.m"
"          0.772    1.603    1.603    1.603"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.772    0.000    1.603    1.603"
" 33      CATCHMENT 20"
"          1  Triangular SCS"
"          1  Equal length"

```

"	1	SCS method"				
"	20	Catchment 20"				
"	0.000	% Impervious"				
"	6.650	Total Area"				
"	150.000	Flow length"				
"	2.000	Overland Slope"				
"	6.650	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.714	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.755	0.000	1.603	1.603 c.m/sec"
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	6.650	0.000	6.650	hectare"
"		Time of concentration	42.319	7.513	42.319	minutes"
"		Time to Centroid	2572.242	2276.224	2572.241	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1.8952	0.0000	1.8952	ha-m"
"		Rainfall losses	81.644	25.621	81.644	mm"
"		Runoff depth	203.356	259.379	203.356	mm"
"		Runoff volume	1.3523	0.0000	1.3523	ha-m"
"		Runoff coefficient	0.714	0.000	0.714	"
"		Maximum flow	0.755	0.000	0.755	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.755	0.755	1.603	1.603"
" 33		CATCHMENT 21"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	21	Catchment 20"				
"	10.000	% Impervious"				
"	0.830	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.747	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.083	Impervious Area"				

```

"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.723  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.850  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.088      0.755      1.603      1.603 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area "
"      Surface Area      0.747      0.083      0.830      hectare"
"      Time of concentration 19.148      3.399      17.327      minutes"
"      Time to Centroid      2545.193      2266.333      2512.963      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2128.95      236.55      2365.50      c.m"
"      Rainfall losses      78.940      42.646      75.310      mm"
"      Runoff depth      206.060      242.354      209.690      mm"
"      Runoff volume      1539.27      201.15      1740.42      c.m"
"      Runoff coefficient      0.723      0.850      0.736      "
"      Maximum flow      0.079      0.010      0.088      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.088      0.841      1.603      1.603"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.088      0.841      0.841      1.603"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow          0.841      c.m/sec"
"      Hydrograph volume      15263.571      c.m"
" 40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"          Total"
"      Maximum flow          2.444      c.m/sec"
"      Hydrograph volume      45846.176      c.m"
"          0.088      0.841      0.841      2.444"
" 38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"      Total Catchment area          22.730      hectare"
"      Total Impervious area          0.083      hectare"
"      Total % impervious          0.365"
" 19      EXIT"

```

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411009
March 2023

Catchment 1000 : Stormwater Management Facility No. 1

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
411.00	0.00	14.8	0	0.0	CB.1 Lip
411.10	0.10	353.3	18	18.4	
411.20	0.20	1,188.7	77	95.5	
411.30	0.30	1,699.2	144	239.9	
411.40	0.40	2,215.9	196	435.7	CB.2 Lip
411.50	0.50	2,897.0	256	691.3	
411.60	0.60	3,828.7	336	1,027.6	
411.70	0.70	4,620.0	422	1,450.0	
411.80	0.80	5,096.6	486	1,935.8	Weir
411.85	0.85	5,297.4	260	2,195.7	
411.90	0.90	5,498.2	270	2,465.6	
412.00	1.00	5,792.6	565	3,030.1	Top of bank

Outlet #1

450 mm diameter pipe
250 mm orifice

Q = 0.133 m³/s
Cd = 0.600
H = 1.045 m
2g = 19.620

A = 0.049 m²
D = 0.250 m
D/2 = 0.125 m
Invert = 410.83

Outlet #2

525 mm diameter pipe
350 mm orifice

Q = 0.246 m³/s
Cd = 0.600
H = 0.925 m
2g = 19.620

A = 0.096 m²
D = 0.350 m
D/2 = 0.175 m
Invert = 410.9

Overflow Weir

Elevation = 411.85

d1 = 1.00 m
h = 0.85 m
H = 0.15 m
2g = 19.620
L = 20.00 m

Q = 1.638 m³/s

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1	Outlet #2	Overflow	Actual	
(m)	(m)	(m³)	250 mm	350mm	Weir	Discharge	
			(m³/s)	(m³/s)	(m³/s)	(m³/s)	
411.00	0.00	0.0	0.000	0.000	0.000	0.000	CB.1 Lip
411.10	0.10	18.4	0.032	0.000	0.000	0.032	
411.20	0.20	95.5	0.065	0.000	0.000	0.065	
411.30	0.30	239.9	0.077	0.000	0.000	0.077	
411.40	0.40	435.7	0.087	0.000	0.000	0.087	CB.2 Lip
411.50	0.50	691.3	0.096	0.000	0.000	0.096	
411.60	0.60	1,027.6	0.105	0.000	0.000	0.105	
411.70	0.70	1,450.0	0.113	0.202	0.000	0.315	
411.80	0.80	1,935.8	0.120	0.218	0.000	0.338	Weir
411.85	0.85	2,195.7	0.123	0.225	0.000	0.349	
411.90	0.90	2,465.6	0.127	0.232	0.308	0.667	
412.00	1.00	3,030.1	0.133	0.246	1.638	2.018	Top of bank

**Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411009
March 2023**

Catchment 2100 : Stormwater Management Facility No. 2

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Quality Storage	Accum. Quality Storage	Increm. Active Storage	Accum. Active Storage	
(m)	(m)	(m ²)	(m ³)	(m ³)	(m ³)	(m ³)	
410.51	0.00	795.09	0.00	0.00			Outlet Pipe Invert
410.65	0.14	835.70	114.16	114.16	0	0.0	Pond Bottom/Knockout
410.70	0.19	888.00			43	43.1	
410.80	0.29	941.90			91	134.6	
410.90	0.39	996.40			97	231.5	
411.00	0.49	1052.40			102	333.9	
411.10	0.59	1109.60			108	442.0	CB Lip
411.20	0.69	1167.90			114	555.9	
411.30	0.79	1227.50			120	675.7	
411.40	0.89	1288.15			126	801.5	
411.50	0.99	1350.10			132	933.4	
411.60	1.09	1413.30			138	1,071.6	
411.65	1.14	1444.90			71	1,143.0	Weir
411.70	1.19	1476.50			73	1,216.0	
411.80	1.29	1559.40			152	1,367.8	Top of bank

Outlet #1

150 mm Diameter Knockout

$$\begin{aligned} Q &= 0.032 \text{ m}^3/\text{s} \\ Cd &= 0.600 \\ H &= 0.475 \text{ m} \\ 2g &= 19.620 \end{aligned}$$

$$\begin{aligned} A &= 0.018 \text{ m}^2 \\ D &= 0.150 \text{ m} \\ D/2 &= 0.075 \text{ m} \end{aligned}$$

Outlet #2

300 mm diameter orifice

$$\begin{aligned} Q &= 0.201 \text{ m}^3/\text{s} \\ Cd &= 0.600 \\ H &= 0.940 \text{ m} \\ 2g &= 19.620 \end{aligned}$$

$$\begin{aligned} A &= 0.071 \text{ m}^2 \\ D &= 0.300 \text{ m} \\ D/2 &= 0.150 \text{ m} \\ \text{Invert} &= 410.51 \end{aligned}$$

Overflow Weir

Elevation = 411.65

$$\begin{aligned} d1 &= 1.15 \text{ m} \\ h &= 1.00 \text{ m} \\ H &= 0.15 \text{ m} \\ 2g &= 19.62 \\ L &= 10.00 \text{ m} \end{aligned}$$

$$Q = 0.815 \text{ m}^3/\text{s}$$

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1 150 mm	Outlet #2 300 mm	Overflow Weir	Actual Discharge	
(m)	(m)	(m ³)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
410.65	0.00	0.0	0.000	0.000	0.000	0.000	Pond Bottom/Knockout
410.70	0.05	43.1	0.006	0.000	0.000	0.006	
410.80	0.15	134.6	0.013	0.000	0.000	0.013	
410.90	0.25	231.5	0.020	0.000	0.000	0.020	
411.00	0.35	333.9	0.025	0.000	0.000	0.025	
411.10	0.45	442.0	0.029	0.000	0.000	0.029	CB Lip
411.20	0.55	555.9	0.000	0.138	0.000	0.138	
411.30	0.65	675.7	0.000	0.150	0.000	0.150	
411.40	0.75	801.5	0.000	0.162	0.000	0.162	
411.50	0.85	933.4	0.000	0.172	0.000	0.172	
411.60	0.95	1,071.6	0.000	0.182	0.000	0.182	
411.65	1.00	1,143.0	0.000	0.187	0.000	0.187	Weir
411.70	1.05	1,216.0	0.000	0.192	0.153	0.345	
411.80	1.15	1,367.8	0.000	0.201	0.815	1.015	Top of bank

AINLEY SUBDIVISION
TOWNSHIP OF CENTRE WELLINGTON
Our File: 411009
March 14, 2023

Catchment 3200: Proposed Super Pipe

Pipe Volumes

From	To	Length (m)	Diameter (m)	Volume (m ³)	Initial Invert (Approx)	Final Invert (Approx)	Initial Obvert	Final Obvert
MH.4	MH.3	27.4	1.2	30.99	410.96	411.23	412.16	412.43
MH.3	CBMH.1	28.3	1.2	32.01	410.65	410.93	411.85	412.13
Total				63.00				

Structure Volumes

	Area	Inv.	T/G	Volume (m ³) - up to Weir Depth or Top of Grate
CBMH.1	1.13	410.62	413.98	3.6
MH.3	1.13	410.93	414.66	3.3
MH.4	1.13	411.23	415.44	2.9
Total				9.8

Total Volume from Pipes and Structures
 (Up to Weir Elevation of 413.98)

72.84

ELEV	INC. DEPTH	PIPE + MANHOLE STORAGE VOL	ACCUM STORAGE VOL	
(m)	(m)	(m ³)	(m ³)	
410.62	0.00	0.00	0.00	Orifice Invert
410.87	0.25	4.85	4.85	
411.12	0.50	9.50	14.35	
411.37	0.75	9.72	24.07	
411.62	1.00	9.85	33.92	
411.87	1.25	9.85	43.77	
412.12	1.50	9.85	53.61	
412.37	1.75	9.85	63.46	
412.62	2.00	9.85	73.31	Pipe Obvert
412.87	2.25	0.85	74.16	
413.12	2.50	0.85	75.00	
413.37	2.75	0.85	75.85	
413.62	3.00	0.85	76.70	
413.98	3.36	1.22	77.92	CBMH.43 T/G Weir
414.23	3.61	0.57	78.48	Overflow

ORIFICE CALCULATIONS

Invert = 410.62 m
 Q = 0.057 m³/s
 Cd = 0.6
 H = 3.55 m
 2g = 19.62
 A = 0.011 m²
 D = 0.120 m

OVERFLOW WEIR

Q = 0.104 cu m/s
 d1 = 3.610 m
 h = 3.360 m
 H = 0.250 m
 2g = 19.620
 L = 0.600 m

Super Pipe (continued)

ELEVATION	STAGE (m)	STORAGE (cu m)	ORIFICE DISCHARGE (cu m/s)	WEIR DISCHARGE (cu m/s)	TOTAL DISCHARGE (cu m/s)	
410.62	0.000	0.00	0.000	0.000	0.000	Orifice Invert
410.87	0.250	4.85	0.013	0.000	0.013	
411.12	0.500	14.35	0.020	0.000	0.020	
411.37	0.750	24.07	0.025	0.000	0.025	
411.62	1.000	33.92	0.029	0.000	0.029	
411.87	1.250	43.77	0.033	0.000	0.033	
412.12	1.500	53.61	0.036	0.000	0.036	Pipe Obvert
412.37	1.750	63.46	0.039	0.000	0.039	
412.62	2.000	73.31	0.042	0.000	0.042	
412.87	2.250	74.16	0.044	0.000	0.044	
413.12	2.500	75.00	0.047	0.000	0.047	
413.37	2.750	75.85	0.049	0.000	0.049	
413.62	3.000	76.70	0.052	0.000	0.052	
413.98	3.360	77.92	0.055	0.000	0.055	CBMH.43 T/G Weir
414.23	3.610	78.48	0.057	0.104	0.160	Overflow

**Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411009
March 2023**

Catchment 4000: Wetland

Stage Storage Volume Calculations					
Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
409.63	0.00	2,833	0	0.0	Wetland Bottom
409.75	0.12	3,871	402	402.2	
410.00	0.37	10,414	1786	2,187.9	
410.25	0.62	14,634	3131	5,318.9	
410.50	0.87	19,953	4323	9,642.3	
410.75	1.12	24,730	5585	15,227.7	Overflow

WEIR CALCULATIONS

H = 1.12 m

L = 10 m

Q = 18.965 m³/s

Stage/Storage/Discharge Table				
Elevation	Stage	Storage	Actual Discharge	
(m)	(m)	(m³)	(m³/s)	
409.63	0.00	0.0	0.000	Wetland Bottom
409.75	0.12	402.2	0.665	
410.00	0.37	2,187.9	3.601	
410.25	0.62	5,318.9	7.811	
410.50	0.87	9,642.3	12.984	
410.75	1.12	15,227.7	18.965	Overflow

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411009
March 2023

Catchment 2200 : Private Stormwater Management Facility Multi-Family Block

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m ²)	(m ³)	(m ³)	
413.70	0.00	847	0	0.0	Bottom of Pond/Knockout
413.80	0.10	924	89	88.6	
413.90	0.20	1,049	99	187.2	
414.00	0.30	1,175	111	298.4	
414.10	0.40	1,302	124	422.2	
414.20	0.50	1,431	137	558.9	
414.30	0.60	1,561	150	708.5	
414.40	0.70	1,692	163	871.1	CB Lip
414.50	0.80	1,825	176	1,046.9	
414.60	0.90	1,959	189	1,236.1	
414.70	1.00	2,094	203	1,438.7	Weir
415.00	1.30	2,231	649	2,087.4	Top of bank

Outlet #1
110 mm Diameter Knockout

$$\begin{aligned} Q &= 0.020 \text{ m}^3/\text{s} \\ Cd &= 0.600 \\ H &= 0.645 \text{ m} \\ 2g &= 19.620 \\ A &= 0.010 \text{ m}^2 \\ D &= 0.110 \text{ m} \\ D/2 &= 0.055 \text{ m} \end{aligned}$$

Outlet #2
450 mm diameter pipe

$$\begin{aligned} Q &= 0.395 \text{ m}^3/\text{s} \\ Cd &= 0.600 \\ H &= 0.875 \text{ m} \\ 2g &= 19.620 \\ A &= 0.159 \text{ m}^2 \\ D &= 0.450 \text{ m} \\ D/2 &= 0.225 \text{ m} \\ \text{Invert} &= 413.6 \end{aligned}$$

Overflow Weir
Elevation = 414.70

$$\begin{aligned} d1 &= 1.30 \text{ m} \\ h &= 1.00 \text{ m} \\ H &= 0.30 \text{ m} \\ 2g &= 19.62 \\ L &= 10.00 \text{ m} \\ Q &= 2.369 \text{ m}^3/\text{s} \end{aligned}$$

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1	Outlet #2	Overflow	Actual	
(m)	(m)	(m ³)	110 mm	450 mm	Weir	Discharge	
			(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
413.70	0.00	0.0	0.000	0.000	0.000	0.000	Bottom of Pond/Knockout
413.80	0.10	88.6	0.005	0.000	0.000	0.005	
413.90	0.20	187.2	0.010	0.000	0.000	0.010	
414.00	0.30	298.4	0.013	0.000	0.000	0.013	
414.10	0.40	422.2	0.015	0.000	0.000	0.015	CB Lip
414.20	0.50	558.9	0.000	0.259	0.000	0.259	
414.30	0.60	708.5	0.000	0.291	0.000	0.291	
414.40	0.70	871.1	0.000	0.321	0.000	0.321	
414.50	0.80	1,046.9	0.000	0.347	0.000	0.347	
414.60	0.90	1,236.1	0.000	0.372	0.000	0.372	
414.70	1.00	1,438.7	0.000	0.395	0.000	0.395	Weir
415.00	1.30	2,087.4	0.000	0.458	2.369	2.828	Top of bank

Ainley Farm Subdivision
Our File: 411009
March 14, 2023

Catchment 1200: Proposed Infiltration Gallery

STORAGE VOLUME CALCULATIONS

ELEV	INC	SURFACE	INCR.	INCR.	ACCUM
	D	AREA	PIPE	GALLERY	STORAGE
(m)	(m)	(Infil. Gall) (sq m)	STORAGE VOL (cu m)	STORAGE VOL (cu m)	VOL (cu m)
412.000	0.000	539.90	0.00	0.00	0.00 Bottom of Stone
412.100	0.100	539.90	25.13	9.52	34.65
412.200	0.200	539.90	25.13	9.52	69.31
412.300	0.300	539.90	25.13	9.52	103.96
412.400	0.400	539.90	25.13	9.52	138.61
412.500	0.500	539.90	0.00	17.82	156.43
412.600	0.600	539.90	0.00	17.82	174.25
412.700	0.700	539.90	0.00	17.82	192.06 Top of Stone
412.900	0.900	1.85	0.00	0.37	192.43
413.230	1.230	1.85	0.00	0.61	193.04 CB 46N Lip
413.430	1.430	5.00	0.00	0.68	193.73
413.630	1.630	10.00	0.00	1.50	195.23
413.830	1.830	15.00	0.00	2.50	197.73
414.030	2.030	20.00	0.00	3.50	201.23
414.090	2.090	25.00	0.00	1.35	202.58 RYCB 46O Lip
414.290	2.290	30.00	0.00	5.50	208.08
414.490	2.490	35.00	0.00	6.50	214.58 Overflow

BOTTOM INFILTRATION

L1(dw) =	79.4	m
W1(dw) =	3.5	m
L2(dw) =	52.4	m
W2(dw) =	5.0	m
D(dw) =	0.70	m
A(c) =	539.9	sq m
VOL(dw)=	277.4	cu m
VOL(st)=	92.5	cu m
K =	4	mm/hr
=	1.11E-04	cm/s

SIDE INFILTRATION

ALL SIDES

L1(dw) =	79.4	m
W1(dw) =	3.5	m
L2(dw) =	52.4	m
W2(dw) =	5.0	m
D(dw) =	0.7	m
A(c) =	196.4	sq m
K =	4	mm/hr
=	1.11E-04	cm/s

Weir (1)

Elevation = 413.23

d1 =	2.49	m
h =	1.23	m
H =	1.26	m
2g =	19.620	
L =	0.36	m
Q =	0.799	m ³ /s

Weir (2)

Elevation = 414.09

d1 =	2.49	m
h =	2.09	m
H =	0.40	m
2g =	19.620	
L =	0.36	m
Q =	0.129	m ³ /s

ELEVATION	STAGE (m)	STORAGE (cu m)	INFILTRATION DISCHARGE (cu m/s)	OVERFLOW WEIR (cu m/s)	OVERFLOW WEIR (cu m/s)	TOTAL DISCHARGE (cu m/s)	
412.000	0.000	0.00	0.0003	0.000	0.000	0.0003	Bottom of Stone
412.100	0.100	34.65	0.0003	0.000	0.000	0.0003	
412.200	0.200	69.31	0.0003	0.000	0.000	0.0003	
412.300	0.300	103.96	0.0003	0.000	0.000	0.0003	
412.400	0.400	138.61	0.0003	0.000	0.000	0.0003	
412.500	0.500	156.43	0.0003	0.000	0.000	0.0003	
412.600	0.600	174.25	0.0003	0.000	0.000	0.0003	
412.700	0.700	192.06	0.0003	0.000	0.000	0.0003	Top of Stone
412.900	0.900	192.43	0.0003	0.000	0.000	0.0003	
413.230	1.230	193.04	0.0003	0.000	0.000	0.0003	CB 46N Lip
413.430	1.430	193.73	0.0000	0.045	0.000	0.0453	
413.630	1.630	195.23	0.0000	0.132	0.000	0.1319	
413.830	1.830	197.73	0.0000	0.248	0.000	0.2482	
414.030	2.030	201.23	0.0000	0.390	0.000	0.3899	RYCB 46O Lip
414.090	2.090	202.58	0.0000	0.437	0.000	0.4369	
414.290	2.290	208.08	0.0000	0.608	0.045	0.6524	
414.490	2.490	214.58	0.0000	0.799	0.129	0.9278	Overflow

Ainley Farm Subdivision
Our File: 411009
March 14, 2023

Catchment 1400: Proposed Infiltration Gallery

STORAGE VOLUME CALCULATIONS

ELEV	INC	SURFACE	INCR.	INCR.	ACCUM	
	D	AREA	PIPE	GALLERY	STORAGE	
(m)	(m)	(sq m)	STORAGE	STORAGE	VOL	
			VOL	VOL	(cu m)	
			(cu m)	(cu m)		
413.920	0.000	790.50	0.00	0.00	0.00	Bottom of Stone
414.020	0.100	790.50	0.00	26.35	26.35	
414.120	0.200	790.50	3.68	25.12	55.15	
414.220	0.300	790.50	3.68	25.12	83.95	
414.320	0.400	790.50	3.68	25.12	112.75	
414.420	0.500	790.50	0.00	26.35	139.10	
414.520	0.600	790.50	0.00	26.35	165.45	
414.620	0.700	790.50	0.00	26.35	191.80	Top of Stone
414.720	0.800	0.72	0.07	0.00	191.87	Pipe Invert
414.820	0.900	0.72	0.07	0.00	191.95	
414.920	1.000	0.72	0.07	0.00	192.02	
415.020	1.100	0.72	0.07	0.00	192.09	
415.120	1.200	0.72	0.07	0.00	192.16	Top of Grate
415.220	1.300	185.00	9.29	0.00	201.45	
415.320	1.400	565.00	37.50	0.00	238.95	
415.420	1.500	750.00	65.75	0.00	304.70	Weir
415.520	1.600	800.00	77.50	0.00	382.20	Overflow

BOTTOM INFILTRATION

L1(dw) = 79.1 m
W1(dw) = 10.0 m
D(dw) = 0.70 m

A(c) = 790.5 sq m
VOL(dw)= 553.4 cu m
VOL(st)= 184.5 cu m
K = 4 mm/hr
= 1.11E-04 cm/s

SIDE INFILTRATION

ALL SIDES

L1(dw) = 79.1 m
W1(dw) = 10.0 m
D(dw) = 0.7 m

A(c) = 124.7 sq m

K = 4 mm/hr
= 1.11E-04 cm/s

Pipe Outlet

300 mm diameter pipe
200 mm orifice
Q = 0.075 m³/s
Cd = 0.6
H = 0.80 m
2g = 19.62

A = 0.031 m²
D = 0.2 m
D/2 = 0.1 m

Overflow Weir

Elevation = 411.90
d1 = 1.600 m
h = 1.500 m
H = 0.100 m
2g = 19.620
L = 5.000 m

Q = 0.218 m³/s

ELEVATION	STAGE (m)	STORAGE (cu m)	INFILTRATION DISCHARGE (cu m/s)	PIPE DISCHARGE (cu m/s)	OVERFLOW WEIR (cu m/s)	TOTAL DISCHARGE (cu m/s)	
413.920	0.000	0.00	0.0009	0.000	0.000	0.0009	Bottom of Stone
414.020	0.100	26.35	0.0009	0.000	0.000	0.0009	
414.120	0.200	55.15	0.0009	0.000	0.000	0.0009	
414.220	0.300	83.95	0.0009	0.000	0.000	0.0009	
414.320	0.400	112.75	0.0009	0.000	0.000	0.0009	
414.420	0.500	139.10	0.0009	0.000	0.000	0.0009	
414.520	0.600	165.45	0.0009	0.000	0.000	0.0009	
414.620	0.700	191.80	0.0009	0.000	0.000	0.0009	Top of Stone
414.720	0.800	191.87	0.0009	0.000	0.000	0.0009	Pipe Invert
414.820	0.900	191.95	0.0000	0.026	0.000	0.0264	
414.920	1.000	192.02	0.0000	0.037	0.000	0.0373	
415.020	1.100	192.09	0.0000	0.046	0.000	0.0457	
415.120	1.200	192.16	0.0000	0.053	0.000	0.0528	Top of Grate
415.220	1.300	201.45	0.0000	0.059	0.219	0.2777	
415.320	1.400	238.95	0.0000	0.065	0.629	0.6941	
415.420	1.500	304.70	0.0000	0.070	1.175	1.2444	Weir
415.520	1.600	382.20	0.0000	0.075	1.834	1.9086	Overflow

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"          MIDUSS Output ----->"
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"          MIDUSS created                      Sunday, February 07, 2010"
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"          Output filename:                    Post__2yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:17:12 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
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"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                          33.014  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.829  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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"	0.021	0.000	0.000	0.000 c.m/sec"	
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	13.484	1.233	3.680	minutes"
"	Time to Centroid	116.839	87.775	93.582	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	36.32	36.32	72.63	c.m"
"	Rainfall losses	26.179	5.640	15.910	mm"
"	Runoff depth	6.835	27.374	17.104	mm"
"	Runoff volume	7.52	30.11	37.63	c.m"
"	Runoff coefficient	0.207	0.829	0.518	"
"	Maximum flow	0.003	0.021	0.021	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.021	0.021	0.000	0.000"
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" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.207	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.838	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.063	0.021	0.000	0.000 c.m/sec"
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"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	20.437	1.868	5.553	minutes"
"	Time to Centroid	125.085	88.659	95.888	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	115.55	115.55	231.10	c.m"
"	Rainfall losses	26.169	5.363	15.766	mm"
"	Runoff depth	6.845	27.651	17.248	mm"
"	Runoff volume	23.96	96.78	120.74	c.m"

"	Runoff coefficient	0.207	0.838	0.522	"
"	Maximum flow	0.007	0.063	0.063	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.063	0.084	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.836 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.020	0.084	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	17.197	1.572	4.676	minutes"
"	Time to Centroid	121.230	88.299	94.841	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	36.32	36.32	72.63	c.m"
"	Rainfall losses	26.170	5.407	15.789	mm"
"	Runoff depth	6.844	27.606	17.225	mm"
"	Runoff volume	7.53	30.37	37.90	c.m"
"	Runoff coefficient	0.207	0.836	0.522	"
"	Maximum flow	0.002	0.020	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.020	0.105	0.000	0.000"	
" 54	POND DESIGN"				
"	0.105 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	196.3 Hydrograph volume	c.m"			
"	17. Number of stages"				

"	412.000	Minimum water level	metre"		
"	414.490	Maximum water level	metre"		
"	412.000	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	412.000	0.00031	0.000"		
"	412.100	0.00031	34.650"		
"	412.200	0.00031	69.310"		
"	412.300	0.00032	103.960"		
"	412.400	0.00032	138.610"		
"	412.500	0.00032	156.430"		
"	412.600	0.00032	174.250"		
"	412.700	0.00032	192.060"		
"	412.900	0.00032	192.430"		
"	413.230	0.00032	193.040"		
"	413.430	0.04528	193.730"		
"	413.630	0.1319	195.230"		
"	413.830	0.2482	197.730"		
"	414.030	0.3899	201.230"		
"	414.090	0.4369	202.580"		
"	414.290	0.6524	208.080"		
"	414.490	0.9278	214.580"		
"		Peak outflow	0.000	c.m/sec"	
"		Maximum level	412.300	metre"	
"		Maximum storage	104.033	c.m"	
"		Centroidal lag	85.446	hours"	
"		0.020 0.105	0.000 0.000	c.m/sec"	
" 40		HYDROGRAPH Combine	1000"		
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.000	c.m/sec"	
"		Hydrograph volume	190.804	c.m"	
"		0.020 0.105	0.000 0.000	"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.020 0.000	0.000 0.000	"	
" 33		CATCHMENT 1400"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1400	Catchment 1400"			
"	20.000	% Impervious"			
"	0.620	Total Area"			
"	30.000	Flow length"			
"	2.000	Overland Slope"			
"	0.496	Pervious Area"			
"	30.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.124	Impervious Area"			

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.837  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.024      0.000      0.000      0.000 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 26.066      2.383      14.173      minutes"
"      Time to Centroid      131.779      89.515      110.556      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      163.75      40.94      204.69      c.m"
"      Rainfall losses      26.167      5.389      22.011      mm"
"      Runoff depth      6.847      27.625      11.003      mm"
"      Runoff volume      33.96      34.26      68.22      c.m"
"      Runoff coefficient      0.207      0.837      0.333      "
"      Maximum flow      0.008      0.023      0.024      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.024      0.024      0.000      0.000"
" 54      POND DESIGN"
"      0.024      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      68.2      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.001	c.m/sec"	
"		Maximum level	414.120	metre"	
"		Maximum storage	55.240	c.m"	
"		Centroidal lag	13.537	hours"	
"		0.024	0.024	0.001	0.000 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.001	c.m/sec"	
"		Hydrograph volume	259.018	c.m"	
"		0.024	0.024	0.001	0.001"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.024	0.000	0.001	0.001"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.207	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.834	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.105	0.000	0.001	0.001 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	30.977	2.832	8.436 minutes"
"		Time to Centroid	137.612	90.217	99.655 minutes"
"		Rainfall depth	33.014	33.014	33.014 mm"

"	Rainfall volume	183.23	183.23	366.46	c.m"
"	Rainfall losses	26.164	5.467	15.815	mm"
"	Runoff depth	6.850	27.547	17.198	mm"
"	Runoff volume	38.02	152.89	190.90	c.m"
"	Runoff coefficient	0.207	0.834	0.521	"
"	Maximum flow	0.008	0.104	0.105	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.105 0.105 0.001 0.001"				
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000		c.m/sec"	
"	Volume of diverted flow	0.000		c.m"	
"	DIV01500.002hyd"				
"	Major flow at 1500"				
"	0.105 0.105 0.105 0.001 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.105 0.105 0.105 0.001"				
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.208 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.846 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.662 0.105 0.105 0.001 c.m/sec"				
"	Catchment 1000 Pervious Impervious Total Area "				
"	Surface Area 3.490 3.490 6.980 hectare"				

"	Time of concentration	53.679	4.907	14.516	minutes"
"	Time to Centroid	164.608	93.181	107.253	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	1152.19	1152.19	2304.37	c.m"
"	Rainfall losses	26.162	5.089	15.626	mm"
"	Runoff depth	6.851	27.925	17.388	mm"
"	Runoff volume	239.12	974.59	1213.71	c.m"
"	Runoff coefficient	0.208	0.846	0.527	"
"	Maximum flow	0.036	0.659	0.662	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.662	0.768	0.105	0.001"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.009	0.768	0.105	0.001 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	20.437	1.868	20.437	minutes"
"	Time to Centroid	125.085	88.659	125.085	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	158.47	0.00	158.47	c.m"
"	Rainfall losses	26.169	5.363	26.169	mm"
"	Runoff depth	6.845	27.651	6.845	mm"
"	Runoff volume	32.86	0.00	32.86	c.m"
"	Runoff coefficient	0.207	0.000	0.207	"
"	Maximum flow	0.009	0.000	0.009	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

```

"          4  Add Runoff "
"              0.009      0.770      0.105      0.001"
" 54      POND DESIGN"
"          0.770  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          1437.5  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow              0.097      c.m/sec"
"          Maximum level              411.511      metre"
"          Maximum storage              726.712      c.m"
"          Centroidal lag              3.225      hours"
"              0.009      0.770      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.009      0.097      0.097      0.001"
" 47      FILEI_O Read/Open DIV01500.002hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.002hyd"
"          Major flow at 1500"
"          Total volume              0.000      c.m"
"          Maximum flow              0.000      c.m/sec"
"              0.000      0.097      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.000      0.097      0.097      0.001"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

```

"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	50.000	Pervious SCS Curve No. "			
"	0.007	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No. "			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.001 0.097 0.097 0.001 c.m/sec"			
"		Catchment 4000 Pervious Impervious Total Area "			
"		Surface Area 7.330 0.000 7.330 hectare"			
"		Time of concentration 236.855 3.611 236.826 minutes"			
"		Time to Centroid 321.151 91.497 321.122 minutes"			
"		Rainfall depth 33.014 33.014 33.014 mm"			
"		Rainfall volume 2419.92 0.00 2419.92 c.m"			
"		Rainfall losses 32.792 5.642 32.792 mm"			
"		Runoff depth 0.222 27.372 0.222 mm"			
"		Runoff volume 16.24 0.00 16.24 c.m"			
"		Runoff coefficient 0.007 0.000 0.007 "			
"		Maximum flow 0.001 0.000 0.001 c.m/sec"			
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.001 0.097 0.097 0.001"			
" 54		POND DESIGN"			
"	0.097	Current peak flow c.m/sec"			
"	0.250	Target outflow c.m/sec"			
"	1454.3	Hydrograph volume c.m"			
"	6.	Number of stages"			
"	409.630	Minimum water level metre"			
"	410.750	Maximum water level metre"			
"	409.630	Starting water level metre"			
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge Volume"			
"	409.630	0.000 0.000"			
"	409.750	0.6650 402.200"			
"	410.000	3.601 2187.900"			
"	410.250	7.811 5318.900"			
"	410.500	12.984 9642.300"			
"	410.750	18.965 15227.70"			

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"          Peak outflow          0.097    c.m/sec"
"          Maximum level        409.647    metre"
"          Maximum storage      58.572    c.m"
"          Centroidal lag       3.417    hours"
"          0.001    0.097    0.097    0.001 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.001    0.097    0.097    0.001"
" 52    CHANNEL DESIGN"
"          0.097    Current peak flow    c.m/sec"
"          0.035    Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          0.000    Basewidth    metre"
"          7.410    Left bank slope"
"          6.000    Right bank slope"
"          0.950    Channel depth    metre"
"          1.040    Gradient    %"
"          Depth of flow          0.163    metre"
"          Velocity                0.544    m/sec"
"          Channel capacity        10.655    c.m/sec"
"          Critical depth          0.134    metre"
" 53    ROUTE    Channel Route 72"
"          72.40    Channel Route 72 Reach length    ( metre)"
"          0.459    X-factor <= 0.5"
"          99.839    K-lag    ( seconds)"
"          0.000    Default(0) or user spec.(1) values used"
"          0.500    X-factor <= 0.5"
"          30.000    K-lag    ( seconds)"
"          0.500    Beta weighting factor"
"          100.000    Routing time step    ( seconds)"
"          1    No. of sub-reaches"
"          Peak outflow          0.097    c.m/sec"
"          0.001    0.097    0.097    0.001 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.001    0.097    0.097    0.001"
" 52    CHANNEL DESIGN"
"          0.097    Current peak flow    c.m/sec"
"          0.035    Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          2.000    Basewidth    metre"
"          2.950    Left bank slope"
"          3.000    Right bank slope"
"          0.950    Channel depth    metre"
"          1.040    Gradient    %"
"          Depth of flow          0.084    metre"
"          Velocity                0.516    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.060    metre"
" 53    ROUTE    Channel Route 40"

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.441      X-factor <= 0.5"
"      57.872      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"      60.000      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                      0.097      c.m/sec"
"          0.001      0.097      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine "
"          100      Node #"
"          Existing Wetland"
"          Maximum flow                      0.097      c.m/sec"
"          Hydrograph volume                  1454.315      c.m"
"          0.001      0.097      0.097      0.097"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.001      0.000      0.097      0.097"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100      Catchment 2100"
"          60.000      % Impervious"
"          1.960      Total Area"
"          40.000      Flow length"
"          2.000      Overland Slope"
"          0.784      Pervious Area"
"          40.000      Pervious length"
"          2.000      Pervious slope"
"          1.176      Impervious Area"
"          40.000      Impervious length"
"          2.000      Impervious slope"
"          0.250      Pervious Manning 'n'"
"          78.000      Pervious SCS Curve No."
"          0.207      Pervious Runoff coefficient"
"          0.100      Pervious Ia/S coefficient"
"          7.164      Pervious Initial abstraction"
"          0.015      Impervious Manning 'n'"
"          98.000      Impervious SCS Curve No."
"          0.834      Impervious Runoff coefficient"
"          0.100      Impervious Ia/S coefficient"
"          0.518      Impervious Initial abstraction"
"          0.223      0.000      0.097      0.097 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area      0.784      1.176      1.960      hectare"
"          Time of concentration      30.977      2.832      6.834      minutes"

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"	Time to Centroid	137.612	90.217	96.956	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	258.83	388.24	647.07	c.m"
"	Rainfall losses	26.164	5.467	13.746	mm"
"	Runoff depth	6.850	27.547	19.268	mm"
"	Runoff volume	53.70	323.96	377.66	c.m"
"	Runoff coefficient	0.207	0.834	0.584	"
"	Maximum flow	0.012	0.221	0.223	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.223 0.223 0.097 0.097"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.838 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.129 0.223 0.097 0.097 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 20.437 1.868 2.365 minutes"				
"	Time to Centroid 125.085 88.659 89.634 minutes"				
"	Rainfall depth 33.014 33.014 33.014 mm"				
"	Rainfall volume 26.41 237.70 264.11 c.m"				
"	Rainfall losses 26.169 5.363 7.444 mm"				
"	Runoff depth 6.845 27.651 25.570 mm"				
"	Runoff volume 5.48 199.09 204.56 c.m"				
"	Runoff coefficient 0.207 0.838 0.775 "				
"	Maximum flow 0.002 0.129 0.129 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.129	0.349	0.097	0.097"
" 54	POND DESIGN"				
"	0.349	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	582.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.027	c.m/sec"	
"		Maximum level	411.061	metre"	
"		Maximum storage	400.164	c.m"	
"		Centroidal lag	4.889	hours"	
"		0.129	0.349	0.027	0.097 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.129	0.027	0.027	0.097"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.207	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.838	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.011	0.027	0.027	0.097 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	20.437	1.868	14.685 minutes"
"		Time to Centroid	125.085	88.659	113.801 minutes"
"		Rainfall depth	33.014	33.014	33.014 mm"
"		Rainfall volume	142.62	15.85	158.47 c.m"
"		Rainfall losses	26.169	5.363	24.088 mm"
"		Runoff depth	6.845	27.651	8.926 mm"
"		Runoff volume	29.57	13.27	42.84 c.m"
"		Runoff coefficient	0.207	0.838	0.270 "
"		Maximum flow	0.008	0.009	0.011 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"		4 Add Runoff "			
"		0.011	0.036	0.027	0.097"
" 40		HYDROGRAPH Copy to Outflow"			
"		8 Copy to Outflow"			
"		0.011	0.036	0.036	0.097"
" 40		HYDROGRAPH Combine 200"			
"		6 Combine "			
"		200 Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.036		c.m/sec"
"		Hydrograph volume	625.068		c.m"
"		0.011	0.036	0.036	0.036"
" 40		HYDROGRAPH Start - New Tributary"			
"		2 Start - New Tributary"			
"		0.011	0.000	0.036	0.036"
" 33		CATCHMENT 2200"			
"		1 Triangular SCS"			
"		1 Equal length"			
"		1 SCS method"			
"		2200 Catchment 2200"			
"		75.000 % Impervious"			
"		0.920 Total Area"			
"		40.000 Flow length"			
"		2.000 Overland Slope"			
"		0.230 Pervious Area"			
"		40.000 Pervious length"			
"		2.000 Pervious slope"			
"		0.690 Impervious Area"			
"		40.000 Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.207	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.834	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.130	0.000	0.036	0.036 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	30.977	2.832	4.986	minutes"
"		Time to Centroid	137.612	90.217	93.844	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	75.93	227.80	303.73	c.m"
"		Rainfall losses	26.164	5.467	10.641	mm"
"		Runoff depth	6.850	27.547	22.373	mm"
"		Runoff volume	15.75	190.08	205.83	c.m"
"		Runoff coefficient	0.207	0.834	0.678	"
"		Maximum flow	0.004	0.129	0.130	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.130	0.130	0.036	0.036"	
" 54		POND DESIGN"				
"	0.130	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	205.8	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.008	c.m/sec"		
"		Maximum level	413.864	metre"		

"	Maximum storage	151.520	c.m"
"	Centroidal lag	6.574	hours"
"	0.130 0.130 0.008 0.036		c.m/sec"
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.043	c.m/sec"
"	Hydrograph volume	830.894	c.m"
"	0.130 0.130 0.008 0.043"		
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.130 0.000 0.008 0.043"		
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.208 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.830 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.049 0.000 0.008 0.043		c.m/sec"
"	Catchment 3100 Pervious Impervious Total Area "		
"	Surface Area 0.168 0.252 0.420		hectare"
"	Time of concentration 38.137 3.486 8.435		minutes"
"	Time to Centroid 146.130 91.278 99.112		minutes"
"	Rainfall depth 33.014 33.014 33.014		mm"
"	Rainfall volume 55.46 83.20 138.66		c.m"
"	Rainfall losses 26.163 5.605 13.828		mm"
"	Runoff depth 6.851 27.409 19.186		mm"
"	Runoff volume 11.51 69.07 80.58		c.m"
"	Runoff coefficient 0.208 0.830 0.581		"
"	Maximum flow 0.002 0.048 0.049		c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.049      0.049      0.008      0.043"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"              0  Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.000      c.m/sec"
"              Volume of diverted flow      0.000      c.m"
"              DIV32001.002hyd"
"              Major flow at 32001"
"                  0.049      0.049      0.049      0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.049      0.049      0.049      0.043"
" 54      POND DESIGN"
"          0.049  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          80.6  Hydrograph volume      c.m"
"          15.  Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"              410.620      0.000      0.000"
"              410.870      0.01300      4.855"
"              411.120      0.02000      14.351"
"              411.370      0.02500      24.074"
"              411.620      0.02900      33.921"
"              411.870      0.03300      43.768"
"              412.120      0.03600      53.614"
"              412.370      0.03900      63.461"
"              412.620      0.04200      73.308"
"              412.870      0.04400      74.155"
"              413.120      0.04700      75.003"
"              413.370      0.04900      75.850"
"              413.620      0.05200      76.698"
"              413.980      0.05500      77.918"
"              414.230      0.1600      78.483"
"              Peak outflow      0.022      c.m/sec"
"              Maximum level      411.239      metre"
"              Maximum storage      18.968      c.m"
"              Centroidal lag      1.810      hours"
"                  0.049      0.049      0.022      0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.049      0.022      0.022      0.043"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3200	Catchment 3200"				
"	60.000	% Impervious"				
"	0.130	Total Area"				
"	20.000	Flow length"				
"	1.000	Overland Slope"				
"	0.052	Pervious Area"				
"	20.000	Pervious length"				
"	1.000	Pervious slope"				
"	0.078	Impervious Area"				
"	20.000	Impervious length"				
"	1.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	78.000	Pervious SCS Curve No. "				
"	0.207	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n' "				
"	98.000	Impervious SCS Curve No. "				
"	0.837	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.014	0.022	0.022	0.043 c.m/sec"	
"		Catchment 3200	Pervious	Impervious	Total Area	"
"		Surface Area	0.052	0.078	0.130	hectare"
"		Time of concentration	25.161	2.300	5.542	minutes"
"		Time to Centroid	130.696	89.401	95.257	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	17.17	25.75	42.92	c.m"
"		Rainfall losses	26.167	5.393	13.702	mm"
"		Runoff depth	6.847	27.621	19.312	mm"
"		Runoff volume	3.56	21.54	25.11	c.m"
"		Runoff coefficient	0.207	0.837	0.585	"
"		Maximum flow	0.001	0.014	0.014	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.014	0.034	0.022	0.043"	
" 33		CATCHMENT 3300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3300	Catchment 3300"				
"	60.000	% Impervious"				
"	0.240	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.096	Pervious Area"				
"	20.000	Pervious length"				

```

"      2.000  Pervious slope"
"      0.144  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.838  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.026      0.034      0.022      0.043 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 20.437      1.868      4.499      minutes"
"      Time to Centroid 125.085      88.659      93.819      minutes"
"      Rainfall depth 33.014      33.014      33.014      mm"
"      Rainfall volume 31.69      47.54      79.23      c.m"
"      Rainfall losses 26.169      5.363      13.685      mm"
"      Runoff depth 6.845      27.651      19.329      mm"
"      Runoff volume 6.57      39.82      46.39      c.m"
"      Runoff coefficient 0.207      0.838      0.585      "
"      Maximum flow 0.002      0.026      0.026      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.026      0.060      0.022      0.043"
" 47      FILEI_0 Read/Open DIV32001.002hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.002hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.060      0.022      0.043 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.060      0.022      0.043"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.060      0.060      0.043"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.060      c.m/sec"
"      Hydrograph volume 152.160      c.m"

```

"		0.000	0.060	0.060	0.060"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.060		c.m/sec"	
"		Hydrograph volume	152.160		c.m"	
"		0.000	0.060	0.060	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.060	0.060	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.114		c.m/sec"	
"		Hydrograph volume	1606.475		c.m"	
"		0.000	0.060	0.060	0.114"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.114		c.m/sec"	
"		Hydrograph volume	1606.475		c.m"	
"		0.000	0.114	0.060	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.114	0.114	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.156		c.m/sec"	
"		Hydrograph volume	2437.367		c.m"	
"		0.000	0.114	0.114	0.156"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.156		c.m/sec"	
"		Hydrograph volume	2437.366		c.m"	
"		0.000	0.156	0.114	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__5yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:20:45 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          113.586  mm/hr"
"          Total depth                49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000  % Impervious"
"          0.220  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.317  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.871  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"	0.029	0.000	0.000	0.000 c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area "
"	Surface Area	0.110	0.110	0.220 hectare"
"	Time of concentration	9.868	1.116	3.453 minutes"
"	Time to Centroid	109.069	86.405	92.457 minutes"
"	Rainfall depth	49.792	49.792	49.792 mm"
"	Rainfall volume	54.77	54.77	109.54 c.m"
"	Rainfall losses	33.989	6.414	20.202 mm"
"	Runoff depth	15.803	43.377	29.590 mm"
"	Runoff volume	17.38	47.72	65.10 c.m"
"	Runoff coefficient	0.317	0.871	"
"	Maximum flow	0.007	0.028	0.029 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.029	0.029	0.000	0.000"
---	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.319	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.883	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.093	0.029	0.000	0.000 c.m/sec"
---	-------	-------	-------	----------------

"	Catchment 1300	Pervious	Impervious	Total Area "
"	Surface Area	0.350	0.350	0.700 hectare"
"	Time of concentration	14.957	1.691	5.209 minutes"
"	Time to Centroid	115.000	87.210	94.579 minutes"
"	Rainfall depth	49.792	49.792	49.792 mm"
"	Rainfall volume	174.27	174.27	348.54 c.m"
"	Rainfall losses	33.921	5.811	19.866 mm"
"	Runoff depth	15.871	43.981	29.926 mm"
"	Runoff volume	55.55	153.93	209.48 c.m"

"	Runoff coefficient	0.319	0.883	0.601	"
"	Maximum flow	0.020	0.086	0.093	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.093	0.122	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.318 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.881 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.029	0.122	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	12.585	1.423	4.386	minutes"
"	Time to Centroid	112.243	86.878	93.610	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	54.77	54.77	109.54	c.m"
"	Rainfall losses	33.944	5.926	19.935	mm"
"	Runoff depth	15.848	43.866	29.857	mm"
"	Runoff volume	17.43	48.25	65.69	c.m"
"	Runoff coefficient	0.318	0.881	0.600	"
"	Maximum flow	0.007	0.028	0.029	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.029	0.151	0.000	0.000"	
" 54	POND DESIGN"				
"	0.151 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	340.3 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

"      412.000 Minimum water level      metre"
"      414.490 Maximum water level      metre"
"      412.000 Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      412.000      0.00031      0.000"
"      412.100      0.00031      34.650"
"      412.200      0.00031      69.310"
"      412.300      0.00032      103.960"
"      412.400      0.00032      138.610"
"      412.500      0.00032      156.430"
"      412.600      0.00032      174.250"
"      412.700      0.00032      192.060"
"      412.900      0.00032      192.430"
"      413.230      0.00032      193.040"
"      413.430      0.04528      193.730"
"      413.630      0.1319      195.230"
"      413.830      0.2482      197.730"
"      414.030      0.3899      201.230"
"      414.090      0.4369      202.580"
"      414.290      0.6524      208.080"
"      414.490      0.9278      214.580"
"          Peak outflow              0.074      c.m/sec"
"          Maximum level              413.507      metre"
"          Maximum storage            194.308      c.m"
"          Centroidal lag             49.492      hours"
"          0.029      0.151      0.074      0.000 c.m/sec"
" 40      HYDROGRAPH      Combine      1000"
"          6      Combine "
"      1000      Node #"
"          Infiltrated On-Site"
"          Maximum flow              0.074      c.m/sec"
"          Hydrograph volume          335.882      c.m"
"          0.029      0.151      0.074      0.074"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.029      0.000      0.074      0.074"
" 33      CATCHMENT 1400"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          1400      Catchment 1400"
"      20.000      % Impervious"
"          0.620      Total Area"
"      30.000      Flow length"
"          2.000      Overland Slope"
"          0.496      Pervious Area"
"      30.000      Pervious length"
"          2.000      Pervious slope"
"          0.124      Impervious Area"

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.882  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.038      0.000      0.074      0.074 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 19.076      2.157      12.156      minutes"
"      Time to Centroid      119.796      87.903      106.752      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      246.97      61.74      308.71      c.m"
"      Rainfall losses      33.923      5.866      28.312      mm"
"      Runoff depth      15.868      43.926      21.480      mm"
"      Runoff volume      78.71      54.47      133.17      c.m"
"      Runoff coefficient      0.319      0.882      0.431      "
"      Maximum flow      0.025      0.031      0.038      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.038      0.038      0.074      0.074"
" 54      POND DESIGN"
"      0.038      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"     133.2      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.001	c.m/sec"	
"		Maximum level	414.361	metre"	
"		Maximum storage	123.613	c.m"	
"		Centroidal lag	22.741	hours"	
"		0.038	0.038	0.001	0.074 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.075	c.m/sec"	
"		Hydrograph volume	469.052	c.m"	
"		0.038	0.038	0.001	0.075"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.038	0.000	0.001	0.075"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.319	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.878	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.146	0.000	0.001	0.075 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	22.670	2.563	7.922 minutes"
"		Time to Centroid	124.006	88.517	97.975 minutes"
"		Rainfall depth	49.792	49.792	49.792 mm"

"	Rainfall volume	276.34	276.34	552.69	c.m"
"	Rainfall losses	33.904	6.066	19.985	mm"
"	Runoff depth	15.888	43.726	29.807	mm"
"	Runoff volume	88.18	242.68	330.86	c.m"
"	Runoff coefficient	0.319	0.878	0.599	"
"	Maximum flow	0.026	0.139	0.146	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.146	0.146	0.001	0.075"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000		c.m/sec"	
"	Volume of diverted flow	0.000		c.m"	
"	DIV01500.005hyd"				
"	Major flow at 1500"				
"		0.146	0.146	0.146	0.075 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.146	0.146	0.146	0.075"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.888 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.935	0.146	0.146	0.075 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	39.284	4.442	13.658	minutes"
"	Time to Centroid	143.413	91.124	104.955	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	1737.73	1737.73	3475.46	c.m"
"	Rainfall losses	33.893	5.585	19.739	mm"
"	Runoff depth	15.898	44.206	30.052	mm"
"	Runoff volume	554.85	1542.80	2097.65	c.m"
"	Runoff coefficient	0.319	0.888	0.604	"
"	Maximum flow	0.113	0.920	0.935	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.935	1.081	0.146	0.075"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.028	1.081	0.146	0.075 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	14.957	1.691	14.957	minutes"
"	Time to Centroid	114.999	87.210	114.999	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	239.00	0.00	239.00	c.m"
"	Rainfall losses	33.921	5.811	33.921	mm"
"	Runoff depth	15.871	43.981	15.871	mm"
"	Runoff volume	76.18	0.00	76.18	c.m"
"	Runoff coefficient	0.319	0.000	0.319	"
"	Maximum flow	0.028	0.000	0.028	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"          0.028      1.091      0.146      0.075"
" 54      POND DESIGN"
"          1.091  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          2504.7  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow      0.234      c.m/sec"
"          Maximum level      411.661      metre"
"          Maximum storage      1286.482      c.m"
"          Centroidal lag      3.422      hours"
"          0.028      1.091      0.234      0.075 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.028      0.234      0.234      0.075"
" 47      FILEI_0 Read/Open DIV01500.005hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.005hyd"
"          Major flow at 1500"
"          Total volume      0.000      c.m"
"          Maximum flow      0.000      c.m/sec"
"          0.000      0.234      0.234      0.075 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.000      0.234      0.234      0.075"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	50.000	Pervious SCS Curve No. "			
"	0.043	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No. "			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.019 0.234 0.234 0.075 c.m/sec"			
"		Catchment 4000 Pervious Impervious Total Area "			
"		Surface Area 7.330 0.000 7.330 hectare"			
"		Time of concentration 82.074 3.269 82.072 minutes"			
"		Time to Centroid 193.297 89.581 193.294 minutes"			
"		Rainfall depth 49.792 49.792 49.792 mm"			
"		Rainfall volume 3649.73 0.00 3649.73 c.m"			
"		Rainfall losses 47.655 6.236 47.655 mm"			
"		Runoff depth 2.137 43.556 2.137 mm"			
"		Runoff volume 156.63 0.00 156.64 c.m"			
"		Runoff coefficient 0.043 0.000 0.043 "			
"		Maximum flow 0.019 0.000 0.019 c.m/sec"			
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.019 0.244 0.234 0.075"			
" 54		POND DESIGN"			
"	0.244	Current peak flow c.m/sec"			
"	0.250	Target outflow c.m/sec"			
"	2662.1	Hydrograph volume c.m"			
"	6.	Number of stages"			
"	409.630	Minimum water level metre"			
"	410.750	Maximum water level metre"			
"	409.630	Starting water level metre"			
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge Volume"			
"	409.630	0.000 0.000"			
"	409.750	0.6650 402.200"			
"	410.000	3.601 2187.900"			
"	410.250	7.811 5318.900"			
"	410.500	12.984 9642.300"			
"	410.750	18.965 15227.70"			

"	Peak outflow	0.237	c.m/sec"
"	Maximum level	409.673	metre"
"	Maximum storage	143.048	c.m"
"	Centroidal lag	3.578	hours"
"	0.019 0.244 0.237 0.075		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.019 0.237 0.237 0.075		
" 52	CHANNEL DESIGN"		
"	0.237 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	0.000 Basewidth	metre"	
"	7.410 Left bank slope"		
"	6.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.228	metre"
"	Velocity	0.680	m/sec"
"	Channel capacity	10.655	c.m/sec"
"	Critical depth	0.191	metre"
" 53	ROUTE Channel Route 72"		
"	72.40 Channel Route 72 Reach length	(metre)"	
"	0.443 X-factor <= 0.5"		
"	79.856 K-lag (seconds)"		
"	0.000 Default(0) or user spec.(1) values used"		
"	0.500 X-factor <= 0.5"		
"	30.000 K-lag (seconds)"		
"	0.500 Beta weighting factor"		
"	75.000 Routing time step (seconds)"		
"	1 No. of sub-reaches"		
"	Peak outflow	0.236	c.m/sec"
"	0.019 0.237 0.236 0.075		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.019 0.236 0.236 0.075		
" 52	CHANNEL DESIGN"		
"	0.236 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	2.000 Basewidth	metre"	
"	2.950 Left bank slope"		
"	3.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.140	metre"
"	Velocity	0.699	m/sec"
"	Channel capacity	9.246	c.m/sec"
"	Critical depth	0.106	metre"
" 53	ROUTE Channel Route 40"		

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.403      X-factor <= 0.5"
"      42.730      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"      50.000      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                      0.236      c.m/sec"
"          0.019      0.236      0.236      0.075 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine "
"          100      Node #"
"          Existing Wetland"
"          Maximum flow                      0.236      c.m/sec"
"          Hydrograph volume                  2662.062      c.m"
"          0.019      0.236      0.236      0.236"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.019      0.000      0.236      0.236"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100      Catchment 2100"
"          60.000      % Impervious"
"          1.960      Total Area"
"          40.000      Flow length"
"          2.000      Overland Slope"
"          0.784      Pervious Area"
"          40.000      Pervious length"
"          2.000      Pervious slope"
"          1.176      Impervious Area"
"          40.000      Impervious length"
"          2.000      Impervious slope"
"          0.250      Pervious Manning 'n'"
"          78.000      Pervious SCS Curve No."
"          0.319      Pervious Runoff coefficient"
"          0.100      Pervious Ia/S coefficient"
"          7.164      Pervious Initial abstraction"
"          0.015      Impervious Manning 'n'"
"          98.000      Impervious SCS Curve No."
"          0.878      Impervious Runoff coefficient"
"          0.100      Impervious Ia/S coefficient"
"          0.518      Impervious Initial abstraction"
"          0.304      0.000      0.236      0.236 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area      0.784      1.176      1.960      hectare"
"          Time of concentration      22.670      2.563      6.484      minutes"

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"	Time to Centroid	124.006	88.517	95.437	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	390.37	585.55	975.92	c.m"
"	Rainfall losses	33.904	6.066	17.201	mm"
"	Runoff depth	15.888	43.726	32.591	mm"
"	Runoff volume	124.56	514.22	638.78	c.m"
"	Runoff coefficient	0.319	0.878	0.655	"
"	Maximum flow	0.036	0.295	0.304	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.304 0.304 0.236 0.236"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.883 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.177 0.304 0.236 0.236 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 14.957 1.691 2.203 minutes"				
"	Time to Centroid 114.999 87.210 88.281 minutes"				
"	Rainfall depth 49.792 49.792 49.792 mm"				
"	Rainfall volume 39.83 358.50 398.33 c.m"				
"	Rainfall losses 33.921 5.811 8.622 mm"				
"	Runoff depth 15.871 43.981 41.170 mm"				
"	Runoff volume 12.70 316.66 329.36 c.m"				
"	Runoff coefficient 0.319 0.883 0.827 "				
"	Maximum flow 0.005 0.177 0.177 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.177	0.481	0.236	0.236"
" 54	POND DESIGN"				
"	0.481	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	968.1	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.121	c.m/sec"	
"		Maximum level	411.195	metre"	
"		Maximum storage	550.354	c.m"	
"		Centroidal lag	4.085	hours"	
"		0.177	0.481	0.121	0.236 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.177	0.121	0.121	0.236"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.319	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.883	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.030	0.121	0.121	0.236 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	14.957	1.691	11.834 minutes"
"		Time to Centroid	114.999	87.210	108.457 minutes"
"		Rainfall depth	49.792	49.792	49.792 mm"
"		Rainfall volume	215.10	23.90	239.00 c.m"
"		Rainfall losses	33.921	5.811	31.110 mm"
"		Runoff depth	15.871	43.981	18.682 mm"
"		Runoff volume	68.56	21.11	89.67 c.m"
"		Runoff coefficient	0.319	0.883	0.375 "
"		Maximum flow	0.025	0.012	0.030 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.030	0.141	0.121	0.236"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.030	0.141	0.141	0.236"
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.141		c.m/sec"
"		Hydrograph volume	1056.969		c.m"
"		0.030	0.141	0.141	0.141"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.030	0.000	0.141	0.141"
" 33		CATCHMENT 2200"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2200	Catchment 2200"			
"	75.000	% Impervious"			
"	0.920	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.230	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.690	Impervious Area"			
"	40.000	Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.319	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.878	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.176	0.000	0.141	0.141 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	22.670	2.563	4.735	minutes"
"		Time to Centroid	124.006	88.517	92.351	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	114.52	343.56	458.08	c.m"
"		Rainfall losses	33.904	6.066	13.025	mm"
"		Runoff depth	15.888	43.726	36.767	mm"
"		Runoff volume	36.54	301.71	338.25	c.m"
"		Runoff coefficient	0.319	0.878	0.738	"
"		Maximum flow	0.011	0.173	0.176	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.176	0.176	0.141	0.141"	
" 54		POND DESIGN"				
"	0.176	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	338.3	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.012	c.m/sec"		
"		Maximum level	413.963	metre"		

"	Maximum storage	257.625	c.m"
"	Centroidal lag	6.867	hours"
"	0.176 0.176 0.012 0.141	c.m/sec"	
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.151	c.m/sec"
"	Hydrograph volume	1395.219	c.m"
"	0.176 0.176 0.012	0.151"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.176 0.000 0.012	0.151"	
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.319 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.877 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.067 0.000 0.012	0.151 c.m/sec"	
"	Catchment 3100 Pervious Impervious Total Area "		
"	Surface Area 0.168 0.252 0.420	hectare"	
"	Time of concentration 27.910 3.156 7.991	minutes"	
"	Time to Centroid 130.112 89.411 97.361	minutes"	
"	Rainfall depth 49.792 49.792 49.792	mm"	
"	Rainfall volume 83.65 125.48 209.13	c.m"	
"	Rainfall losses 33.900 6.148 17.249	mm"	
"	Runoff depth 15.891 43.643 32.543	mm"	
"	Runoff volume 26.70 109.98 136.68	c.m"	
"	Runoff coefficient 0.319 0.877 0.654	"	
"	Maximum flow 0.007 0.066 0.067	c.m/sec"	

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.067      0.067      0.012      0.151"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"              0  Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.000      c.m/sec"
"              Volume of diverted flow      0.000      c.m"
"              DIV32001.005hyd"
"              Major flow at 32001"
"                  0.067      0.067      0.067      0.151 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.067      0.067      0.067      0.151"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          136.7  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"              0  Keep Design Data: 1 = True; 0 = False"
"                  Level Discharge      Volume"
"                  410.620      0.000      0.000"
"                  410.870      0.01300      4.855"
"                  411.120      0.02000      14.351"
"                  411.370      0.02500      24.074"
"                  411.620      0.02900      33.921"
"                  411.870      0.03300      43.768"
"                  412.120      0.03600      53.614"
"                  412.370      0.03900      63.461"
"                  412.620      0.04200      73.308"
"                  412.870      0.04400      74.155"
"                  413.120      0.04700      75.003"
"                  413.370      0.04900      75.850"
"                  413.620      0.05200      76.698"
"                  413.980      0.05500      77.918"
"                  414.230      0.1600      78.483"
"              Peak outflow      0.029      c.m/sec"
"              Maximum level      411.630      metre"
"              Maximum storage      34.317      c.m"
"              Centroidal lag      1.847      hours"
"                  0.067      0.067      0.029      0.151 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.067      0.029      0.029      0.151"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3200	Catchment 3200"				
"	60.000	% Impervious"				
"	0.130	Total Area"				
"	20.000	Flow length"				
"	1.000	Overland Slope"				
"	0.052	Pervious Area"				
"	20.000	Pervious length"				
"	1.000	Pervious slope"				
"	0.078	Impervious Area"				
"	20.000	Impervious length"				
"	1.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	78.000	Pervious SCS Curve No."				
"	0.319	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n' "				
"	98.000	Impervious SCS Curve No."				
"	0.883	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.020	0.029	0.029	0.151 c.m/sec"
"		Catchment 3200	Pervious	Impervious	Total Area	"
"		Surface Area	0.052	0.078	0.130	hectare"
"		Time of concentration	18.414	2.082	5.253	minutes"
"		Time to Centroid	119.025	87.780	93.846	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	25.89	38.84	64.73	c.m"
"		Rainfall losses	33.912	5.846	17.073	mm"
"		Runoff depth	15.879	43.946	32.719	mm"
"		Runoff volume	8.26	34.28	42.53	c.m"
"		Runoff coefficient	0.319	0.883	0.657	"
"		Maximum flow	0.003	0.019	0.020	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.020	0.045	0.029	0.151"
" 33		CATCHMENT 3300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3300	Catchment 3300"				
"	60.000	% Impervious"				
"	0.240	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.096	Pervious Area"				
"	20.000	Pervious length"				

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.319 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.883 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.037      0.045      0.029      0.151 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 14.957      1.691      4.264      minutes"
"      Time to Centroid 115.000      87.210      92.599      minutes"
"      Rainfall depth 49.792      49.792      49.792      mm"
"      Rainfall volume 47.80      71.70      119.50      c.m"
"      Rainfall losses 33.921      5.811      17.055      mm"
"      Runoff depth 15.871      43.981      32.737      mm"
"      Runoff volume 15.24      63.33      78.57      c.m"
"      Runoff coefficient 0.319      0.883      0.657      "
"      Maximum flow 0.006      0.035      0.037      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.037      0.082      0.029      0.151"
" 47      FILEI_0 Read/Open DIV32001.005hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.005hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.082      0.029      0.151 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.082      0.029      0.151"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.082      0.082      0.151"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.082      c.m/sec"
"      Hydrograph volume 257.887      c.m"

```

"		0.000	0.082	0.082	0.082"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.082		c.m/sec"	
"		Hydrograph volume	257.887		c.m"	
"		0.000	0.082	0.082	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.082	0.082	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.258		c.m/sec"	
"		Hydrograph volume	2919.949		c.m"	
"		0.000	0.082	0.082	0.258"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.258		c.m/sec"	
"		Hydrograph volume	2919.948		c.m"	
"		0.000	0.258	0.082	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.258	0.258	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.366		c.m/sec"	
"		Hydrograph volume	4315.163		c.m"	
"		0.000	0.258	0.258	0.366"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.366		c.m/sec"	
"		Hydrograph volume	4315.163		c.m"	
"		0.000	0.366	0.258	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__10yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:41:13 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          2327.596 Coefficient A"
"          19.500  Constant B"
"          0.894  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          126.171  mm/hr"
"          Total depth                61.359  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.379  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.887  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"	0.037	0.000	0.000	0.000 c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area "
"	Surface Area	0.110	0.110	0.220 hectare"
"	Time of concentration	8.639	1.063	3.329 minutes"
"	Time to Centroid	106.058	85.846	91.893 minutes"
"	Rainfall depth	61.359	61.359	61.359 mm"
"	Rainfall volume	67.50	67.50	134.99 c.m"
"	Rainfall losses	38.124	6.938	22.531 mm"
"	Runoff depth	23.235	54.421	38.828 mm"
"	Runoff volume	25.56	59.86	85.42 c.m"
"	Runoff coefficient	0.379	0.887	0.633 "
"	Maximum flow	0.011	0.032	0.037 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.037	0.037	0.000	0.000"
---	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.379	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.901	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.114	0.037	0.000	0.000 c.m/sec"
---	-------	-------	-------	----------------

"	Catchment 1300	Pervious	Impervious	Total Area "
"	Surface Area	0.350	0.350	0.700 hectare"
"	Time of concentration	13.094	1.611	5.010 minutes"
"	Time to Centroid	111.234	86.563	93.866 minutes"
"	Rainfall depth	61.359	61.359	61.359 mm"
"	Rainfall volume	214.76	214.76	429.51 c.m"
"	Rainfall losses	38.098	6.044	22.071 mm"
"	Runoff depth	23.262	55.315	39.288 mm"
"	Runoff volume	81.42	193.60	275.02 c.m"

"	Runoff coefficient	0.379	0.901	0.640	"
"	Maximum flow	0.032	0.100	0.114	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.114	0.151	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.380 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.898 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.036	0.151	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	11.018	1.355	4.226	minutes"
"	Time to Centroid	108.801	86.227	92.933	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	67.50	67.50	134.99	c.m"
"	Rainfall losses	38.068	6.251	22.159	mm"
"	Runoff depth	23.291	55.108	39.200	mm"
"	Runoff volume	25.62	60.62	86.24	c.m"
"	Runoff coefficient	0.380	0.898	0.639	"
"	Maximum flow	0.011	0.032	0.036	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.036	0.188	0.000	0.000"	
" 54	POND DESIGN"				
"	0.188 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	446.7 Hydrograph volume	c.m"			
"	17. Number of stages"				

"	412.000	Minimum water level	metre"		
"	414.490	Maximum water level	metre"		
"	412.000	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	412.000	0.00031	0.000"		
"	412.100	0.00031	34.650"		
"	412.200	0.00031	69.310"		
"	412.300	0.00032	103.960"		
"	412.400	0.00032	138.610"		
"	412.500	0.00032	156.430"		
"	412.600	0.00032	174.250"		
"	412.700	0.00032	192.060"		
"	412.900	0.00032	192.430"		
"	413.230	0.00032	193.040"		
"	413.430	0.04528	193.730"		
"	413.630	0.1319	195.230"		
"	413.830	0.2482	197.730"		
"	414.030	0.3899	201.230"		
"	414.090	0.4369	202.580"		
"	414.290	0.6524	208.080"		
"	414.490	0.9278	214.580"		
"		Peak outflow	0.130	c.m/sec"	
"		Maximum level	413.661	metre"	
"		Maximum storage	195.615	c.m"	
"		Centroidal lag	39.670	hours"	
"		0.036 0.188 0.130 0.000		c.m/sec"	
" 40		HYDROGRAPH Combine	1000"		
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.130	c.m/sec"	
"		Hydrograph volume	422.535	c.m"	
"		0.036 0.188 0.130 0.130			
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.036 0.000 0.130 0.130			
" 33		CATCHMENT 1400"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1400	Catchment 1400"			
"	20.000	% Impervious"			
"	0.620	Total Area"			
"	30.000	Flow length"			
"	2.000	Overland Slope"			
"	0.496	Pervious Area"			
"	30.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.124	Impervious Area"			

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.380  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.900  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.056      0.000      0.130      0.130 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 16.700      2.054      11.252      minutes"
"      Time to Centroid      115.361      87.160      104.869      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      304.34      76.09      380.43      c.m"
"      Rainfall losses      38.059      6.144      31.676      mm"
"      Runoff depth      23.300      55.215      29.683      mm"
"      Runoff volume      115.57      68.47      184.04      c.m"
"      Runoff coefficient      0.380      0.900      0.484      "
"      Maximum flow      0.040      0.036      0.056      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.056      0.056      0.130      0.130"
" 54      POND DESIGN"
"      0.056      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"     184.0      Hydrograph volume      c.m"
"      17.      Number of stages"
"    413.920      Minimum water level      metre"
"    415.520      Maximum water level      metre"
"    413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"    413.920      0.00088      0.000"
"    414.020      0.00088      26.350"
"    414.120      0.00089      55.150"
"    414.220      0.00089      83.950"
"    414.320      0.00089      112.750"
"    414.420      0.00090      139.100"
"    414.520      0.00090      165.450"
"    414.620      0.00090      191.800"
"    414.720      0.00090      191.870"
"    414.820      0.02640      191.950"
"    414.920      0.03734      192.020"
"    415.020      0.04573      192.090"
"    415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.001	c.m/sec"	
"		Maximum level	414.422	metre"	
"		Maximum storage	139.619	c.m"	
"		Centroidal lag	30.244	hours"	
"		0.056	0.056	0.001	0.130 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.130	c.m/sec"	
"		Hydrograph volume	606.570	c.m"	
"		0.056	0.056	0.001	0.130"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.056	0.000	0.001	0.130"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.380	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.897	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.174	0.000	0.001	0.130 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	19.847	2.441	7.618 minutes"
"		Time to Centroid	118.992	87.742	97.037 minutes"
"		Rainfall depth	61.359	61.359	61.359 mm"

"	Rainfall volume	340.54	340.54	681.09	c.m"
"	Rainfall losses	38.054	6.310	22.182	mm"
"	Runoff depth	23.305	55.050	39.177	mm"
"	Runoff volume	129.34	305.52	434.87	c.m"
"	Runoff coefficient	0.380	0.897	0.638	"
"	Maximum flow	0.041	0.162	0.174	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.174 0.174 0.001 0.130"				
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.028		c.m/sec"	
"	Volume of diverted flow	10.733		c.m"	
"	DIV01500.010hyd"				
"	Major flow at 1500"				
"	0.174 0.174 0.146 0.130 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.174 0.146 0.146 0.130"				
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.380 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.905 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	1.094 0.146 0.146 0.130 c.m/sec"				
"	Catchment 1000 Pervious Impervious Total Area "				
"	Surface Area 3.490 3.490 6.980 hectare"				

"	Time of concentration	34.392	4.230	13.155	minutes"
"	Time to Centroid	135.773	90.174	103.666	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	2141.44	2141.44	4282.88	c.m"
"	Rainfall losses	38.029	5.843	21.936	mm"
"	Runoff depth	23.330	55.517	39.424	mm"
"	Runoff volume	814.23	1937.53	2751.76	c.m"
"	Runoff coefficient	0.380	0.905	0.643	"
"	Maximum flow	0.188	1.064	1.094	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	1.094 1.240 0.146 0.130"				
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.379 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.043 1.240 0.146 0.130 c.m/sec"				
"	Catchment 1100 Pervious Impervious Total Area "				
"	Surface Area 0.480 0.000 0.480 hectare"				
"	Time of concentration 13.094 1.611 13.094 minutes"				
"	Time to Centroid 111.234 86.563 111.234 minutes"				
"	Rainfall depth 61.359 61.359 61.359 mm"				
"	Rainfall volume 294.52 0.00 294.52 c.m"				
"	Rainfall losses 38.098 6.044 38.097 mm"				
"	Runoff depth 23.262 55.315 23.262 mm"				
"	Runoff volume 111.66 0.00 111.66 c.m"				
"	Runoff coefficient 0.379 0.000 0.379 "				
"	Maximum flow 0.043 0.000 0.043 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"          0.043      1.260      0.146      0.130"
" 54      POND DESIGN"
"          1.260  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          3287.6  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow      0.323      c.m/sec"
"          Maximum level      411.736      metre"
"          Maximum storage      1623.071      c.m"
"          Centroidal lag      3.233      hours"
"          0.043      1.260      0.323      0.130 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.043      0.323      0.323      0.130"
" 47      FILEI_O Read/Open DIV01500.010hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.010hyd"
"          Major flow at 1500"
"          Total volume      10.733      c.m"
"          Maximum flow      0.028      c.m/sec"
"          0.028      0.323      0.323      0.130 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.028      0.323      0.323      0.130"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	50.000	Pervious SCS Curve No. "			
"	0.073	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No. "			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.050	0.323	0.323	0.130 c.m/sec"
"		Catchment 4000	Pervious	Impervious	Total Area "
"		Surface Area	7.330	0.000	7.330 hectare"
"		Time of concentration	57.122	3.114	57.121 minutes"
"		Time to Centroid	167.459	88.727	167.458 minutes"
"		Rainfall depth	61.359	61.359	61.359 mm"
"		Rainfall volume	4497.63	0.00	4497.63 c.m"
"		Rainfall losses	56.901	6.470	56.901 mm"
"		Runoff depth	4.458	54.890	4.458 mm"
"		Runoff volume	326.76	0.00	326.77 c.m"
"		Runoff coefficient	0.073	0.000	0.073 "
"		Maximum flow	0.050	0.000	0.050 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.050	0.372	0.323	0.130"
" 54		POND DESIGN"			
"	0.372	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	3625.2	Hydrograph volume	c.m"		
"	6.	Number of stages"			
"	409.630	Minimum water level	metre"		
"	410.750	Maximum water level	metre"		
"	409.630	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	409.630	0.000	0.000"		
"	409.750	0.6650	402.200"		
"	410.000	3.601	2187.900"		
"	410.250	7.811	5318.900"		
"	410.500	12.984	9642.300"		
"	410.750	18.965	15227.70"		

"	Peak outflow	0.367	c.m/sec"
"	Maximum level	409.696	metre"
"	Maximum storage	222.278	c.m"
"	Centroidal lag	3.355	hours"
"	0.050 0.372 0.367 0.130		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.050 0.367 0.367 0.130		"
" 52	CHANNEL DESIGN"		
"	0.367 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	0.000 Basewidth	metre"	
"	7.410 Left bank slope"		
"	6.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.269	metre"
"	Velocity	0.759	m/sec"
"	Channel capacity	10.655	c.m/sec"
"	Critical depth	0.228	metre"
" 53	ROUTE Channel Route 72"		
"	72.40 Channel Route 72 Reach length	(metre)"	
"	0.433 X-factor <= 0.5"		
"	71.586 K-lag (seconds)"		
"	0.000 Default(0) or user spec.(1) values used"		
"	0.500 X-factor <= 0.5"		
"	30.000 K-lag (seconds)"		
"	0.500 Beta weighting factor"		
"	75.000 Routing time step (seconds)"		
"	1 No. of sub-reaches"		
"	Peak outflow	0.367	c.m/sec"
"	0.050 0.367 0.367 0.130		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.050 0.367 0.367 0.130		"
" 52	CHANNEL DESIGN"		
"	0.367 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	2.000 Basewidth	metre"	
"	2.950 Left bank slope"		
"	3.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.180	metre"
"	Velocity	0.806	m/sec"
"	Channel capacity	9.246	c.m/sec"
"	Critical depth	0.140	metre"
" 53	ROUTE Channel Route 40"		

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.378      X-factor <= 0.5"
"      37.024      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"      42.857      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                      0.367      c.m/sec"
"          0.050      0.367      0.367      0.130 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine "
"          100      Node #"
"          Existing Wetland"
"          Maximum flow                      0.367      c.m/sec"
"          Hydrograph volume                  3625.235      c.m"
"          0.050      0.367      0.367      0.367"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.050      0.000      0.367      0.367"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100      Catchment 2100"
"          60.000      % Impervious"
"          1.960      Total Area"
"          40.000      Flow length"
"          2.000      Overland Slope"
"          0.784      Pervious Area"
"          40.000      Pervious length"
"          2.000      Pervious slope"
"          1.176      Impervious Area"
"          40.000      Impervious length"
"          2.000      Impervious slope"
"          0.250      Pervious Manning 'n'"
"          78.000      Pervious SCS Curve No."
"          0.380      Pervious Runoff coefficient"
"          0.100      Pervious Ia/S coefficient"
"          7.164      Pervious Initial abstraction"
"          0.015      Impervious Manning 'n'"
"          98.000      Impervious SCS Curve No."
"          0.897      Impervious Runoff coefficient"
"          0.100      Impervious Ia/S coefficient"
"          0.518      Impervious Initial abstraction"
"          0.359      0.000      0.367      0.367 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area      0.784      1.176      1.960      hectare"
"          Time of concentration  19.847      2.441      6.272      minutes"

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"	Time to Centroid	118.992	87.742	94.620	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	481.06	721.58	1202.64	c.m"
"	Rainfall losses	38.054	6.310	19.008	mm"
"	Runoff depth	23.305	55.050	42.352	mm"
"	Runoff volume	182.71	647.38	830.09	c.m"
"	Runoff coefficient	0.380	0.897	0.690	"
"	Maximum flow	0.058	0.342	0.359	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.359 0.359 0.367 0.367"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.379 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.901 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.208 0.359 0.367 0.367 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 13.094 1.611 2.123 minutes"				
"	Time to Centroid 111.234 86.563 87.664 minutes"				
"	Rainfall depth 61.359 61.359 61.359 mm"				
"	Rainfall volume 49.09 441.79 490.87 c.m"				
"	Rainfall losses 38.098 6.044 9.250 mm"				
"	Runoff depth 23.262 55.315 52.110 mm"				
"	Runoff volume 18.61 398.27 416.88 c.m"				
"	Runoff coefficient 0.379 0.901 0.849 "				
"	Maximum flow 0.007 0.207 0.208 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.208	0.567	0.367	0.367"
" 54	POND DESIGN"				
"	0.567	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1247.0	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.141	c.m/sec"	
"		Maximum level	411.314	metre"	
"		Maximum storage	693.286	c.m"	
"		Centroidal lag	3.701	hours"	
"		0.208	0.567	0.141	0.367 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.208	0.141	0.141	0.367"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.379	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.901	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.045	0.141	0.141	0.367 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	13.094	1.611	10.694 minutes"
"		Time to Centroid	111.234	86.563	106.078 minutes"
"		Rainfall depth	61.359	61.359	61.359 mm"
"		Rainfall volume	265.07	29.45	294.52 c.m"
"		Rainfall losses	38.098	6.044	34.892 mm"
"		Runoff depth	23.262	55.315	26.467 mm"
"		Runoff volume	100.49	26.55	127.04 c.m"
"		Runoff coefficient	0.379	0.901	0.431 "
"		Maximum flow	0.039	0.014	0.045 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.045	0.178	0.141	0.367"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.045	0.178	0.178	0.367"
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.178		c.m/sec"
"		Hydrograph volume	1373.708		c.m"
"		0.045	0.178	0.178	0.178"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.045	0.000	0.178	0.178"
" 33		CATCHMENT 2200"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2200	Catchment 2200"			
"	75.000	% Impervious"			
"	0.920	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.230	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.690	Impervious Area"			
"	40.000	Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.380	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.897	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.206	0.000	0.178	0.178 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	19.847	2.441	4.594	minutes"
"		Time to Centroid	118.992	87.742	91.606	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	141.13	423.38	564.51	c.m"
"		Rainfall losses	38.054	6.310	14.246	mm"
"		Runoff depth	23.305	55.050	47.113	mm"
"		Runoff volume	53.60	379.84	433.44	c.m"
"		Runoff coefficient	0.380	0.897	0.768	"
"		Maximum flow	0.017	0.201	0.206	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.206	0.206	0.178	0.178"	
" 54		POND DESIGN"				
"	0.206	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	433.4	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.014	c.m/sec"		
"		Maximum level	414.033	metre"		

"	Maximum storage	338.951	c.m"
"	Centroidal lag	7.231	hours"
"	0.206 0.206 0.014 0.178	c.m/sec"	
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.190	c.m/sec"
"	Hydrograph volume	1807.152	c.m"
"	0.206 0.206 0.014	0.190"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.206 0.000 0.014	0.190"	
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.380 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.895 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.078 0.000 0.014 0.190	c.m/sec"	
"	Catchment 3100 Pervious Impervious Total Area "		
"	Surface Area 0.168 0.252 0.420	hectare"	
"	Time of concentration 24.434 3.005 7.732	minutes"	
"	Time to Centroid 124.292 88.564 96.444	minutes"	
"	Rainfall depth 61.359 61.359 61.359	mm"	
"	Rainfall volume 103.08 154.63 257.71	c.m"	
"	Rainfall losses 38.047 6.437 19.081	mm"	
"	Runoff depth 23.312 54.922 42.278	mm"	
"	Runoff volume 39.16 138.40 177.57	c.m"	
"	Runoff coefficient 0.380 0.895 0.689	"	
"	Maximum flow 0.011 0.075 0.078	c.m/sec"	

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.078      0.078      0.014      0.190"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"              0  Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.011      c.m/sec"
"              Volume of diverted flow      3.174      c.m"
"              DIV32001.010hyd"
"              Major flow at 32001"
"                  0.078      0.078      0.067      0.190 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.078      0.067      0.067      0.190"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          174.4  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"              410.620      0.000      0.000"
"              410.870      0.01300      4.855"
"              411.120      0.02000      14.351"
"              411.370      0.02500      24.074"
"              411.620      0.02900      33.921"
"              411.870      0.03300      43.768"
"              412.120      0.03600      53.614"
"              412.370      0.03900      63.461"
"              412.620      0.04200      73.308"
"              412.870      0.04400      74.155"
"              413.120      0.04700      75.003"
"              413.370      0.04900      75.850"
"              413.620      0.05200      76.698"
"              413.980      0.05500      77.918"
"              414.230      0.1600      78.483"
"              Peak outflow      0.033      c.m/sec"
"              Maximum level      411.906      metre"
"              Maximum storage      45.197      c.m"
"              Centroidal lag      1.881      hours"
"                  0.078      0.067      0.033      0.190 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.078      0.033      0.033      0.190"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3200	Catchment 3200"				
"	60.000	% Impervious"				
"	0.130	Total Area"				
"	20.000	Flow length"				
"	1.000	Overland Slope"				
"	0.052	Pervious Area"				
"	20.000	Pervious length"				
"	1.000	Pervious slope"				
"	0.078	Impervious Area"				
"	20.000	Impervious length"				
"	1.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	78.000	Pervious SCS Curve No."				
"	0.379	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n' "				
"	98.000	Impervious SCS Curve No."				
"	0.901	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.024	0.033	0.033	0.190 c.m/sec"
"		Catchment 3200	Pervious	Impervious	Total Area	"
"		Surface Area	0.052	0.078	0.130	hectare"
"		Time of concentration	16.121	1.983	5.083	minutes"
"		Time to Centroid	114.721	87.040	93.109	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	31.91	47.86	79.77	c.m"
"		Rainfall losses	38.081	6.103	18.894	mm"
"		Runoff depth	23.278	55.256	42.465	mm"
"		Runoff volume	12.10	43.10	55.20	c.m"
"		Runoff coefficient	0.379	0.901	0.692	"
"		Maximum flow	0.004	0.023	0.024	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.024	0.051	0.033	0.190"
" 33		CATCHMENT 3300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3300	Catchment 3300"				
"	60.000	% Impervious"				
"	0.240	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.096	Pervious Area"				
"	20.000	Pervious length"				

```

"      2.000  Pervious slope"
"      0.144  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.379  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.901  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.045      0.051      0.033      0.190 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 13.094      1.611      4.125      minutes"
"      Time to Centroid      111.234      86.563      91.965      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      58.90      88.36      147.26      c.m"
"      Rainfall losses      38.097      6.044      18.866      mm"
"      Runoff depth      23.262      55.315      42.494      mm"
"      Runoff volume      22.33      79.65      101.98      c.m"
"      Runoff coefficient      0.379      0.901      0.693      "
"      Maximum flow      0.009      0.041      0.045      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.045      0.096      0.033      0.190"
" 47      FILEI_0 Read/Open DIV32001.010hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.010hyd"
"      Major flow at 32001"
"      Total volume      3.174      c.m"
"      Maximum flow      0.011      c.m/sec"
"          0.011      0.096      0.033      0.190 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.011      0.107      0.033      0.190"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.011      0.107      0.107      0.190"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.107      c.m/sec"
"      Hydrograph volume      334.694      c.m"

```

"		0.011	0.107	0.107	0.107"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.107		c.m/sec"	
"		Hydrograph volume	334.694		c.m"	
"		0.011	0.107	0.107	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.011	0.107	0.107	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.395		c.m/sec"	
"		Hydrograph volume	3959.929		c.m"	
"		0.011	0.107	0.107	0.395"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.395		c.m/sec"	
"		Hydrograph volume	3959.929		c.m"	
"		0.011	0.395	0.107	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.011	0.395	0.395	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.551		c.m/sec"	
"		Hydrograph volume	5767.070		c.m"	
"		0.011	0.395	0.395	0.551"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.551		c.m/sec"	
"		Hydrograph volume	5767.070		c.m"	
"		0.011	0.551	0.395	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__25yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:43:00 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          3701.648 Coefficient A"
"          25.500  Constant B"
"          0.937  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          143.371  mm/hr"
"          Total depth                75.581  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.440  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.899  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"	0.047	0.000	0.000	0.000 c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area "
"	Surface Area	0.110	0.110	0.220 hectare"
"	Time of concentration	7.622	1.005	3.181 minutes"
"	Time to Centroid	103.628	85.398	91.394 minutes"
"	Rainfall depth	75.581	75.581	75.581 mm"
"	Rainfall volume	83.14	83.14	166.28 c.m"
"	Rainfall losses	42.296	7.669	24.982 mm"
"	Runoff depth	33.285	67.912	50.598 mm"
"	Runoff volume	36.61	74.70	111.32 c.m"
"	Runoff coefficient	0.440	0.899	"
"	Maximum flow	0.018	0.038	0.047 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.047	0.047	0.000	0.000"
---	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n' "
"	78.000	Pervious SCS Curve No. "
"	0.441	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n' "
"	98.000	Impervious SCS Curve No. "
"	0.916	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.144	0.047	0.000	0.000 c.m/sec"
---	-------	-------	-------	----------------

"	Catchment 1300	Pervious	Impervious	Total Area "
"	Surface Area	0.350	0.350	0.700 hectare"
"	Time of concentration	11.553	1.523	4.782 minutes"
"	Time to Centroid	108.042	85.984	93.151 minutes"
"	Rainfall depth	75.581	75.581	75.581 mm"
"	Rainfall volume	264.53	264.53	529.07 c.m"
"	Rainfall losses	42.253	6.330	24.292 mm"
"	Runoff depth	33.328	69.250	51.289 mm"
"	Runoff volume	116.65	242.38	359.02 c.m"

"	Runoff coefficient	0.441	0.916	0.679	"
"	Maximum flow	0.047	0.118	0.144	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.144	0.191	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.439 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.912 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.046	0.191	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	9.721	1.281	4.026	minutes"
"	Time to Centroid	106.001	85.673	92.284	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	83.14	83.14	166.28	c.m"
"	Rainfall losses	42.377	6.681	24.529	mm"
"	Runoff depth	33.204	68.900	51.052	mm"
"	Runoff volume	36.52	75.79	112.31	c.m"
"	Runoff coefficient	0.439	0.912	0.675	"
"	Maximum flow	0.016	0.038	0.046	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.046	0.238	0.000	0.000"	
" 54	POND DESIGN"				
"	0.238 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	582.7 Hydrograph volume	c.m"			
"	17. Number of stages"				

"	412.000	Minimum water level	metre"		
"	414.490	Maximum water level	metre"		
"	412.000	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	412.000	0.00031	0.000"		
"	412.100	0.00031	34.650"		
"	412.200	0.00031	69.310"		
"	412.300	0.00032	103.960"		
"	412.400	0.00032	138.610"		
"	412.500	0.00032	156.430"		
"	412.600	0.00032	174.250"		
"	412.700	0.00032	192.060"		
"	412.900	0.00032	192.430"		
"	413.230	0.00032	193.040"		
"	413.430	0.04528	193.730"		
"	413.630	0.1319	195.230"		
"	413.830	0.2482	197.730"		
"	414.030	0.3899	201.230"		
"	414.090	0.4369	202.580"		
"	414.290	0.6524	208.080"		
"	414.490	0.9278	214.580"		
"		Peak outflow	0.213	c.m/sec"	
"		Maximum level	413.791	metre"	
"		Maximum storage	197.238	c.m"	
"		Centroidal lag	30.349	hours"	
"		0.046	0.238	0.213	0.000 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.213	c.m/sec"	
"		Hydrograph volume	559.248	c.m"	
"		0.046	0.238	0.213	0.213"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.046	0.000	0.213	0.213"
" 33		CATCHMENT 1400"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1400	Catchment 1400"			
"	20.000	% Impervious"			
"	0.620	Total Area"			
"	30.000	Flow length"			
"	2.000	Overland Slope"			
"	0.496	Pervious Area"			
"	30.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.124	Impervious Area"			

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.441  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.916  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.082      0.000      0.213      0.213 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 14.734      1.942      10.366      minutes"
"      Time to Centroid 111.700      86.497      103.093      minutes"
"      Rainfall depth 75.581      75.581      75.581      mm"
"      Rainfall volume 374.88      93.72      468.60      c.m"
"      Rainfall losses 42.213      6.364      35.043      mm"
"      Runoff depth 33.368      69.217      40.538      mm"
"      Runoff volume 165.50      85.83      251.33      c.m"
"      Runoff coefficient 0.441      0.916      0.536      "
"      Maximum flow 0.061      0.042      0.082      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.082      0.082      0.213      0.213"
" 54      POND DESIGN"
"      0.082      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      251.3      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.017	c.m/sec"	
"		Maximum level	414.821	metre"	
"		Maximum storage	191.951	c.m"	
"		Centroidal lag	27.025	hours"	
"		0.082	0.082	0.017	0.213 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.213	c.m/sec"	
"		Hydrograph volume	808.641	c.m"	
"		0.082	0.082	0.017	0.213"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.082	0.000	0.017	0.213"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.442	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.913	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.213	0.000	0.017	0.213 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	17.510	2.308	7.265 minutes"
"		Time to Centroid	114.842	87.059	96.118 minutes"
"		Rainfall depth	75.581	75.581	75.581 mm"

"	Rainfall volume	419.47	419.47	838.95	c.m"
"	Rainfall losses	42.200	6.593	24.396	mm"
"	Runoff depth	33.381	68.988	51.185	mm"
"	Runoff volume	185.27	382.88	568.15	c.m"
"	Runoff coefficient	0.442	0.913	0.677	"
"	Maximum flow	0.064	0.190	0.213	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.213 0.213 0.017 0.213"				
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.067		c.m/sec"	
"	Volume of diverted flow	46.077		c.m"	
"	DIV01500.025hyd"				
"	Major flow at 1500"				
"	0.213 0.213 0.146 0.213 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.213 0.146 0.146 0.213"				
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.442 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.917 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	1.292 0.146 0.146 0.213 c.m/sec"				
"	Catchment 1000 Pervious Impervious Total Area "				
"	Surface Area 3.490 3.490 6.980 hectare"				

"	Time of concentration	30.343	4.000	12.568	minutes"
"	Time to Centroid	129.499	89.355	102.412	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	2637.77	2637.77	5275.54	c.m"
"	Rainfall losses	42.180	6.290	24.235	mm"
"	Runoff depth	33.401	69.291	51.346	mm"
"	Runoff volume	1165.68	2418.25	3583.93	c.m"
"	Runoff coefficient	0.442	0.917	0.679	"
"	Maximum flow	0.298	1.235	1.292	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		1.292	1.438	0.146	0.213"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.441 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.065	1.438	0.146	0.213 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	11.553	1.523	11.553	minutes"
"	Time to Centroid	108.042	85.984	108.042	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	362.79	0.00	362.79	c.m"
"	Rainfall losses	42.253	6.330	42.253	mm"
"	Runoff depth	33.328	69.250	33.328	mm"
"	Runoff volume	159.97	0.00	159.98	c.m"
"	Runoff coefficient	0.441	0.000	0.441	"
"	Maximum flow	0.065	0.000	0.065	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"          0.065      1.476      0.146      0.213"
" 54      POND DESIGN"
"          1.476  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          4266.0  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow      0.398      c.m/sec"
"          Maximum level      411.858      metre"
"          Maximum storage      2237.928      c.m"
"          Centroidal lag      3.270      hours"
"          0.065      1.476      0.398      0.213 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.065      0.398      0.398      0.213"
" 47      FILEI_0 Read/Open DIV01500.025hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.025hyd"
"          Major flow at 1500"
"          Total volume      46.077      c.m"
"          Maximum flow      0.067      c.m/sec"
"          0.067      0.398      0.398      0.213 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.067      0.398      0.398      0.213"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	50.000	Pervious SCS Curve No. "			
"	0.109	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No. "			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.114	0.398	0.398	0.213 c.m/sec"
"		Catchment 4000	Pervious	Impervious	Total Area "
"		Surface Area	7.330	0.000	7.330 hectare"
"		Time of concentration	43.720	2.944	43.719 minutes"
"		Time to Centroid	151.037	87.974	151.037 minutes"
"		Rainfall depth	75.581	75.581	75.581 mm"
"		Rainfall volume	5540.07	0.01	5540.08 c.m"
"		Rainfall losses	67.305	6.942	67.305 mm"
"		Runoff depth	8.276	68.639	8.276 mm"
"		Runoff volume	606.62	0.01	606.62 c.m"
"		Runoff coefficient	0.109	0.000	0.109 "
"		Maximum flow	0.114	0.000	0.114 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.114	0.512	0.398	0.213"
" 54		POND DESIGN"			
"	0.512	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	4920.7	Hydrograph volume	c.m"		
"	6.	Number of stages"			
"	409.630	Minimum water level	metre"		
"	410.750	Maximum water level	metre"		
"	409.630	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	409.630	0.000	0.000"		
"	409.750	0.6650	402.200"		
"	410.000	3.601	2187.900"		
"	410.250	7.811	5318.900"		
"	410.500	12.984	9642.300"		
"	410.750	18.965	15227.70"		

"	Peak outflow	0.485	c.m/sec"
"	Maximum level	409.717	metre"
"	Maximum storage	293.288	c.m"
"	Centroidal lag	3.326	hours"
"	0.114 0.512 0.485 0.213		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.114 0.485 0.485 0.213		
" 52	CHANNEL DESIGN"		
"	0.485 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	0.000 Basewidth	metre"	
"	7.410 Left bank slope"		
"	6.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.298	metre"
"	Velocity	0.813	m/sec"
"	Channel capacity	10.655	c.m/sec"
"	Critical depth	0.254	metre"
" 53	ROUTE Channel Route 72"		
"	72.40 Channel Route 72 Reach length	(metre)"	
"	0.426 X-factor <= 0.5"		
"	66.766 K-lag (seconds)"		
"	0.000 Default(0) or user spec.(1) values used"		
"	0.500 X-factor <= 0.5"		
"	30.000 K-lag (seconds)"		
"	0.500 Beta weighting factor"		
"	75.000 Routing time step (seconds)"		
"	1 No. of sub-reaches"		
"	Peak outflow	0.484	c.m/sec"
"	0.114 0.485 0.484 0.213		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.114 0.484 0.484 0.213		
" 52	CHANNEL DESIGN"		
"	0.484 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	2.000 Basewidth	metre"	
"	2.950 Left bank slope"		
"	3.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.210	metre"
"	Velocity	0.880	m/sec"
"	Channel capacity	9.246	c.m/sec"
"	Critical depth	0.166	metre"
" 53	ROUTE Channel Route 40"		

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.359      X-factor <= 0.5"
"      33.931      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"      42.857      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                      0.482      c.m/sec"
"          0.114      0.484      0.482      0.213 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine "
"          100      Node #"
"          Existing Wetland"
"          Maximum flow                      0.482      c.m/sec"
"          Hydrograph volume                  4920.682      c.m"
"          0.114      0.484      0.482      0.482"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.114      0.000      0.482      0.482"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100      Catchment 2100"
"          60.000      % Impervious"
"          1.960      Total Area"
"          40.000      Flow length"
"          2.000      Overland Slope"
"          0.784      Pervious Area"
"          40.000      Pervious length"
"          2.000      Pervious slope"
"          1.176      Impervious Area"
"          40.000      Impervious length"
"          2.000      Impervious slope"
"          0.250      Pervious Manning 'n'"
"          78.000      Pervious SCS Curve No."
"          0.442      Pervious Runoff coefficient"
"          0.100      Pervious Ia/S coefficient"
"          7.164      Pervious Initial abstraction"
"          0.015      Impervious Manning 'n'"
"          98.000      Impervious SCS Curve No."
"          0.913      Impervious Runoff coefficient"
"          0.100      Impervious Ia/S coefficient"
"          0.518      Impervious Initial abstraction"
"          0.435      0.000      0.482      0.482 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area      0.784      1.176      1.960      hectare"
"          Time of concentration  17.510      2.308      6.016      minutes"

```

"	Time to Centroid	114.842	87.059	93.835	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	592.55	888.83	1481.38	c.m"
"	Rainfall losses	42.200	6.593	20.835	mm"
"	Runoff depth	33.381	68.988	54.745	mm"
"	Runoff volume	261.71	811.30	1073.01	c.m"
"	Runoff coefficient	0.442	0.913	0.724	"
"	Maximum flow	0.090	0.402	0.435	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.435 0.435 0.482 0.482"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.441 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.916 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.246 0.435 0.482 0.482 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 11.553 1.523 2.032 minutes"				
"	Time to Centroid 108.042 85.984 87.103 minutes"				
"	Rainfall depth 75.581 75.581 75.581 mm"				
"	Rainfall volume 60.46 544.18 604.65 c.m"				
"	Rainfall losses 42.253 6.330 9.923 mm"				
"	Runoff depth 33.328 69.250 65.658 mm"				
"	Runoff volume 26.66 498.60 525.27 c.m"				
"	Runoff coefficient 0.441 0.916 0.869 "				
"	Maximum flow 0.011 0.243 0.246 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.246	0.680	0.482	0.482"
" 54	POND DESIGN"				
"	0.680	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1598.3	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.161	c.m/sec"	
"		Maximum level	411.484	metre"	
"		Maximum storage	911.685	c.m"	
"		Centroidal lag	3.538	hours"	
"		0.246	0.680	0.161	0.482 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.246	0.161	0.161	0.482"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.441	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.916	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.067	0.161	0.161	0.482 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	11.553	1.523	9.671 minutes"
"		Time to Centroid	108.042	85.984	103.905 minutes"
"		Rainfall depth	75.581	75.581	75.581 mm"
"		Rainfall volume	326.51	36.28	362.79 c.m"
"		Rainfall losses	42.253	6.330	38.660 mm"
"		Runoff depth	33.328	69.250	36.920 mm"
"		Runoff volume	143.98	33.24	177.22 c.m"
"		Runoff coefficient	0.441	0.916	0.488 "
"		Maximum flow	0.059	0.016	0.067 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.067	0.214	0.161	0.482"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.067	0.214	0.214	0.482"
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.214		c.m/sec"
"		Hydrograph volume	1777.141		c.m"
"		0.067	0.214	0.214	0.214"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.067	0.000	0.214	0.214"
" 33		CATCHMENT 2200"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2200	Catchment 2200"			
"	75.000	% Impervious"			
"	0.920	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.230	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.690	Impervious Area"			
"	40.000	Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.442	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.913	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.245	0.000	0.214	0.214 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	17.510	2.308	4.420	minutes"
"		Time to Centroid	114.842	87.059	90.917	minutes"
"		Rainfall depth	75.581	75.581	75.581	mm"
"		Rainfall volume	173.84	521.51	695.34	c.m"
"		Rainfall losses	42.200	6.593	15.494	mm"
"		Runoff depth	33.381	68.988	60.086	mm"
"		Runoff volume	76.78	476.02	552.80	c.m"
"		Runoff coefficient	0.442	0.913	0.795	"
"		Maximum flow	0.026	0.236	0.245	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.245	0.245	0.214	0.214"	
" 54		POND DESIGN"				
"	0.245	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	552.8	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.027	c.m/sec"		
"		Maximum level	414.105	metre"		

"	Maximum storage	428.861	c.m"
"	Centroidal lag	7.439	hours"
"	0.245 0.245 0.027 0.214	c.m/sec"	
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.227	c.m/sec"
"	Hydrograph volume	2329.845	c.m"
"	0.245 0.245 0.027	0.227"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.245 0.000 0.027	0.227"	
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.442 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.908 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.091 0.000 0.027	0.227 c.m/sec"	
"	Catchment 3100	Pervious	Impervious Total Area "
"	Surface Area	0.168	0.252 0.420 hectare"
"	Time of concentration	21.558	2.842 7.426 minutes"
"	Time to Centroid	119.480	87.822 95.576 minutes"
"	Rainfall depth	75.581	75.581 75.581 mm"
"	Rainfall volume	126.98	190.46 317.44 c.m"
"	Rainfall losses	42.201	6.986 21.072 mm"
"	Runoff depth	33.380	68.594 54.509 mm"
"	Runoff volume	56.08	172.86 228.94 c.m"
"	Runoff coefficient	0.442	0.908 0.721 "
"	Maximum flow	0.017	0.086 0.091 c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.091      0.091      0.027      0.227"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"              0  Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.024      c.m/sec"
"              Volume of diverted flow      12.772      c.m"
"              DIV32001.025hyd"
"              Major flow at 32001"
"                  0.091      0.091      0.067      0.227 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.091      0.067      0.067      0.227"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          216.2  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"              410.620      0.000      0.000"
"              410.870      0.01300      4.855"
"              411.120      0.02000      14.351"
"              411.370      0.02500      24.074"
"              411.620      0.02900      33.921"
"              411.870      0.03300      43.768"
"              412.120      0.03600      53.614"
"              412.370      0.03900      63.461"
"              412.620      0.04200      73.308"
"              412.870      0.04400      74.155"
"              413.120      0.04700      75.003"
"              413.370      0.04900      75.850"
"              413.620      0.05200      76.698"
"              413.980      0.05500      77.918"
"              414.230      0.1600      78.483"
"              Peak outflow      0.038      c.m/sec"
"              Maximum level      412.267      metre"
"              Maximum storage      59.408      c.m"
"              Centroidal lag      1.926      hours"
"                  0.091      0.067      0.038      0.227 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.091      0.038      0.038      0.227"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	3200	Catchment 3200"
"	60.000	% Impervious"
"	0.130	Total Area"
"	20.000	Flow length"
"	1.000	Overland Slope"
"	0.052	Pervious Area"
"	20.000	Pervious length"
"	1.000	Pervious slope"
"	0.078	Impervious Area"
"	20.000	Impervious length"
"	1.000	Impervious slope"
"	0.250	Pervious Manning 'n' "
"	78.000	Pervious SCS Curve No."
"	0.441	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n' "
"	98.000	Impervious SCS Curve No."
"	0.916	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"
"	0.029	0.038 0.038 0.227 c.m/sec"
"	Catchment 3200	Pervious Impervious Total Area "
"	Surface Area	0.052 0.078 0.130 hectare"
"	Time of concentration	14.223 1.875 4.878 minutes"
"	Time to Centroid	111.113 86.404 92.413 minutes"
"	Rainfall depth	75.581 75.581 75.581 mm"
"	Rainfall volume	39.30 58.95 98.26 c.m"
"	Rainfall losses	42.219 6.365 20.706 mm"
"	Runoff depth	33.362 69.216 54.874 mm"
"	Runoff volume	17.35 53.99 71.34 c.m"
"	Runoff coefficient	0.441 0.916 0.726 "
"	Maximum flow	0.007 0.026 0.029 c.m/sec"
" 40	HYDROGRAPH Add Runoff "	
"	4 Add Runoff "	
"	0.029 0.058 0.038 0.227"	
" 33	CATCHMENT 3300"	
"	1 Triangular SCS"	
"	1 Equal length"	
"	1 SCS method"	
"	3300 Catchment 3300"	
"	60.000 % Impervious"	
"	0.240 Total Area"	
"	20.000 Flow length"	
"	2.000 Overland Slope"	
"	0.096 Pervious Area"	
"	20.000 Pervious length"	

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.441 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.916 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.055      0.058      0.038      0.227 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 11.553      1.523      3.959      minutes"
"      Time to Centroid 108.042      85.984      91.342      minutes"
"      Rainfall depth 75.581      75.581      75.581      mm"
"      Rainfall volume 72.56      108.84      181.39      c.m"
"      Rainfall losses 42.253      6.330      20.699      mm"
"      Runoff depth 33.328      69.250      54.881      mm"
"      Runoff volume 31.99      99.72      131.72      c.m"
"      Runoff coefficient 0.441      0.916      0.726      "
"      Maximum flow 0.013      0.049      0.055      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.055      0.113      0.038      0.227"
" 47      FILEI_0 Read/Open DIV32001.025hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.025hyd"
"      Major flow at 32001"
"      Total volume      12.772      c.m"
"      Maximum flow      0.024      c.m/sec"
"          0.024      0.113      0.038      0.227 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.024      0.137      0.038      0.227"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.024      0.137      0.137      0.227"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.137      c.m/sec"
"      Hydrograph volume 431.922      c.m"

```

"		0.024	0.137	0.137	0.137"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.137		c.m/sec"	
"		Hydrograph volume	431.922		c.m"	
"		0.024	0.137	0.137	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.024	0.137	0.137	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.524		c.m/sec"	
"		Hydrograph volume	5352.604		c.m"	
"		0.024	0.137	0.137	0.524"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.524		c.m/sec"	
"		Hydrograph volume	5352.604		c.m"	
"		0.024	0.524	0.137	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.024	0.524	0.524	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.719		c.m/sec"	
"		Hydrograph volume	7682.439		c.m"	
"		0.024	0.524	0.524	0.719"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.719		c.m/sec"	
"		Hydrograph volume	7682.439		c.m"	
"		0.024	0.719	0.524	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__50yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:46:50 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          5089.418 Coefficient A"
"          30.000  Constant B"
"          0.967  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          156.350  mm/hr"
"          Total depth                86.737  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.479  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.904  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"	0.055	0.000	0.000	0.000 c.m/sec"	
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	7.056	0.968	3.076	minutes"
"	Time to Centroid	102.283	85.178	91.100	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	95.41	95.41	190.82	c.m"
"	Rainfall losses	45.201	8.296	26.748	mm"
"	Runoff depth	41.536	78.441	59.988	mm"
"	Runoff volume	45.69	86.28	131.97	c.m"
"	Runoff coefficient	0.479	0.904	0.692	"
"	Maximum flow	0.021	0.041	0.055	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.055	0.055	0.000	0.000"
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" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.481	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.924	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.169	0.055	0.000	0.000 c.m/sec"
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"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	10.695	1.467	4.627	minutes"
"	Time to Centroid	106.283	85.675	92.731	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	303.58	303.58	607.16	c.m"
"	Rainfall losses	44.994	6.561	25.777	mm"
"	Runoff depth	41.743	80.176	60.959	mm"
"	Runoff volume	146.10	280.61	426.72	c.m"

"	Runoff coefficient	0.481	0.924	0.703	"
"	Maximum flow	0.060	0.131	0.169	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.169	0.224	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.480 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.919 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.054	0.224	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	9.000	1.235	3.900	minutes"
"	Time to Centroid	104.357	85.382	91.895	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	95.41	95.41	190.82	c.m"
"	Rainfall losses	45.077	7.038	26.057	mm"
"	Runoff depth	41.660	79.699	60.679	mm"
"	Runoff volume	45.83	87.67	133.49	c.m"
"	Runoff coefficient	0.480	0.919	0.700	"
"	Maximum flow	0.020	0.042	0.054	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.054	0.279	0.000	0.000"	
" 54	POND DESIGN"				
"	0.279 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	692.2 Hydrograph volume	c.m"			
"	17. Number of stages"				

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"      412.000  Minimum water level    metre"
"      414.490  Maximum water level    metre"
"      412.000  Starting water level    metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"      412.000  0.00031    0.000"
"      412.100  0.00031    34.650"
"      412.200  0.00031    69.310"
"      412.300  0.00032   103.960"
"      412.400  0.00032   138.610"
"      412.500  0.00032   156.430"
"      412.600  0.00032   174.250"
"      412.700  0.00032   192.060"
"      412.900  0.00032   192.430"
"      413.230  0.00032   193.040"
"      413.430  0.04528   193.730"
"      413.630  0.1319    195.230"
"      413.830  0.2482    197.730"
"      414.030  0.3899    201.230"
"      414.090  0.4369    202.580"
"      414.290  0.6524    208.080"
"      414.490  0.9278    214.580"
"          Peak outflow          0.270    c.m/sec"
"          Maximum level          413.868    metre"
"          Maximum storage          198.404    c.m"
"          Centroidal lag          24.205    hours"
"          0.054    0.279    0.270    0.000 c.m/sec"
" 40      HYDROGRAPH    Combine    1000"
"          6    Combine "
"      1000    Node #"
"          Infiltrated On-Site"
"          Maximum flow          0.270    c.m/sec"
"          Hydrograph volume          710.278    c.m"
"          0.054    0.279    0.270    0.270"
" 40      HYDROGRAPH Start - New Tributary"
"          2    Start - New Tributary"
"          0.054    0.000    0.270    0.270"
" 33      CATCHMENT 1400"
"          1    Triangular SCS"
"          1    Equal length"
"          1    SCS method"
"      1400    Catchment 1400"
"      20.000    % Impervious"
"          0.620    Total Area"
"      30.000    Flow length"
"          2.000    Overland Slope"
"          0.496    Pervious Area"
"      30.000    Pervious length"
"          2.000    Pervious slope"
"          0.124    Impervious Area"

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.482  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.925  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.105      0.000      0.270      0.270 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 13.641      1.872      9.825      minutes"
"      Time to Centroid      109.632      86.148      102.018      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      430.21      107.55      537.77      c.m"
"      Rainfall losses      44.941      6.526      37.258      mm"
"      Runoff depth      41.796      80.211      49.479      mm"
"      Runoff volume      207.31      99.46      306.77      c.m"
"      Runoff coefficient      0.482      0.925      0.570      "
"      Maximum flow      0.079      0.047      0.105      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.105      0.105      0.270      0.270"
" 54      POND DESIGN"
"      0.105      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      306.8      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.046	c.m/sec"	
"		Maximum level	415.121	metre"	
"		Maximum storage	192.296	c.m"	
"		Centroidal lag	22.464	hours"	
"		0.105	0.105	0.046	0.270 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.271	c.m/sec"	
"		Hydrograph volume	1014.930	c.m"	
"		0.105	0.105	0.046	0.271"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.105	0.000	0.046	0.271"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.481	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.922	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.242	0.000	0.046	0.271 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	16.211	2.224	7.023 minutes"
"		Time to Centroid	112.570	86.667	95.554 minutes"
"		Rainfall depth	86.737	86.737	86.737 mm"

"	Rainfall volume	481.39	481.39	962.78	c.m"
"	Rainfall losses	44.974	6.774	25.874	mm"
"	Runoff depth	41.763	79.963	60.863	mm"
"	Runoff volume	231.78	443.80	675.58	c.m"
"	Runoff coefficient	0.481	0.922	0.702	"
"	Maximum flow	0.084	0.210	0.242	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.242	0.242	0.046	0.271"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.096			c.m/sec"
"	Volume of diverted flow	90.793			c.m"
"	DIV01500.050hyd"				
"	Major flow at 1500"				
"		0.242	0.242	0.146	0.271 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.242	0.146	0.146	0.271"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.482 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.923 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		1.452	0.146	0.146	0.271 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	28.092	3.854	12.172	minutes"
"	Time to Centroid	125.992	88.892	101.624	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	3027.11	3027.11	6054.23	c.m"
"	Rainfall losses	44.916	6.695	25.806	mm"
"	Runoff depth	41.821	80.042	60.931	mm"
"	Runoff volume	1459.54	2793.46	4253.00	c.m"
"	Runoff coefficient	0.482	0.923	0.702	"
"	Maximum flow	0.393	1.367	1.452	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	1.452 1.598 0.146 0.271"				
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.481 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.082 1.598 0.146 0.271 c.m/sec"				
"	Catchment 1100 Pervious Impervious Total Area "				
"	Surface Area 0.480 0.000 0.480 hectare"				
"	Time of concentration 10.695 1.467 10.695 minutes"				
"	Time to Centroid 106.283 85.675 106.283 minutes"				
"	Rainfall depth 86.737 86.737 86.737 mm"				
"	Rainfall volume 416.34 0.00 416.34 c.m"				
"	Rainfall losses 44.994 6.561 44.994 mm"				
"	Runoff depth 41.743 80.176 41.743 mm"				
"	Runoff volume 200.37 0.00 200.37 c.m"				
"	Runoff coefficient 0.481 0.000 0.481 "				
"	Maximum flow 0.082 0.000 0.082 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				

```

"          4  Add Runoff "
"              0.082      1.654      0.146      0.271"
" 54      POND DESIGN"
"          1.654  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          5038.2  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow      0.658      c.m/sec"
"          Maximum level      411.899      metre"
"          Maximum storage      2458.114      c.m"
"          Centroidal lag      3.134      hours"
"              0.082      1.654      0.658      0.271 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5      Next link "
"              0.082      0.658      0.658      0.271"
" 47      FILEI_0 Read/Open DIV01500.050hyd"
"          1      1=read/open; 2=write/save"
"          2      1=rainfall; 2=hydrograph"
"          1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.050hyd"
"          Major flow at 1500"
"          Total volume      90.793      c.m"
"          Maximum flow      0.096      c.m/sec"
"              0.096      0.658      0.658      0.271 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"              0.096      0.658      0.658      0.271"
" 33      CATCHMENT 4000"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          4000      Catchment 4000"
"          0.000      % Impervious"

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"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	50.000	Pervious SCS Curve No."			
"	0.137	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.183	0.658	0.658	0.271 c.m/sec"
"		Catchment 4000	Pervious	Impervious	Total Area "
"		Surface Area	7.330	0.000	7.330 hectare"
"		Time of concentration	37.472	2.837	37.472 minutes"
"		Time to Centroid	142.983	87.552	142.982 minutes"
"		Rainfall depth	86.737	86.737	86.737 mm"
"		Rainfall volume	6357.80	0.01	6357.80 c.m"
"		Rainfall losses	74.812	7.307	74.812 mm"
"		Runoff depth	11.925	79.429	11.925 mm"
"		Runoff volume	874.09	0.01	874.10 c.m"
"		Runoff coefficient	0.137	0.000	0.137 "
"		Maximum flow	0.183	0.000	0.183 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.183	0.841	0.658	0.271"
" 54		POND DESIGN"			
"	0.841	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	6000.2	Hydrograph volume	c.m"		
"	6.	Number of stages"			
"	409.630	Minimum water level	metre"		
"	410.750	Maximum water level	metre"		
"	409.630	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	409.630	0.000	0.000"		
"	409.750	0.6650	402.200"		
"	410.000	3.601	2187.900"		
"	410.250	7.811	5318.900"		
"	410.500	12.984	9642.300"		
"	410.750	18.965	15227.70"		

"	Peak outflow	0.768	c.m/sec"
"	Maximum level	409.759	metre"
"	Maximum storage	466.326	c.m"
"	Centroidal lag	3.164	hours"
"	0.183 0.841 0.768 0.271		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.183 0.768 0.768 0.271		"
" 52	CHANNEL DESIGN"		
"	0.768 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	0.000 Basewidth	metre"	
"	7.410 Left bank slope"		
"	6.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.354	metre"
"	Velocity	0.912	m/sec"
"	Channel capacity	10.655	c.m/sec"
"	Critical depth	0.306	metre"
" 53	ROUTE Channel Route 72"		
"	72.40 Channel Route 72 Reach length	(metre)"	
"	0.412 X-factor <= 0.5"		
"	59.519 K-lag (seconds)"		
"	0.000 Default(0) or user spec.(1) values used"		
"	0.500 X-factor <= 0.5"		
"	30.000 K-lag (seconds)"		
"	0.500 Beta weighting factor"		
"	60.000 Routing time step (seconds)"		
"	1 No. of sub-reaches"		
"	Peak outflow	0.767	c.m/sec"
"	0.183 0.768 0.767 0.271		c.m/sec"
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.183 0.767 0.767 0.271		"
" 52	CHANNEL DESIGN"		
"	0.767 Current peak flow	c.m/sec"	
"	0.035 Manning 'n'"		
"	0. Cross-section type: 0=trapezoidal; 1=general"		
"	2.000 Basewidth	metre"	
"	2.950 Left bank slope"		
"	3.000 Right bank slope"		
"	0.950 Channel depth	metre"	
"	1.040 Gradient	%"	
"	Depth of flow	0.270	metre"
"	Velocity	1.013	m/sec"
"	Channel capacity	9.246	c.m/sec"
"	Critical depth	0.220	metre"
" 53	ROUTE Channel Route 40"		

"	39.80	Channel Route 40 Reach length	(metre)"		
"	0.323	X-factor <= 0.5"			
"	29.473	K-lag (seconds)"			
"	0.000	Default(0) or user spec.(1) values used"			
"	0.500	X-factor <= 0.5"			
"	30.000	K-lag (seconds)"			
"	0.500	Beta weighting factor"			
"	37.500	Routing time step (seconds)"			
"	1	No. of sub-reaches"			
"		Peak outflow	0.766	c.m/sec"	
"		0.183 0.767 0.766	0.271	c.m/sec"	
" 40		HYDROGRAPH Combine 100"			
"	6	Combine "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	0.766	c.m/sec"	
"		Hydrograph volume	6000.176	c.m"	
"		0.183 0.767 0.766	0.766"		
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.183 0.000 0.766	0.766"		
" 33		CATCHMENT 2100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2100	Catchment 2100"			
"	60.000	% Impervious"			
"	1.960	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.784	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	1.176	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.481	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.922	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.491 0.000 0.766	0.766	c.m/sec"	
"		Catchment 2100 Pervious Impervious Total Area "			
"		Surface Area 0.784 1.176 1.960		hectare"	
"		Time of concentration 16.211 2.224 5.836		minutes"	

"	Time to Centroid	112.570	86.667	93.357	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	680.02	1020.02	1700.04	c.m"
"	Rainfall losses	44.974	6.773	22.054	mm"
"	Runoff depth	41.763	79.963	64.683	mm"
"	Runoff volume	327.42	940.37	1267.79	c.m"
"	Runoff coefficient	0.481	0.922	0.746	"
"	Maximum flow	0.119	0.446	0.491	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.491 0.491 0.766 0.766"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.481 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.924 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.274 0.491 0.766 0.766 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 10.695 1.467 1.972 minutes"				
"	Time to Centroid 106.283 85.675 86.802 minutes"				
"	Rainfall depth 86.737 86.737 86.737 mm"				
"	Rainfall volume 69.39 624.50 693.89 c.m"				
"	Rainfall losses 44.994 6.561 10.404 mm"				
"	Runoff depth 41.743 80.176 76.332 mm"				
"	Runoff volume 33.39 577.26 610.66 c.m"				
"	Runoff coefficient 0.481 0.924 0.880 "				
"	Maximum flow 0.014 0.270 0.274 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.274	0.764	0.766	0.766"
" 54	POND DESIGN"				
"	0.764	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1878.4	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.175	c.m/sec"	
"		Maximum level	411.616	metre"	
"		Maximum storage	1094.876	c.m"	
"		Centroidal lag	3.520	hours"	
"		0.274	0.764	0.175	0.766 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.274	0.175	0.175	0.766"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	78.000	Pervious SCS Curve No. "			

"	0.481	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.924	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.086	0.175	0.175	0.766 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	10.695	1.467	9.072 minutes"
"		Time to Centroid	106.283	85.675	102.658 minutes"
"		Rainfall depth	86.737	86.737	86.737 mm"
"		Rainfall volume	374.70	41.63	416.34 c.m"
"		Rainfall losses	44.994	6.561	41.150 mm"
"		Runoff depth	41.743	80.176	45.586 mm"
"		Runoff volume	180.33	38.48	218.81 c.m"
"		Runoff coefficient	0.481	0.924	0.526 "
"		Maximum flow	0.074	0.018	0.086 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.086	0.242	0.175	0.766"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.086	0.242	0.242	0.766"
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.242		c.m/sec"
"		Hydrograph volume	2097.427		c.m"
"		0.086	0.242	0.242	0.242"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.086	0.000	0.242	0.242"
" 33		CATCHMENT 2200"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2200	Catchment 2200"			
"	75.000	% Impervious"			
"	0.920	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.230	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.690	Impervious Area"			
"	40.000	Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.481	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.922	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.275	0.000	0.242	0.242 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	16.211	2.224	4.298	minutes"
"		Time to Centroid	112.570	86.667	90.508	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	199.49	598.48	797.98	c.m"
"		Rainfall losses	44.974	6.774	16.324	mm"
"		Runoff depth	41.763	79.963	70.413	mm"
"		Runoff volume	96.05	551.75	647.80	c.m"
"		Runoff coefficient	0.481	0.922	0.812	"
"		Maximum flow	0.035	0.262	0.275	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.275	0.275	0.242	0.242"	
" 54		POND DESIGN"				
"	0.275	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	647.8	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.061	c.m/sec"		
"		Maximum level	414.119	metre"		

"	Maximum storage	447.957	c.m"
"	Centroidal lag	6.667	hours"
"	0.275 0.275 0.061 0.242	c.m/sec"	
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.267	c.m/sec"
"	Hydrograph volume	2744.608	c.m"
"	0.275 0.275 0.061	0.267"	
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.275 0.000 0.061	0.267"	
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.482 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.917 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.102 0.000 0.061	0.267 c.m/sec"	
"	Catchment 3100	Pervious Impervious Total Area "	
"	Surface Area	0.168 0.252 0.420	hectare"
"	Time of concentration	19.958 2.738 7.208	minutes"
"	Time to Centroid	116.779 87.402 95.027	minutes"
"	Rainfall depth	86.737 86.737 86.737	mm"
"	Rainfall volume	145.72 218.58 364.29	c.m"
"	Rainfall losses	44.916 7.204 22.289	mm"
"	Runoff depth	41.821 79.533 64.448	mm"
"	Runoff volume	70.26 200.42 270.68	c.m"
"	Runoff coefficient	0.482 0.917 0.743	"
"	Maximum flow	0.023 0.095 0.102	c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.102      0.102      0.061      0.267"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"              0  Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.035      c.m/sec"
"              Volume of diverted flow      24.243      c.m"
"              DIV32001.050hyd"
"              Major flow at 32001"
"                  0.102      0.102      0.067      0.267 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.102      0.067      0.067      0.267"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          246.4  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"              410.620      0.000      0.000"
"              410.870      0.01300      4.855"
"              411.120      0.02000      14.351"
"              411.370      0.02500      24.074"
"              411.620      0.02900      33.921"
"              411.870      0.03300      43.768"
"              412.120      0.03600      53.614"
"              412.370      0.03900      63.461"
"              412.620      0.04200      73.308"
"              412.870      0.04400      74.155"
"              413.120      0.04700      75.003"
"              413.370      0.04900      75.850"
"              413.620      0.05200      76.698"
"              413.980      0.05500      77.918"
"              414.230      0.1600      78.483"
"              Peak outflow      0.041      c.m/sec"
"              Maximum level      412.539      metre"
"              Maximum storage      70.123      c.m"
"              Centroidal lag      1.956      hours"
"                  0.102      0.067      0.041      0.267 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.102      0.041      0.041      0.267"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3200	Catchment 3200"				
"	60.000	% Impervious"				
"	0.130	Total Area"				
"	20.000	Flow length"				
"	1.000	Overland Slope"				
"	0.052	Pervious Area"				
"	20.000	Pervious length"				
"	1.000	Pervious slope"				
"	0.078	Impervious Area"				
"	20.000	Impervious length"				
"	1.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	78.000	Pervious SCS Curve No."				
"	0.481	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n' "				
"	98.000	Impervious SCS Curve No."				
"	0.925	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.034	0.041	0.041	0.267 c.m/sec"	
"		Catchment 3200	Pervious	Impervious	Total Area	"
"		Surface Area	0.052	0.078	0.130	hectare"
"		Time of concentration	13.168	1.807	4.730	minutes"
"		Time to Centroid	109.105	86.056	91.988	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	45.10	67.65	112.76	c.m"
"		Rainfall losses	45.027	6.490	21.905	mm"
"		Runoff depth	41.710	80.247	64.832	mm"
"		Runoff volume	21.69	62.59	84.28	c.m"
"		Runoff coefficient	0.481	0.925	0.747	"
"		Maximum flow	0.008	0.029	0.034	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.034	0.064	0.041	0.267"	
" 33		CATCHMENT 3300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3300	Catchment 3300"				
"	60.000	% Impervious"				
"	0.240	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.096	Pervious Area"				
"	20.000	Pervious length"				

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.481 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.924 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.064      0.064      0.041      0.267 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 10.695      1.467      3.845      minutes"
"      Time to Centroid 106.283      85.675      90.985      minutes"
"      Rainfall depth 86.737      86.737      86.737      mm"
"      Rainfall volume 83.27      124.90      208.17      c.m"
"      Rainfall losses 44.994      6.561      21.934      mm"
"      Runoff depth 41.743      80.176      64.803      mm"
"      Runoff volume 40.07      115.45      155.53      c.m"
"      Runoff coefficient 0.481      0.924      0.747      "
"      Maximum flow 0.016      0.054      0.064      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.064      0.128      0.041      0.267"
" 47      FILEI_0 Read/Open DIV32001.050hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.050hyd"
"      Major flow at 32001"
"      Total volume      24.243      c.m"
"      Maximum flow      0.035      c.m/sec"
"          0.035      0.128      0.041      0.267 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.035      0.163      0.041      0.267"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.035      0.163      0.163      0.267"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.163      c.m/sec"
"      Hydrograph volume 510.569      c.m"

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"		0.035	0.163	0.163	0.163"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.163		c.m/sec"	
"		Hydrograph volume	510.569		c.m"	
"		0.035	0.163	0.163	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.035	0.163	0.163	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.817		c.m/sec"	
"		Hydrograph volume	6510.743		c.m"	
"		0.035	0.163	0.163	0.817"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.817		c.m/sec"	
"		Hydrograph volume	6510.742		c.m"	
"		0.035	0.817	0.163	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.035	0.817	0.817	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.063		c.m/sec"	
"		Hydrograph volume	9255.348		c.m"	
"		0.035	0.817	0.817	1.063"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.063		c.m/sec"	
"		Hydrograph volume	9255.348		c.m"	
"		0.035	1.063	0.817	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__100yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:49:23 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.019 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          168.777  mm/hr"
"          Total depth                97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.511  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.908  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"	0.063	0.000	0.000	0.000 c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area "
"	Surface Area	0.110	0.110	0.220 hectare"
"	Time of concentration	6.616	0.937	2.983 minutes"
"	Time to Centroid	101.199	85.009	90.841 minutes"
"	Rainfall depth	97.921	97.921	97.921 mm"
"	Rainfall volume	107.71	107.71	215.43 c.m"
"	Rainfall losses	47.838	8.977	28.408 mm"
"	Runoff depth	50.084	88.944	69.514 mm"
"	Runoff volume	55.09	97.84	152.93 c.m"
"	Runoff coefficient	0.511	0.908	"
"	Maximum flow	0.026	0.045	0.063 c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

"	0.063	0.063	0.000	0.000"
---	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n' "
"	78.000	Pervious SCS Curve No. "
"	0.515	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n' "
"	98.000	Impervious SCS Curve No. "
"	0.931	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

"	0.194	0.063	0.000	0.000 c.m/sec"
---	-------	-------	-------	----------------

"	Catchment 1300	Pervious	Impervious	Total Area "
"	Surface Area	0.350	0.350	0.700 hectare"
"	Time of concentration	10.027	1.421	4.487 minutes"
"	Time to Centroid	104.871	85.423	92.352 minutes"
"	Rainfall depth	97.921	97.921	97.921 mm"
"	Rainfall volume	342.72	342.72	685.45 c.m"
"	Rainfall losses	47.483	6.787	27.135 mm"
"	Runoff depth	50.438	91.134	70.786 mm"
"	Runoff volume	176.53	318.97	495.50 c.m"

"	Runoff coefficient	0.515	0.931	0.723	"
"	Maximum flow	0.074	0.144	0.194	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.194	0.257	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.516 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.924 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.062	0.257	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	8.438	1.196	3.791	minutes"
"	Time to Centroid	103.033	85.162	91.567	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	107.71	107.71	215.43	c.m"
"	Rainfall losses	47.375	7.430	27.402	mm"
"	Runoff depth	50.546	90.491	70.519	mm"
"	Runoff volume	55.60	99.54	155.14	c.m"
"	Runoff coefficient	0.516	0.924	0.720	"
"	Maximum flow	0.025	0.045	0.062	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.062	0.319	0.000	0.000"	
" 54	POND DESIGN"				
"	0.319 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	803.6 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

"      412.000  Minimum water level    metre"
"      414.490  Maximum water level    metre"
"      412.000  Starting water level    metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"      412.000  0.00031    0.000"
"      412.100  0.00031    34.650"
"      412.200  0.00031    69.310"
"      412.300  0.00032   103.960"
"      412.400  0.00032   138.610"
"      412.500  0.00032   156.430"
"      412.600  0.00032   174.250"
"      412.700  0.00032   192.060"
"      412.900  0.00032   192.430"
"      413.230  0.00032   193.040"
"      413.430  0.04528   193.730"
"      413.630  0.1319    195.230"
"      413.830  0.2482    197.730"
"      414.030  0.3899    201.230"
"      414.090  0.4369    202.580"
"      414.290  0.6524    208.080"
"      414.490  0.9278    214.580"
"          Peak outflow          0.316    c.m/sec"
"          Maximum level          413.928    metre"
"          Maximum storage          199.438    c.m"
"          Centroidal lag          21.816    hours"
"          0.062    0.319    0.316    0.000 c.m/sec"
" 40      HYDROGRAPH    Combine    1000"
"          6    Combine "
"      1000    Node #"
"          Infiltrated On-Site"
"          Maximum flow          0.316    c.m/sec"
"          Hydrograph volume          794.278    c.m"
"          0.062    0.319    0.316    0.316"
" 40      HYDROGRAPH Start - New Tributary"
"          2    Start - New Tributary"
"          0.062    0.000    0.316    0.316"
" 33      CATCHMENT 1400"
"          1    Triangular SCS"
"          1    Equal length"
"          1    SCS method"
"      1400    Catchment 1400"
"      20.000    % Impervious"
"          0.620    Total Area"
"      30.000    Flow length"
"          2.000    Overland Slope"
"          0.496    Pervious Area"
"      30.000    Pervious length"
"          2.000    Pervious slope"
"          0.124    Impervious Area"

```

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.516  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.932  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.129      0.000      0.316      0.316 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 12.789      1.812      9.373      minutes"
"      Time to Centroid      107.990      85.865      101.105      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      485.69      121.42      607.11      c.m"
"      Rainfall losses      47.421      6.662      39.270      mm"
"      Runoff depth      50.500      91.259      58.652      mm"
"      Runoff volume      250.48      113.16      363.64      c.m"
"      Runoff coefficient      0.516      0.932      0.599      "
"      Maximum flow      0.098      0.051      0.129      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.129      0.129      0.316      0.316"
" 54      POND DESIGN"
"      0.129      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      363.6      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

```

"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.090	c.m/sec"	
"		Maximum level	415.137	metre"	
"		Maximum storage	193.705	c.m"	
"		Centroidal lag	18.767	hours"	
"		0.129	0.129	0.090	0.316 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.317	c.m/sec"	
"		Hydrograph volume	1164.892	c.m"	
"		0.129	0.129	0.090	0.317"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.129	0.000	0.090	0.317"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.517	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.929	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.277	0.000	0.090	0.317 c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area "
"		Surface Area	0.555	0.555	1.110 hectare"
"		Time of concentration	15.199	2.153	6.817 minutes"
"		Time to Centroid	110.688	86.345	95.048 minutes"
"		Rainfall depth	97.921	97.921	97.921 mm"

"	Rainfall volume	543.46	543.46	1086.93	c.m"
"	Rainfall losses	47.301	6.948	27.124	mm"
"	Runoff depth	50.621	90.973	70.797	mm"
"	Runoff volume	280.94	504.90	785.85	c.m"
"	Runoff coefficient	0.517	0.929	0.723	"
"	Maximum flow	0.105	0.230	0.277	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.277	0.277	0.090	0.317"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.131			c.m/sec"
"	Volume of diverted flow	145.528			c.m"
"	DIV01500.100hyd"				
"	Major flow at 1500"				
"		0.277	0.277	0.146	0.317 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.277	0.146	0.146	0.317"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.517 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.926 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		1.610	0.146	0.146	0.317 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	26.337	3.732	11.833	minutes"
"	Time to Centroid	123.197	88.516	100.945	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	3417.45	3417.45	6834.91	c.m"
"	Rainfall losses	47.249	7.203	27.226	mm"
"	Runoff depth	50.672	90.719	70.695	mm"
"	Runoff volume	1768.46	3166.08	4934.54	c.m"
"	Runoff coefficient	0.517	0.926	0.722	"
"	Maximum flow	0.497	1.489	1.610	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		1.610	1.756	0.146	0.317"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.515 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.101	1.756	0.146	0.317 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	10.027	1.421	10.027	minutes"
"	Time to Centroid	104.871	85.423	104.871	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	470.02	0.00	470.02	c.m"
"	Rainfall losses	47.483	6.787	47.483	mm"
"	Runoff depth	50.438	91.134	50.438	mm"
"	Runoff volume	242.10	0.00	242.10	c.m"
"	Runoff coefficient	0.515	0.000	0.515	"
"	Maximum flow	0.101	0.000	0.101	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

```

"          4  Add Runoff "
"              0.101      1.830      0.146      0.317"
" 54      POND DESIGN"
"          1.830  Current peak flow      c.m/sec"
"          0.250  Target outflow      c.m/sec"
"          5817.0  Hydrograph volume      c.m"
"          12.    Number of stages"
"          411.000  Minimum water level      metre"
"          412.000  Maximum water level      metre"
"          411.000  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow              0.970      c.m/sec"
"          Maximum level              411.923      metre"
"          Maximum storage              2593.056      c.m"
"          Centroidal lag              2.996      hours"
"              0.101      1.830      0.970      0.317 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.101      0.970      0.970      0.317"
" 47      FILEI_0 Read/Open DIV01500.100hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.100hyd"
"          Major flow at 1500"
"          Total volume              145.528      c.m"
"          Maximum flow              0.131      c.m/sec"
"              0.131      0.970      0.970      0.317 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.131      0.970      0.970      0.317"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

```

"	7.330	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.330	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	50.000	Pervious SCS Curve No. "			
"	0.164	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	25.400	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No. "			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.268	0.970	0.970	0.317 c.m/sec"
"		Catchment 4000	Pervious	Impervious	Total Area "
"		Surface Area	7.330	0.000	7.330 hectare"
"		Time of concentration	32.980	2.747	32.980 minutes"
"		Time to Centroid	137.344	87.189	137.344 minutes"
"		Rainfall depth	97.921	97.921	97.921 mm"
"		Rainfall volume	7177.62	0.01	7177.63 c.m"
"		Rainfall losses	81.821	7.496	81.821 mm"
"		Runoff depth	16.100	90.426	16.100 mm"
"		Runoff volume	1180.13	0.01	1180.14 c.m"
"		Runoff coefficient	0.164	0.000	0.164 "
"		Maximum flow	0.268	0.000	0.268 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.268	1.235	0.970	0.317"
" 54		POND DESIGN"			
"	1.235	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	7146.6	Hydrograph volume	c.m"		
"	6.	Number of stages"			
"	409.630	Minimum water level	metre"		
"	410.750	Maximum water level	metre"		
"	409.630	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	409.630	0.000	0.000"		
"	409.750	0.6650	402.200"		
"	410.000	3.601	2187.900"		
"	410.250	7.811	5318.900"		
"	410.500	12.984	9642.300"		
"	410.750	18.965	15227.70"		

```

"          Peak outflow          1.083    c.m/sec"
"          Maximum level         409.786    metre"
"          Maximum storage        656.386    c.m"
"          Centroidal lag         3.013    hours"
"          0.268    1.235    1.083    0.317 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.268    1.083    1.083    0.317"
" 52    CHANNEL DESIGN"
"          1.083    Current peak flow    c.m/sec"
"          0.035    Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          0.000    Basewidth    metre"
"          7.410    Left bank slope"
"          6.000    Right bank slope"
"          0.950    Channel depth    metre"
"          1.040    Gradient    %"
"          Depth of flow          0.403    metre"
"          Velocity              0.994    m/sec"
"          Channel capacity       10.655    c.m/sec"
"          Critical depth         0.351    metre"
" 53    ROUTE    Channel Route 72"
"          72.40    Channel Route 72 Reach length    ( metre)"
"          0.400    X-factor <= 0.5"
"          54.618    K-lag    ( seconds)"
"          0.000    Default(0) or user spec.(1) values used"
"          0.500    X-factor <= 0.5"
"          30.000    K-lag    ( seconds)"
"          0.500    Beta weighting factor"
"          60.000    Routing time step    ( seconds)"
"          1    No. of sub-reaches"
"          Peak outflow          1.078    c.m/sec"
"          0.268    1.083    1.078    0.317 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.268    1.078    1.078    0.317"
" 52    CHANNEL DESIGN"
"          1.078    Current peak flow    c.m/sec"
"          0.035    Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          2.000    Basewidth    metre"
"          2.950    Left bank slope"
"          3.000    Right bank slope"
"          0.950    Channel depth    metre"
"          1.040    Gradient    %"
"          Depth of flow          0.325    metre"
"          Velocity              1.120    m/sec"
"          Channel capacity       9.246    c.m/sec"
"          Critical depth         0.269    metre"
" 53    ROUTE    Channel Route 40"

```

"	39.80	Channel Route 40 Reach length	(metre)"		
"	0.291	X-factor <= 0.5"			
"	26.647	K-lag (seconds)"			
"	0.000	Default(0) or user spec.(1) values used"			
"	0.500	X-factor <= 0.5"			
"	30.000	K-lag (seconds)"			
"	0.500	Beta weighting factor"			
"	37.500	Routing time step (seconds)"			
"	1	No. of sub-reaches"			
"		Peak outflow	1.074	c.m/sec"	
"		0.268 1.078 1.074		0.317 c.m/sec"	
" 40		HYDROGRAPH Combine 100"			
"	6	Combine "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	1.074	c.m/sec"	
"		Hydrograph volume	7146.616	c.m"	
"		0.268 1.078 1.074		1.074"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.268 0.000 1.074		1.074"	
" 33		CATCHMENT 2100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2100	Catchment 2100"			
"	60.000	% Impervious"			
"	1.960	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.784	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	1.176	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.517	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.929	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.554 0.000 1.074		1.074 c.m/sec"	
"		Catchment 2100 Pervious Impervious Total Area "			
"		Surface Area 0.784 1.176 1.960		hectare"	
"		Time of concentration 15.199 2.153 5.683		minutes"	

"	Time to Centroid	110.688	86.345	92.932	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	767.70	1151.55	1919.26	c.m"
"	Rainfall losses	47.301	6.948	23.089	mm"
"	Runoff depth	50.621	90.973	74.832	mm"
"	Runoff volume	396.87	1069.84	1466.71	c.m"
"	Runoff coefficient	0.517	0.929	0.764	"
"	Maximum flow	0.148	0.488	0.554	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.554 0.554 1.074 1.074"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n' "				
"	78.000 Pervious SCS Curve No. "				
"	0.515 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n' "				
"	98.000 Impervious SCS Curve No. "				
"	0.931 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.301 0.554 1.074 1.074 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 10.027 1.421 1.919 minutes"				
"	Time to Centroid 104.871 85.423 86.549 minutes"				
"	Rainfall depth 97.921 97.921 97.921 mm"				
"	Rainfall volume 78.34 705.03 783.37 c.m"				
"	Rainfall losses 47.483 6.787 10.857 mm"				
"	Runoff depth 50.438 91.134 87.065 mm"				
"	Runoff volume 40.35 656.17 696.52 c.m"				
"	Runoff coefficient 0.515 0.931 0.889 "				
"	Maximum flow 0.017 0.296 0.301 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.301	0.854	1.074	1.074"
" 54	POND DESIGN"				
"	0.854	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	2163.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.306	c.m/sec"	
"		Maximum level	411.690	metre"	
"		Maximum storage	1201.847	c.m"	
"		Centroidal lag	3.398	hours"	
"		0.301	0.854	0.306	1.074 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.301	0.306	0.306	1.074"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.515	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.931	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.105	0.306	0.306	1.074 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area "
"		Surface Area	0.432	0.048	0.480 hectare"
"		Time of concentration	10.027	1.421	8.588 minutes"
"		Time to Centroid	104.871	85.423	101.619 minutes"
"		Rainfall depth	97.921	97.921	97.921 mm"
"		Rainfall volume	423.02	47.00	470.02 c.m"
"		Rainfall losses	47.483	6.787	43.413 mm"
"		Runoff depth	50.438	91.134	54.508 mm"
"		Runoff volume	217.89	43.74	261.64 c.m"
"		Runoff coefficient	0.515	0.931	0.557 "
"		Maximum flow	0.091	0.020	0.105 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.105	0.355	0.306	1.074"
" 40		HYDROGRAPH Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.105	0.355	0.355	1.074"
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.355		c.m/sec"
"		Hydrograph volume	2419.666		c.m"
"		0.105	0.355	0.355	0.355"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.105	0.000	0.355	0.355"
" 33		CATCHMENT 2200"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2200	Catchment 2200"			
"	75.000	% Impervious"			
"	0.920	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.230	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.690	Impervious Area"			
"	40.000	Impervious length"			

"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.517	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.929	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.306	0.000	0.355	0.355 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	15.199	2.153	4.194	minutes"
"		Time to Centroid	110.688	86.345	90.153	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	225.22	675.66	900.88	c.m"
"		Rainfall losses	47.301	6.948	17.036	mm"
"		Runoff depth	50.621	90.973	80.885	mm"
"		Runoff volume	116.43	627.71	744.14	c.m"
"		Runoff coefficient	0.517	0.929	0.826	"
"		Maximum flow	0.043	0.286	0.306	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.306	0.306	0.355	0.355"	
" 54		POND DESIGN"				
"	0.306	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	744.1	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			
"	414.500	0.3470	1046.900"			
"	414.600	0.3720	1236.100"			
"	414.700	0.3950	1438.700"			
"	415.000	2.828	2087.400"			
"		Peak outflow	0.099	c.m/sec"		
"		Maximum level	414.134	metre"		

"	Maximum storage	469.233	c.m"
"	Centroidal lag	6.040	hours"
"	0.306 0.306 0.099 0.355		c.m/sec"
" 40	HYDROGRAPH Combine 200"		
"	6 Combine "		
"	200 Node #"		
"	To Trib. of Grand River"		
"	Maximum flow	0.454	c.m/sec"
"	Hydrograph volume	3165.424	c.m"
"	0.306 0.306 0.099		0.454"
" 40	HYDROGRAPH Start - New Tributary"		
"	2 Start - New Tributary"		
"	0.306 0.000 0.099		0.454"
" 33	CATCHMENT 3100"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3100 Catchment 3100"		
"	60.000 % Impervious"		
"	0.420 Total Area"		
"	40.000 Flow length"		
"	1.000 Overland Slope"		
"	0.168 Pervious Area"		
"	40.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.252 Impervious Area"		
"	40.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n'"		
"	78.000 Pervious SCS Curve No."		
"	0.517 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n'"		
"	98.000 Impervious SCS Curve No."		
"	0.924 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.114 0.000 0.099		0.454 c.m/sec"
"	Catchment 3100 Pervious Impervious Total Area "		
"	Surface Area 0.168 0.252 0.420		hectare"
"	Time of concentration 18.712 2.651 7.014		minutes"
"	Time to Centroid 114.625 87.045 94.538		minutes"
"	Rainfall depth 97.921 97.921 97.921		mm"
"	Rainfall volume 164.51 246.76 411.27		c.m"
"	Rainfall losses 47.295 7.442 23.384		mm"
"	Runoff depth 50.626 90.479 74.538		mm"
"	Runoff volume 85.05 228.01 313.06		c.m"
"	Runoff coefficient 0.517 0.924 0.761		"
"	Maximum flow 0.029 0.104 0.114		c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.114      0.114      0.099      0.454"
" 56      DIVERSION"
"      32001      Node number"
"      0.067      Overflow threshold"
"      1.000      Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow      0.047      c.m/sec"
"          Volume of diverted flow      39.992      c.m"
"          DIV32001.100hyd"
"          Major flow at 32001"
"          0.114      0.114      0.067      0.454 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"          0.114      0.067      0.067      0.454"
" 54      POND DESIGN"
"      0.067      Current peak flow      c.m/sec"
"      0.756      Target outflow      c.m/sec"
"      273.1      Hydrograph volume      c.m"
"      15.      Number of stages"
"      410.620      Minimum water level      metre"
"      414.230      Maximum water level      metre"
"      410.620      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"          Peak outflow      0.050      c.m/sec"
"          Maximum level      413.465      metre"
"          Maximum storage      76.172      c.m"
"          Centroidal lag      1.972      hours"
"          0.114      0.067      0.050      0.454 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"          0.114      0.050      0.050      0.454"
" 33      CATCHMENT 3200"

```

"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3200	Catchment 3200"			
"	60.000	% Impervious"			
"	0.130	Total Area"			
"	20.000	Flow length"			
"	1.000	Overland Slope"			
"	0.052	Pervious Area"			
"	20.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.078	Impervious Area"			
"	20.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n' "			
"	78.000	Pervious SCS Curve No."			
"	0.516	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n' "			
"	98.000	Impervious SCS Curve No."			
"	0.932	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.038	0.050	0.050	0.454 c.m/sec"
"		Catchment 3200	Pervious	Impervious	Total Area "
"		Surface Area	0.052	0.078	0.130 hectare"
"		Time of concentration	12.345	1.749	4.607 minutes"
"		Time to Centroid	107.474	85.796	91.642 minutes"
"		Rainfall depth	97.921	97.921	97.921 mm"
"		Rainfall volume	50.92	76.38	127.30 c.m"
"		Rainfall losses	47.364	6.648	22.934 mm"
"		Runoff depth	50.557	91.273	74.987 mm"
"		Runoff volume	26.29	71.19	97.48 c.m"
"		Runoff coefficient	0.516	0.932	0.766 "
"		Maximum flow	0.010	0.032	0.038 c.m/sec"
" 40		HYDROGRAPH Add Runoff "			
"	4	Add Runoff "			
"		0.038	0.070	0.050	0.454"
" 33		CATCHMENT 3300"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3300	Catchment 3300"			
"	60.000	% Impervious"			
"	0.240	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.096	Pervious Area"			
"	20.000	Pervious length"			

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.515 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.931 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.072      0.070      0.050      0.454 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 10.027      1.421      3.740      minutes"
"      Time to Centroid 104.871      85.423      90.664      minutes"
"      Rainfall depth 97.921      97.921      97.921      mm"
"      Rainfall volume 94.00      141.01      235.01      c.m"
"      Rainfall losses 47.483      6.787      23.065      mm"
"      Runoff depth 50.438      91.134      74.856      mm"
"      Runoff volume 48.42      131.23      179.65      c.m"
"      Runoff coefficient 0.515      0.931      0.764      "
"      Maximum flow 0.020      0.059      0.072      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.072      0.142      0.050      0.454"
" 47      FILEI_0 Read/Open DIV32001.100hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.100hyd"
"      Major flow at 32001"
"      Total volume      39.992      c.m"
"      Maximum flow      0.047      c.m/sec"
"          0.047      0.142      0.050      0.454 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.047      0.190      0.050      0.454"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.047      0.190      0.190      0.454"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.190      c.m/sec"
"      Hydrograph volume 590.014      c.m"

```

"		0.047	0.190	0.190	0.190"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.190		c.m/sec"	
"		Hydrograph volume	590.014		c.m"	
"		0.047	0.190	0.190	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.047	0.190	0.190	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	1.138		c.m/sec"	
"		Hydrograph volume	7736.632		c.m"	
"		0.047	0.190	0.190	1.138"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	1.138		c.m/sec"	
"		Hydrograph volume	7736.631		c.m"	
"		0.047	1.138	0.190	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.047	1.138	1.138	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.475		c.m/sec"	
"		Hydrograph volume	10902.038		c.m"	
"		0.047	1.138	1.138	1.475"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.475		c.m/sec"	
"		Hydrograph volume	10902.038		c.m"	
"		0.047	1.475	1.138	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                              Modelling Files\2022-07-25"
"          Output filename:                    Post__REG.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 12:28:09 PM"
" 31      TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Historic"
"          5  Historic"
"          2880.000 Duration"
"          48.000 Rainfall intensity values"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.028      2.028      2.028      2.028"
"                  2.028      2.026      2.026      2.026      2.028"
"                  2.026      6.000      4.000      6.000      13.000"
"                  17.000      13.000      23.000      13.000      13.000"
"                  53.000      38.000      13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
"          6  000hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200 Catchment 1200"
"          50.000 % Impervious"
"          0.220 Total Area"
"          10.000 Flow length"
"          2.000 Overland Slope"
"          0.110 Pervious Area"
"          10.000 Pervious length"
"          2.000 Pervious slope"
"          0.110 Impervious Area"
"          10.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.719 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"

```

"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.027	0.000	0.000	0.000 c.m/sec"	
"		Catchment 1200	Pervious	Impervious	Total Area	"
"		Surface Area	0.110	0.110	0.220	hectare"
"		Time of concentration	8.237	1.480	4.583	minutes"
"		Time to Centroid	2489.666	2307.003	2390.883	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	313.50	313.50	627.00	c.m"
"		Rainfall losses	80.153	43.756	61.955	mm"
"		Runoff depth	204.847	241.244	223.045	mm"
"		Runoff volume	225.33	265.37	490.70	c.m"
"		Runoff coefficient	0.719	0.846	0.783	"
"		Maximum flow	0.013	0.014	0.027	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.027	0.027	0.000	0.000"	
" 33		CATCHMENT 1300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1300	Catchment 1300"				
"	50.000	% Impervious"				
"	0.700	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.350	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.350	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.085	0.027	0.000	0.000 c.m/sec"	
"		Catchment 1300	Pervious	Impervious	Total Area	"
"		Surface Area	0.350	0.350	0.700	hectare"
"		Time of concentration	12.485	2.243	7.071	minutes"

"	Time to Centroid	2505.277	2290.972	2391.991	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	997.50	997.50	1995.00	c.m"
"	Rainfall losses	70.073	43.972	57.023	mm"
"	Runoff depth	214.927	241.028	227.977	mm"
"	Runoff volume	752.24	843.60	1595.84	c.m"
"	Runoff coefficient	0.754	0.846	0.800	"
"	Maximum flow	0.040	0.045	0.085	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.085	0.112	0.000	0.000"
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.739 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.844 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.027	0.112	0.000	0.000 c.m/sec"
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	10.506	1.887	5.911	minutes"
"	Time to Centroid	2500.832	2301.045	2394.331	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	313.50	313.50	627.00	c.m"
"	Rainfall losses	74.380	44.547	59.463	mm"
"	Runoff depth	210.620	240.453	225.537	mm"
"	Runoff volume	231.68	264.50	496.18	c.m"
"	Runoff coefficient	0.739	0.844	0.791	"
"	Maximum flow	0.013	0.014	0.027	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.027	0.139	0.000	0.000"
" 54	POND DESIGN"				
"	0.139	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	2582.7	Hydrograph volume	c.m"		
"	17.	Number of stages"			
"	412.000	Minimum water level	metre"		
"	414.490	Maximum water level	metre"		
"	412.000	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	412.000	0.00031	0.000"		
"	412.100	0.00031	34.650"		
"	412.200	0.00031	69.310"		
"	412.300	0.00032	103.960"		
"	412.400	0.00032	138.610"		
"	412.500	0.00032	156.430"		
"	412.600	0.00032	174.250"		
"	412.700	0.00032	192.060"		
"	412.900	0.00032	192.430"		
"	413.230	0.00032	193.040"		
"	413.430	0.04528	193.730"		
"	413.630	0.1319	195.230"		
"	413.830	0.2482	197.730"		
"	414.030	0.3899	201.230"		
"	414.090	0.4369	202.580"		
"	414.290	0.6524	208.080"		
"	414.490	0.9278	214.580"		
"		Peak outflow	0.138	c.m/sec"	
"		Maximum level	413.642	metre"	
"		Maximum storage	195.375	c.m"	
"		Centroidal lag	49.356	hours"	
"		0.027	0.139	0.138	0.000 c.m/sec"
" 40	HYDROGRAPH	Combine	1000"		
"	6	Combine "			
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.138	c.m/sec"	
"		Hydrograph volume	2556.484	c.m"	
"		0.027	0.139	0.138	0.138"
" 40	HYDROGRAPH	Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.027	0.000	0.138	0.138"
" 33	CATCHMENT 1400"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1400	Catchment 1400"			
"	20.000	% Impervious"			
"	0.620	Total Area"			

"	30.000	Flow length"				
"	2.000	Overland Slope"				
"	0.496	Pervious Area"				
"	30.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.124	Impervious Area"				
"	30.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.764	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.849	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.071	0.000	0.138	0.138 c.m/sec"	
"		Catchment 1400	Pervious	Impervious	Total Area	"
"		Surface Area	0.496	0.124	0.620	hectare"
"		Time of concentration	15.924	2.860	13.084	minutes"
"		Time to Centroid	2515.051	2276.456	2463.191	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1413.60	353.40	1767.00	c.m"
"		Rainfall losses	67.258	43.116	62.430	mm"
"		Runoff depth	217.742	241.884	222.570	mm"
"		Runoff volume	1080.00	299.94	1379.93	c.m"
"		Runoff coefficient	0.764	0.849	0.781	"
"		Maximum flow	0.055	0.016	0.071	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.071	0.071	0.138	0.138"	
" 54		POND DESIGN"				
"	0.071	Current peak flow	c.m/sec"			
"	0.250	Target outflow	c.m/sec"			
"	1379.9	Hydrograph volume	c.m"			
"	17.	Number of stages"				
"	413.920	Minimum water level	metre"			
"	415.520	Maximum water level	metre"			
"	413.920	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.920	0.00088	0.000"			
"	414.020	0.00088	26.350"			
"	414.120	0.00089	55.150"			
"	414.220	0.00089	83.950"			
"	414.320	0.00089	112.750"			
"	414.420	0.00090	139.100"			
"	414.520	0.00090	165.450"			

"		414.620	0.00090	191.800"	
"		414.720	0.00090	191.870"	
"		414.820	0.02640	191.950"	
"		414.920	0.03734	192.020"	
"		415.020	0.04573	192.090"	
"		415.120	0.05281	192.160"	
"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.070	c.m/sec"	
"		Maximum level	415.128	metre"	
"		Maximum storage	192.887	c.m"	
"		Centroidal lag	48.418	hours"	
"		0.071	0.071	0.070	0.138 c.m/sec"
" 40		HYDROGRAPH	Combine	1000"	
"	6	Combine	"		
"	1000	Node #"			
"		Infiltrated On-Site"			
"		Maximum flow	0.208	c.m/sec"	
"		Hydrograph volume	3933.528	c.m"	
"		0.071	0.071	0.070	0.208"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.071	0.000	0.070	0.208"
" 33		CATCHMENT 1500"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1500	Catchment 1500"			
"	50.000	% Impervious"			
"	1.110	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.555	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.555	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.765	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.850	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			

	0.129	0.000	0.070	0.208	c.m/sec"
"	Catchment 1500	Pervious	Impervious	Total Area	"
"	Surface Area	0.555	0.555	1.110	hectare"
"	Time of concentration	18.924	3.399	10.752	minutes"
"	Time to Centroid	2520.774	2266.333	2386.847	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	1581.75	1581.75	3163.50	c.m"
"	Rainfall losses	66.918	42.646	54.782	mm"
"	Runoff depth	218.082	242.354	230.218	mm"
"	Runoff volume	1210.36	1345.07	2555.42	c.m"
"	Runoff coefficient	0.765	0.850	0.808	"
"	Maximum flow	0.060	0.070	0.129	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.129	0.129	0.070	0.208"	
" 56	DIVERSION"				
"	1500	Node number"			
"	0.146	Overflow threshold"			
"	1.000	Required diverted fraction"			
"	0	Conduit type; 1=Pipe;2=Channel"			
"	Peak of diverted flow	0.000	c.m/sec"		
"	Volume of diverted flow	0.000	c.m"		
"	DIV01500.000hyd"				
"	Major flow at 1500"				
"	0.129	0.129	0.129	0.208	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.129	0.129	0.129	0.208"	
" 33	CATCHMENT 1000"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	1000	Catchment 1000"			
"	50.000	% Impervious"			
"	6.980	Total Area"			
"	100.000	Flow length"			
"	2.000	Overland Slope"			
"	3.490	Pervious Area"			
"	100.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.490	Impervious Area"			
"	100.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.764	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			

"	0.884	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.768	0.129	0.129	0.208 c.m/sec"	
"		Catchment 1000	Pervious	Impervious	Total Area	"
"		Surface Area	3.490	3.490	6.980	hectare"
"		Time of concentration	32.793	5.891	18.359	minutes"
"		Time to Centroid	2537.302	2260.661	2388.880	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	0.9947	0.9947	1.9893	ha-m"
"		Rainfall losses	67.335	33.040	50.188	mm"
"		Runoff depth	217.665	251.960	234.812	mm"
"		Runoff volume	0.7597	0.8793	1.6390	ha-m"
"		Runoff coefficient	0.764	0.884	0.824	"
"		Maximum flow	0.407	0.441	0.768	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.768	0.898	0.129	0.208"	
" 33		CATCHMENT 1100"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1100	Catchment 1100"				
"	0.000	% Impervious"				
"	0.480	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.480	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n' "				
"	78.000	Pervious SCS Curve No. "				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n' "				
"	98.000	Impervious SCS Curve No. "				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.055	0.898	0.129	0.208 c.m/sec"	
"		Catchment 1100	Pervious	Impervious	Total Area	"
"		Surface Area	0.480	0.000	0.480	hectare"
"		Time of concentration	12.485	2.243	12.485	minutes"
"		Time to Centroid	2505.277	2290.972	2505.276	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1368.00	0.00	1368.00	c.m"

"	Rainfall losses	70.073	43.972	70.073	mm"
"	Runoff depth	214.927	241.028	214.927	mm"
"	Runoff volume	1031.65	0.00	1031.65	c.m"
"	Runoff coefficient	0.754	0.000	0.754	"
"	Maximum flow	0.055	0.000	0.055	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.055	0.952	0.129	0.208"
" 54	POND DESIGN"				
"	0.952	Current peak flow	c.m/sec"		
"	0.250	Target outflow	c.m/sec"		
"	19977.0	Hydrograph volume	c.m"		
"	12.	Number of stages"			
"	411.000	Minimum water level	metre"		
"	412.000	Maximum water level	metre"		
"	411.000	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	411.000	0.000	0.000"		
"	411.100	0.03200	18.400"		
"	411.200	0.06500	95.500"		
"	411.300	0.07700	239.900"		
"	411.400	0.08700	435.700"		
"	411.500	0.09600	691.300"		
"	411.600	0.1050	1027.600"		
"	411.700	0.3150	1450.000"		
"	411.800	0.3380	1935.800"		
"	411.850	0.3490	2195.700"		
"	411.900	0.6670	2465.600"		
"	412.000	2.018	3030.100"		
"		Peak outflow	0.875	c.m/sec"	
"		Maximum level	411.919	metre"	
"		Maximum storage	2573.825	c.m"	
"		Centroidal lag	40.971	hours"	
"		0.055	0.952	0.875	0.208 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.055	0.875	0.875	0.208"
" 47	FILEI_0 Read/Open DIV01500.000hyd"				
"	1	1=read/open; 2=write/save"			
"	2	1=rainfall; 2=hydrograph"			
"	1	1=runoff; 2=inflow; 3=outflow; 4=junction"			
"	DIV01500.000hyd"				
"	Major flow at 1500"				
"		Total volume	0.000	c.m"	
"		Maximum flow	0.000	c.m/sec"	
"		0.000	0.875	0.875	0.208 c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.000	0.875	0.875	0.208"

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" 33      CATCHMENT 4000"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      4000   Catchment 4000"
"      0.000   % Impervious"
"      7.330   Total Area"
"      60.000   Flow length"
"      2.000   Overland Slope"
"      7.330   Pervious Area"
"      60.000   Pervious length"
"      2.000   Pervious slope"
"      0.000   Impervious Area"
"      60.000   Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"      50.000   Pervious SCS Curve No."
"      0.453   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      25.400   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"      98.000   Impervious SCS Curve No."
"      0.000   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"          0.619      0.875      0.875      0.208 c.m/sec"
"      Catchment 4000      Pervious      Impervious      Total Area  "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration 27.692      4.336      27.692      minutes"
"      Time to Centroid 2672.198      2258.968      2672.197      minutes"
"      Rainfall depth 285.000      285.000      285.000      mm"
"      Rainfall volume 2.0890      0.0000      2.0891      ha-m"
"      Rainfall losses 155.800      39.404      155.800      mm"
"      Runoff depth 129.200      245.596      129.200      mm"
"      Runoff volume 9470.35      0.02      9470.37      c.m"
"      Runoff coefficient 0.453      0.000      0.453      "
"      Maximum flow 0.619      0.000      0.619      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"          0.619      1.494      0.875      0.208"
" 54      POND DESIGN"
"          1.494   Current peak flow      c.m/sec"
"          0.250   Target outflow      c.m/sec"
"      29575.5   Hydrograph volume      c.m"
"          6.     Number of stages"
"      409.630   Minimum water level      metre"
"      410.750   Maximum water level      metre"
"      409.630   Starting water level      metre"
"          0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"

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"	409.630	0.000	0.000"		
"	409.750	0.6650	402.200"		
"	410.000	3.601	2187.900"		
"	410.250	7.811	5318.900"		
"	410.500	12.984	9642.300"		
"	410.750	18.965	15227.70"		
"	Peak outflow		1.444	c.m/sec"	
"	Maximum level		409.816	metre"	
"	Maximum storage		875.731	c.m"	
"	Centroidal lag		42.281	hours"	
"	0.619	1.494	1.444	0.208	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.619	1.444	1.444	0.208"	
" 52	CHANNEL DESIGN"				
"	1.444	Current peak flow		c.m/sec"	
"	0.035	Manning 'n'"			
"	0.	Cross-section type: 0=trapezoidal; 1=general"			
"	0.000	Basewidth			metre"
"	7.410	Left bank slope"			
"	6.000	Right bank slope"			
"	0.950	Channel depth		metre"	
"	1.040	Gradient			%"
"	Depth of flow		0.449	metre"	
"	Velocity		1.068	m/sec"	
"	Channel capacity		10.655	c.m/sec"	
"	Critical depth		0.394	metre"	
" 53	ROUTE Channel Route 72"				
"	72.40	Channel Route 72 Reach length		(metre)"	
"	0.388	X-factor <= 0.5"			
"	50.828	K-lag (seconds)"			
"	0.000	Default(0) or user spec.(1) values used"			
"	0.500	X-factor <= 0.5"			
"	30.000	K-lag (seconds)"			
"	0.500	Beta weighting factor"			
"	62.069	Routing time step (seconds)"			
"	1	No. of sub-reaches"			
"	Peak outflow		1.438	c.m/sec"	
"	0.619	1.444	1.438	0.208	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"	0.619	1.438	1.438	0.208"	
" 52	CHANNEL DESIGN"				
"	1.438	Current peak flow		c.m/sec"	
"	0.035	Manning 'n'"			
"	0.	Cross-section type: 0=trapezoidal; 1=general"			
"	2.000	Basewidth			metre"
"	2.950	Left bank slope"			
"	3.000	Right bank slope"			
"	0.950	Channel depth		metre"	

"	1.040	Gradient	%"		
"		Depth of flow		0.378	metre"
"		Velocity		1.217	m/sec"
"		Channel capacity		9.246	c.m/sec"
"		Critical depth		0.318	metre"
" 53		ROUTE	Channel Route 40"		
"	39.80	Channel Route 40	Reach length	(metre)"	
"	0.260	X-factor	<= 0.5"		
"	24.517	K-lag	(seconds)"		
"	0.000	Default(0) or user spec.(1)	values used"		
"	0.500	X-factor	<= 0.5"		
"	30.000	K-lag	(seconds)"		
"	0.500	Beta weighting factor"			
"	36.000	Routing time step	(seconds)"		
"	1	No. of sub-reaches"			
"		Peak outflow		1.436	c.m/sec"
"		0.619	1.438	1.436	0.208 c.m/sec"
" 40		HYDROGRAPH	Combine 100"		
"	6	Combine	"		
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow		1.436	c.m/sec"
"		Hydrograph volume		29574.402	c.m"
"		0.619	1.438	1.436	1.436"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.619	0.000	1.436	1.436"
" 33		CATCHMENT	2100"		
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2100	Catchment	2100"		
"	60.000	% Impervious"			
"	1.960	Total Area"			
"	40.000	Flow length"			
"	2.000	Overland Slope"			
"	0.784	Pervious Area"			
"	40.000	Pervious length"			
"	2.000	Pervious slope"			
"	1.176	Impervious Area"			
"	40.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.765	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.850	Impervious Runoff coefficient"			

"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.232	0.000	1.436	1.436 c.m/sec"	
"		Catchment 2100	Pervious	Impervious	Total Area	"
"		Surface Area	0.784	1.176	1.960	hectare"
"		Time of concentration	18.924	3.399	9.220	minutes"
"		Time to Centroid	2520.773	2266.333	2361.739	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	2234.40	3351.60	5586.00	c.m"
"		Rainfall losses	66.918	42.646	52.354	mm"
"		Runoff depth	218.082	242.354	232.646	mm"
"		Runoff volume	1709.77	2850.09	4559.85	c.m"
"		Runoff coefficient	0.765	0.850	0.816	"
"		Maximum flow	0.085	0.148	0.232	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.232	0.232	1.436	1.436"	
" 33		CATCHMENT 2400"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2400	Catchment 2400"				
"	90.000	% Impervious"				
"	0.800	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.080	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.720	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.102	0.232	1.436	1.436 c.m/sec"	
"		Catchment 2400	Pervious	Impervious	Total Area	"
"		Surface Area	0.080	0.720	0.800	hectare"
"		Time of concentration	12.485	2.243	3.166	minutes"
"		Time to Centroid	2505.277	2290.972	2310.291	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	228.00	2052.00	2280.00	c.m"
"		Rainfall losses	70.073	43.972	46.582	mm"

"	Runoff depth	214.927	241.028	238.418	mm"
"	Runoff volume	171.94	1735.40	1907.34	c.m"
"	Runoff coefficient	0.754	0.846	0.837	"
"	Maximum flow	0.009	0.092	0.102	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.102	0.334	1.436	1.436"	
" 54	POND DESIGN"				
"	0.334	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	6467.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"	Level Discharge	Volume"			
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"	Peak outflow	0.268	c.m/sec"		
"	Maximum level	411.678	metre"		
"	Maximum storage	1184.491	c.m"		
"	Centroidal lag	41.101	hours"		
"	0.102	0.334	0.268	1.436 c.m/sec"	
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.102	0.268	0.268	1.436"	
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			

"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.754	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.846	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.055	0.268	0.268	1.436 c.m/sec"	
"	Catchment 2300	Pervious	Impervious	Total Area	"
"	Surface Area	0.432	0.048	0.480	hectare"
"	Time of concentration	12.485	2.243	11.350	minutes"
"	Time to Centroid	2505.277	2290.972	2481.532	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	1231.20	136.80	1368.00	c.m"
"	Rainfall losses	70.073	43.972	67.463	mm"
"	Runoff depth	214.927	241.028	217.537	mm"
"	Runoff volume	928.48	115.69	1044.18	c.m"
"	Runoff coefficient	0.754	0.846	0.763	"
"	Maximum flow	0.049	0.006	0.055	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.055	0.315	0.268	1.436"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.055	0.315	0.315	1.436"	
" 40	HYDROGRAPH Combine 200"				
"	6 Combine "				
"	200 Node #"				
"	To Trib. of Grand River"				
"	Maximum flow	0.315	c.m/sec"		
"	Hydrograph volume	7582.879	c.m"		
"	0.055	0.315	0.315	0.315"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.055	0.000	0.315	0.315"	
" 33	CATCHMENT 2200"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2200 Catchment 2200"				
"	75.000 % Impervious"				
"	0.920 Total Area"				
"	40.000 Flow length"				

"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.765	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.850	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.112	0.000	0.315	0.315 c.m/sec"	
"		Catchment 2200	Pervious	Impervious	Total Area	"
"		Surface Area	0.230	0.690	0.920	hectare"
"		Time of concentration	18.924	3.399	6.981	minutes"
"		Time to Centroid	2520.774	2266.333	2325.042	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	655.50	1966.50	2622.00	c.m"
"		Rainfall losses	66.918	42.646	48.714	mm"
"		Runoff depth	218.082	242.354	236.286	mm"
"		Runoff volume	501.59	1672.25	2173.83	c.m"
"		Runoff coefficient	0.765	0.850	0.829	"
"		Maximum flow	0.025	0.087	0.112	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.112	0.112	0.315	0.315"	
" 54		POND DESIGN"				
"	0.112	Current peak flow	c.m/sec"			
"	0.756	Target outflow	c.m/sec"			
"	2173.8	Hydrograph volume	c.m"			
"	12.	Number of stages"				
"	413.700	Minimum water level	metre"			
"	415.000	Maximum water level	metre"			
"	413.700	Starting water level	metre"			
"	0	Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"	413.700	0.000	0.000"			
"	413.800	0.00500	88.600"			
"	413.900	0.01000	187.200"			
"	414.000	0.01300	298.400"			
"	414.100	0.01500	422.200"			
"	414.200	0.2590	558.900"			
"	414.300	0.2910	708.500"			
"	414.400	0.3210	871.100"			

"		414.500	0.3470	1046.900"	
"		414.600	0.3720	1236.100"	
"		414.700	0.3950	1438.700"	
"		415.000	2.828	2087.400"	
"		Peak outflow	0.099	c.m/sec"	
"		Maximum level	414.137	metre"	
"		Maximum storage	473.254	c.m"	
"		Centroidal lag	42.637	hours"	
"		0.112	0.112	0.099	0.315 c.m/sec"
" 40		HYDROGRAPH	Combine	200"	
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.406	c.m/sec"	
"		Hydrograph volume	9743.426	c.m"	
"		0.112	0.112	0.099	0.406"
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.112	0.000	0.099	0.406"
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.761	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.860	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.049	0.000	0.099	0.406 c.m/sec"
"		Catchment 3100	Pervious	Impervious	Total Area "
"		Surface Area	0.168	0.252	0.420 hectare"
"		Time of concentration	23.298	4.185	11.277 minutes"
"		Time to Centroid	2524.169	2259.596	2357.767 minutes"
"		Rainfall depth	285.000	285.000	285.000 mm"

"	Rainfall volume	478.80	718.20	1197.00	c.m"
"	Rainfall losses	68.199	40.012	51.287	mm"
"	Runoff depth	216.801	244.988	233.713	mm"
"	Runoff volume	364.23	617.37	981.59	c.m"
"	Runoff coefficient	0.761	0.860	0.820	"
"	Maximum flow	0.019	0.032	0.049	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.049	0.049	0.099	0.406"
" 56	DIVERSION"				
"	32001 Node number"				
"	0.067 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000		c.m/sec"	
"	Volume of diverted flow	0.000		c.m"	
"	DIV32001.000hyd"				
"	Major flow at 32001"				
"		0.049	0.049	0.049	0.406 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.049	0.049	0.049	0.406"
" 54	POND DESIGN"				
"	0.049 Current peak flow			c.m/sec"	
"	0.756 Target outflow			c.m/sec"	
"	981.6 Hydrograph volume			c.m"	
"	15. Number of stages"				
"	410.620 Minimum water level			metre"	
"	414.230 Maximum water level			metre"	
"	410.620 Starting water level			metre"	
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge	Volume"			
"	410.620	0.000	0.000"		
"	410.870	0.01300	4.855"		
"	411.120	0.02000	14.351"		
"	411.370	0.02500	24.074"		
"	411.620	0.02900	33.921"		
"	411.870	0.03300	43.768"		
"	412.120	0.03600	53.614"		
"	412.370	0.03900	63.461"		
"	412.620	0.04200	73.308"		
"	412.870	0.04400	74.155"		
"	413.120	0.04700	75.003"		
"	413.370	0.04900	75.850"		
"	413.620	0.05200	76.698"		
"	413.980	0.05500	77.918"		
"	414.230	0.1600	78.483"		
"	Peak outflow	0.040		c.m/sec"	
"	Maximum level	412.466		metre"	
"	Maximum storage	67.240		c.m"	

"	Centroidal lag	39.444	hours"
"	0.049 0.049 0.040 0.406 c.m/sec"		
" 40	HYDROGRAPH Next link "		
"	5 Next link "		
"	0.049 0.040 0.040 0.406"		
" 33	CATCHMENT 3200"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3200 Catchment 3200"		
"	60.000 % Impervious"		
"	0.130 Total Area"		
"	20.000 Flow length"		
"	1.000 Overland Slope"		
"	0.052 Pervious Area"		
"	20.000 Pervious length"		
"	1.000 Pervious slope"		
"	0.078 Impervious Area"		
"	20.000 Impervious length"		
"	1.000 Impervious slope"		
"	0.250 Pervious Manning 'n' "		
"	78.000 Pervious SCS Curve No. "		
"	0.763 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning 'n' "		
"	98.000 Impervious SCS Curve No. "		
"	0.848 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.016 0.040 0.040 0.406 c.m/sec"		
"	Catchment 3200 Pervious Impervious Total Area "		
"	Surface Area 0.052 0.078 0.130 hectare"		
"	Time of concentration 15.371 2.761 7.490 minutes"		
"	Time to Centroid 2513.554 2278.554 2366.679 minutes"		
"	Rainfall depth 285.000 285.000 285.000 mm"		
"	Rainfall volume 148.20 222.30 370.50 c.m"		
"	Rainfall losses 67.436 43.264 52.933 mm"		
"	Runoff depth 217.564 241.736 232.067 mm"		
"	Runoff volume 113.13 188.55 301.69 c.m"		
"	Runoff coefficient 0.763 0.848 0.814 "		
"	Maximum flow 0.006 0.010 0.016 c.m/sec"		
" 40	HYDROGRAPH Add Runoff "		
"	4 Add Runoff "		
"	0.016 0.053 0.040 0.406"		
" 33	CATCHMENT 3300"		
"	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	3300 Catchment 3300"		

```

"      60.000  % Impervious"
"      0.240  Total Area"
"      20.000  Flow length"
"      2.000  Overland Slope"
"      0.096  Pervious Area"
"      20.000  Pervious length"
"      2.000  Pervious slope"
"      0.144  Impervious Area"
"      20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.754  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.846  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.029      0.053      0.040      0.406 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area  "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 12.485      2.243      6.061      minutes"
"      Time to Centroid      2505.277      2290.972      2370.872      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      273.60      410.40      684.00      c.m"
"      Rainfall losses      70.073      43.972      54.413      mm"
"      Runoff depth      214.927      241.028      230.587      mm"
"      Runoff volume      206.33      347.08      553.41      c.m"
"      Runoff coefficient      0.754      0.846      0.809      "
"      Maximum flow      0.011      0.018      0.029      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.029      0.077      0.040      0.406"
" 47      FILEI_0 Read/Open DIV32001.000hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.000hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.077      0.040      0.406 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.077      0.040      0.406"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.077      0.077      0.406"

```

" 40	HYDROGRAPH	Combine	300"		
"	6	Combine "			
"	300	Node #"			
"		To Walser Street"			
"		Maximum flow	0.077	c.m/sec"	
"		Hydrograph volume	1827.049	c.m"	
"		0.000 0.077 0.077	0.077	0.077"	
" 40	HYDROGRAPH	Confluence	300"		
"	7	Confluence "			
"	300	Node #"			
"		To Walser Street"			
"		Maximum flow	0.077	c.m/sec"	
"		Hydrograph volume	1827.049	c.m"	
"		0.000 0.077 0.077	0.077	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.000 0.077 0.077	0.077	0.000"	
" 40	HYDROGRAPH	Combine	100"		
"	6	Combine "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	1.511	c.m/sec"	
"		Hydrograph volume	31401.453	c.m"	
"		0.000 0.077 0.077	0.077	1.511"	
" 40	HYDROGRAPH	Confluence	100"		
"	7	Confluence "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	1.511	c.m/sec"	
"		Hydrograph volume	31401.457	c.m"	
"		0.000 1.511 0.077	0.077	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.000 1.511 1.511	1.511	0.000"	
" 40	HYDROGRAPH	Combine	200"		
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	1.917	c.m/sec"	
"		Hydrograph volume	41144.883	c.m"	
"		0.000 1.511 1.511	1.511	1.917"	
" 40	HYDROGRAPH	Confluence	200"		
"	7	Confluence "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	1.917	c.m/sec"	
"		Hydrograph volume	41144.879	c.m"	
"		0.000 1.917 1.511	1.511	0.000"	
" 38	START/RE-START TOTALS	200"			
"	3	Runoff Totals on EXIT"			

"	Total Catchment area	22.610	hectare"
"	Total Impervious area	7.847	hectare"
"	Total % impervious	34.706"	
" 19	EXIT"		

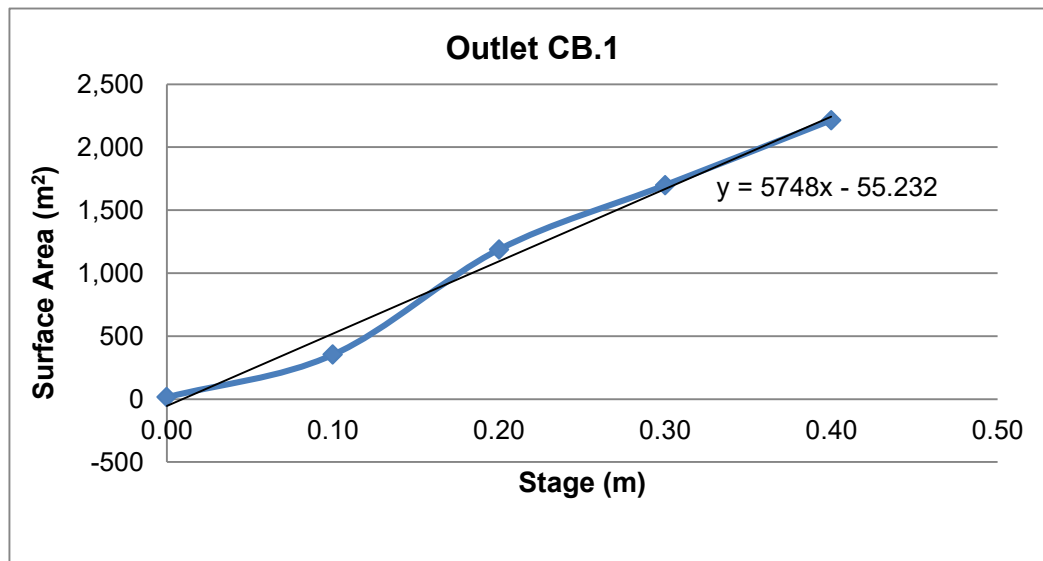
Ainley Farm Subdivision

Our File: 411009

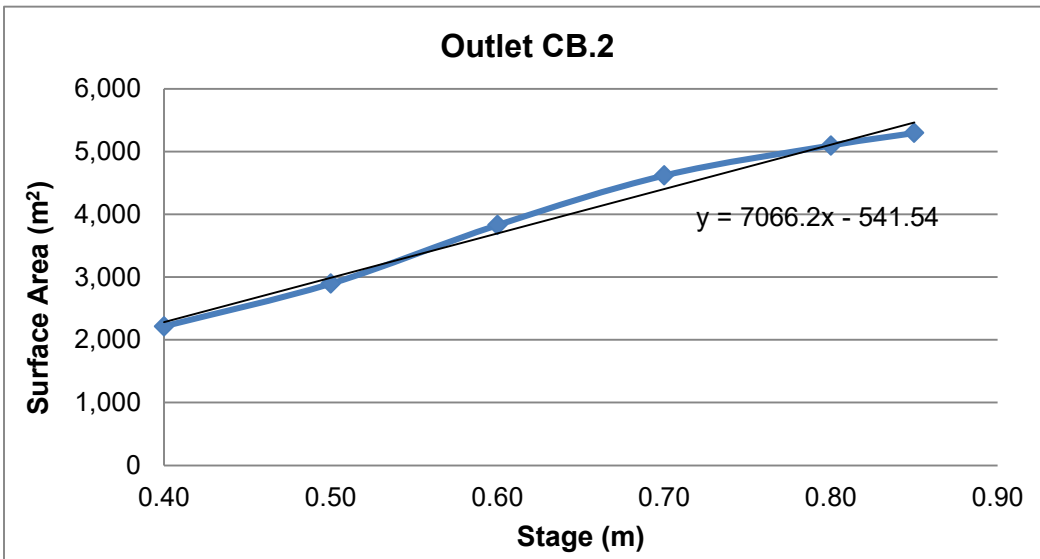
March 14, 2023

Stormwater Management Facility No. 1 - Drawdown Calculations

Elevation	Stage (m)	Surface area (m ²)	
411.00	0.00	15	CB.1 Lip
411.10	0.10	353	
411.20	0.20	1,189	
411.30	0.30	1,699	CB.2 Lip
411.40	0.40	2,216	
411.50	0.50	2,897	
411.60	0.60	3,829	
411.70	0.70	4,620	Weir
411.80	0.80	5,097	
411.85	0.85	5,297	
411.90	0.90	5,498	Top of Bank
412.00	1.00	5792.6	



Ainley Farm Subdivision
Our File: 411009
March 14, 2023



$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o}$$

Eq. 4.11 (MOE, 2003)

Outlet - CB.1
250mm Orifice

Given: d = 0.250
 $A_o = 0.049$
 $C_2 = 5748$
 $C_3 = 55.232$

Outlet - CB.2
350mm Orifice

Given: d = 0.350
 $A_o = 0.096$
 $C_2 = 7066.2$
 $C_3 = 541.54$

Storm	Ponding Depth	CB 1 Outlet h (m)	CB.1 Outlet Drawdown (hr)	CB.2 Outlet h (m)	CB.2 Outlet Drawdown (hr)	Total Drawdown (hr)
2 Year	411.50	0.50	2.9	--	--	2.9
5 Year	411.66	0.60	3.8	0.1	0.4	4.2
10 Year	411.73	0.60	3.8	0.1	0.6	4.4
25 Year	411.85	0.60	3.8	0.3	1.2	5.0
50 Year	411.89	0.60	3.8	0.3	1.2	5.0
100 Year	411.92	0.60	3.8	0.3	1.2	5.0
Regional Storm	411.92	0.60	3.8	0.3	1.2	5.0

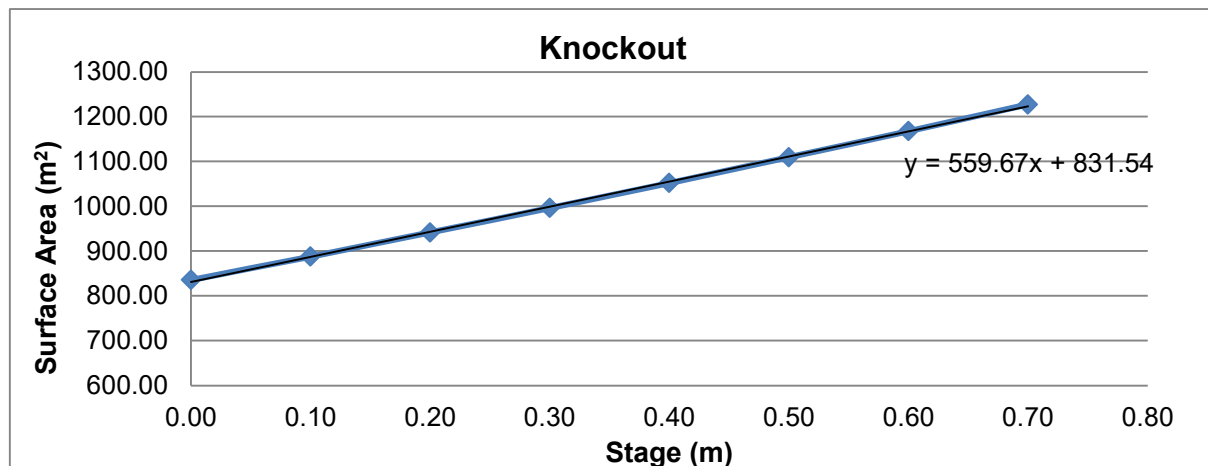
Ainley Farm Subdivision

Our File: 411009

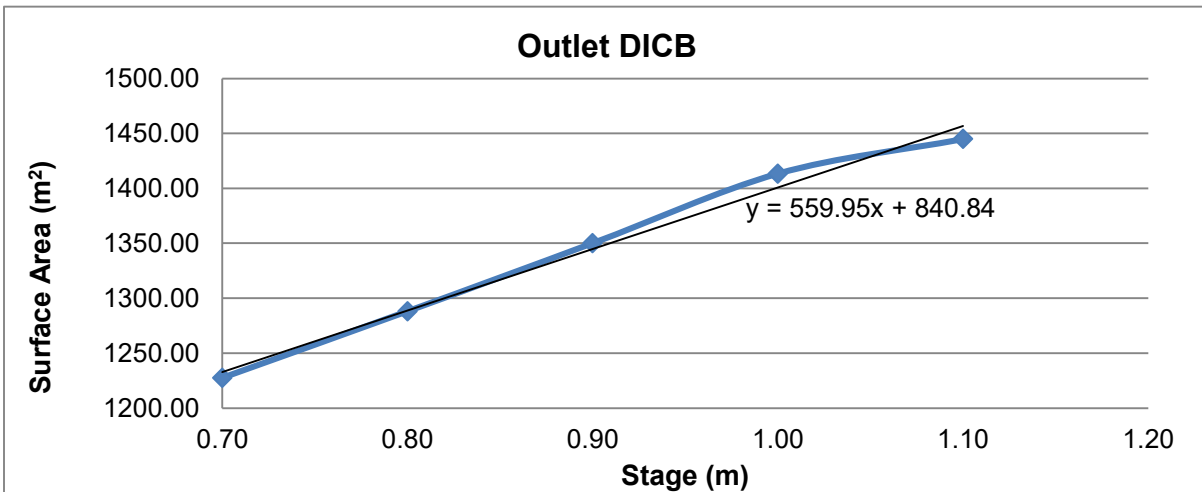
March 14, 2023

Stormwater Management Facility No. 2 Drawdown Calculations

Elevation	Stage (m)	Surface area (m ²)	
410.65	0.00	835.70	Knockout
410.70	0.10	888.00	
410.80	0.20	941.90	
410.90	0.30	996.40	
411.00	0.40	1052.40	
411.10	0.50	1109.60	
411.20	0.60	1167.90	
411.30	0.70	1227.50	CB Lip
411.40	0.80	1288.15	
411.50	0.90	1350.10	
411.60	1.00	1413.30	
411.65	1.10	1444.90	Weir
411.70	1.20	1476.50	
411.80	1.30	1559.40	Top of bank



Ainley Farm Subdivision
Our File: 411009
March 14, 2023



$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o}$$

Eq. 4.11 (MOE, 2003)

**Knockout
150mm Orifice**

Given: d = 0.150
 $A_o = 0.018$
 $C_2 = 559.67$
 $C_3 = 831.54$

**Outlet - DICB
300mm Orifice**

Given: d = 0.300
 $A_o = 0.071$
 $C_2 = 559.95$
 $C_3 = 840.84$

Storm	Ponding Depth	Knockout h (m)	Knockout Drawdown (hr)	Outlet - DICB h (m)	Outlet - DICB Drawdown (hr)	Total Drawdown (hr)
2 Year	411.06	0.41	6.6	--	--	6.6
5 Year	411.19	0.54	7.8	--	--	7.8
10 Year	411.31	0.66	8.9	0.01	0.2	9.1
25 Year	411.48	0.95	11.2	0.18	1.1	12.3
50 Year	411.61	0.95	11.2	0.31	1.4	12.6
100 Year	411.69	0.95	11.2	0.35	1.5	12.8
Regional Storm	411.68	0.95	11.2	0.35	1.5	12.8

Stormceptor®EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

07/15/2022

Province:	Ontario	Project Name:	Ainley Farm Subdivision
City:	Elora	Project Number:	411009
Nearest Rainfall Station:	TORONTO INTL AP	Designer Name:	Patricia Wiebe
Climate Station Id:	6158731	Designer Company:	GM BluePlan Engineering Ltd.
Years of Rainfall Data:	20	Designer Email:	patricia.wiebe@gmblueplan.ca
		Designer Phone:	519-748-1440
Site Name:	SWM Pond 1, Outlet 1 - South	EOR Name:	
		EOR Company:	
Drainage Area (ha):	2.32	EOR Email:	
% Imperviousness:	50.00	EOR Phone:	

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	43.29
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EF4	72
EF6	83
EF8	88
EF10	92
EF12	95

Recommended Stormceptor EF Model: EF6
Estimated Net Annual Sediment (TSS) Load Reduction (%): 83
Water Quality Runoff Volume Capture (%): > 90

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

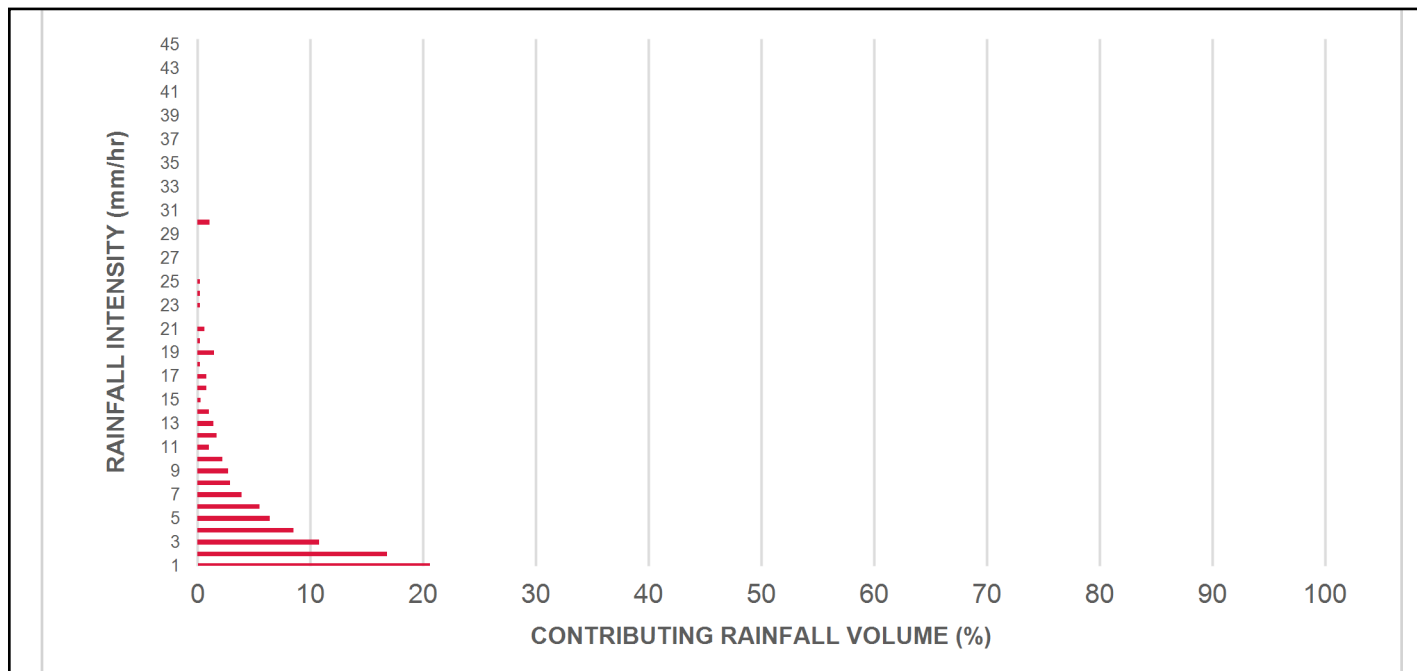
Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	1.93	116.0	44.0	100	8.5	8.5
1	20.6	29.1	3.87	232.0	88.0	98	20.3	28.8
2	16.8	45.9	7.74	464.0	177.0	87	14.6	43.4
3	10.8	56.7	11.61	697.0	265.0	80	8.6	52.1
4	8.5	65.2	15.48	929.0	353.0	76	6.4	58.5
5	6.4	71.6	19.35	1161.0	441.0	73	4.7	63.2
6	5.5	77.0	23.22	1393.0	530.0	72	3.9	67.1
7	3.9	81.0	27.09	1625.0	618.0	71	2.8	69.9
8	2.9	83.9	30.96	1857.0	706.0	70	2.0	71.9
9	2.7	86.5	34.83	2090.0	795.0	69	1.9	73.8
10	2.2	88.7	38.70	2322.0	883.0	69	1.5	75.3
11	1.0	89.7	42.57	2554.0	971.0	68	0.7	75.9
12	1.7	91.3	46.44	2786.0	1059.0	69	1.1	77.1
13	1.4	92.8	50.31	3018.0	1148.0	70	1.0	78.1
14	1.0	93.7	54.18	3251.0	1236.0	72	0.7	78.8
15	0.3	94.0	58.05	3483.0	1324.0	74	0.2	79.0
16	0.8	94.8	61.92	3715.0	1413.0	75	0.6	79.6
17	0.8	95.7	65.79	3947.0	1501.0	70	0.6	80.2
18	0.2	95.8	69.66	4179.0	1589.0	66	0.1	80.3
19	1.5	97.3	73.53	4412.0	1677.0	63	0.9	81.2
20	0.2	97.5	77.40	4644.0	1766.0	60	0.1	81.4
21	0.6	98.2	81.26	4876.0	1854.0	57	0.4	81.7
22	0.0	98.2	85.13	5108.0	1942.0	54	0.0	81.7
23	0.2	98.4	89.00	5340.0	2031.0	52	0.1	81.8
24	0.2	98.6	92.87	5572.0	2119.0	50	0.1	82.0
25	0.2	98.9	96.74	5805.0	2207.0	48	0.1	82.1
30	1.1	100.0	116.09	6966.0	2649.0	41	0.5	82.5
35	0.0	100.0	135.44	8126.0	3090.0	35	0.0	82.5
40	0.0	100.0	154.79	9287.0	3531.0	30	0.0	82.5
45	0.0	100.0	174.14	10448.0	3973.0	27	0.0	82.5
Estimated Net Annual Sediment (TSS) Load Reduction =								83 %

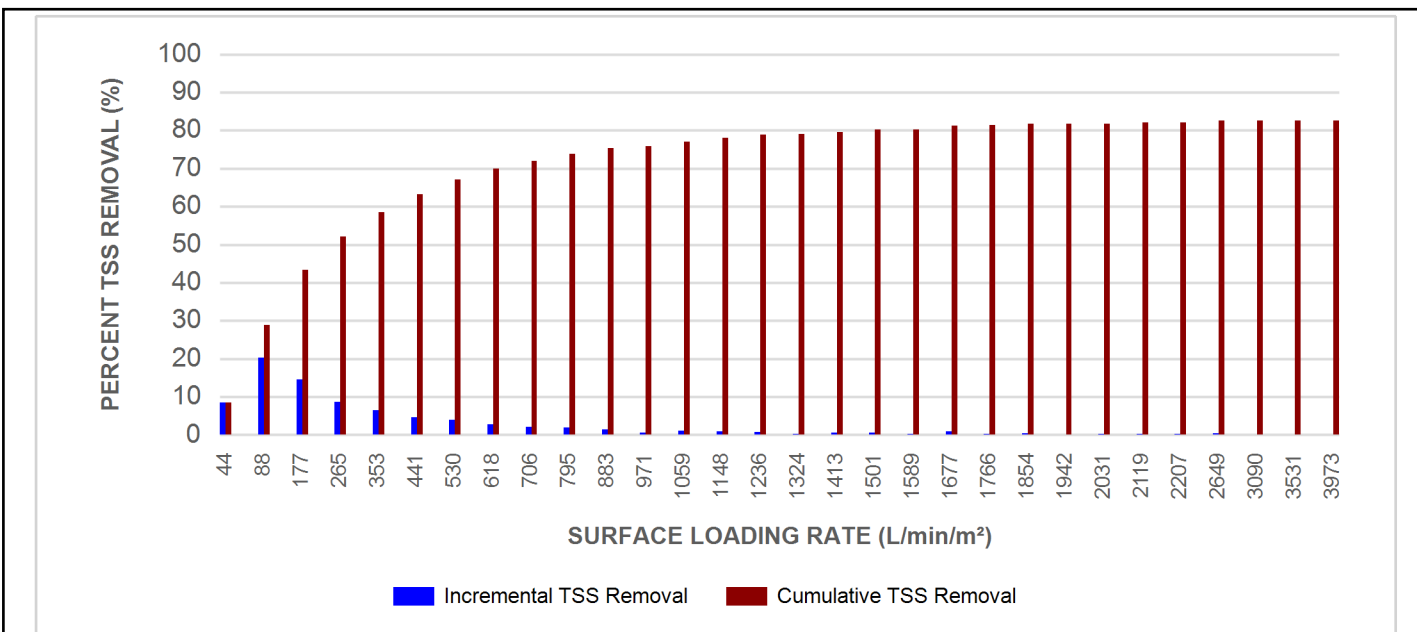
Climate Station ID: 6158731 Years of Rainfall Data: 20

Stormceptor®EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

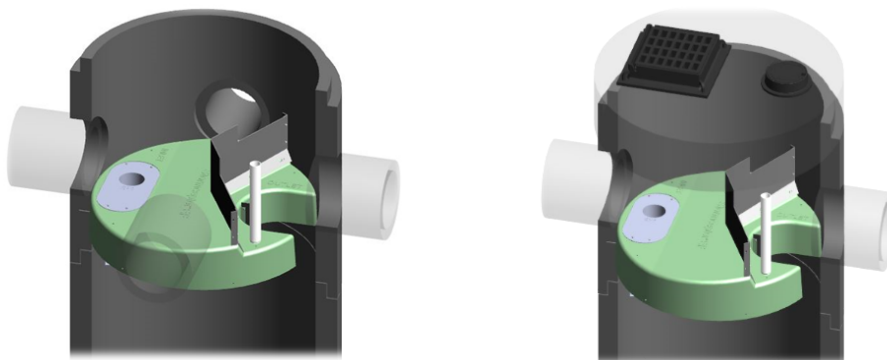
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

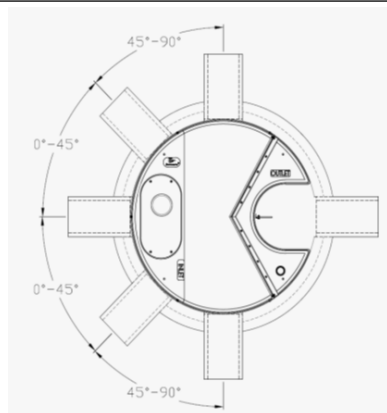
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor®EF Sizing Report

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

Stormceptor®EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

07/15/2022

Province:	Ontario	Project Name:	Ainley Farm Subdivision
City:	Elora	Project Number:	411009
Nearest Rainfall Station:	TORONTO INTL AP	Designer Name:	Patricia Wiebe
Climate Station Id:	6158731	Designer Company:	GM BluePlan Engineering Ltd.
Years of Rainfall Data:	20	Designer Email:	patricia.wiebe@gmblueplan.ca
		Designer Phone:	519-748-1440
Site Name:	SWM Pond 1, Outlet 1 - North	EOR Name:	
		EOR Company:	
Drainage Area (ha):	3.15	EOR Email:	
% Imperviousness:	50.00	EOR Phone:	

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	58.77
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EF4	67
EF6	78
EF8	85
EF10	90
EF12	92

Recommended Stormceptor EF Model: EF8
Estimated Net Annual Sediment (TSS) Load Reduction (%): 85
Water Quality Runoff Volume Capture (%): > 90

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

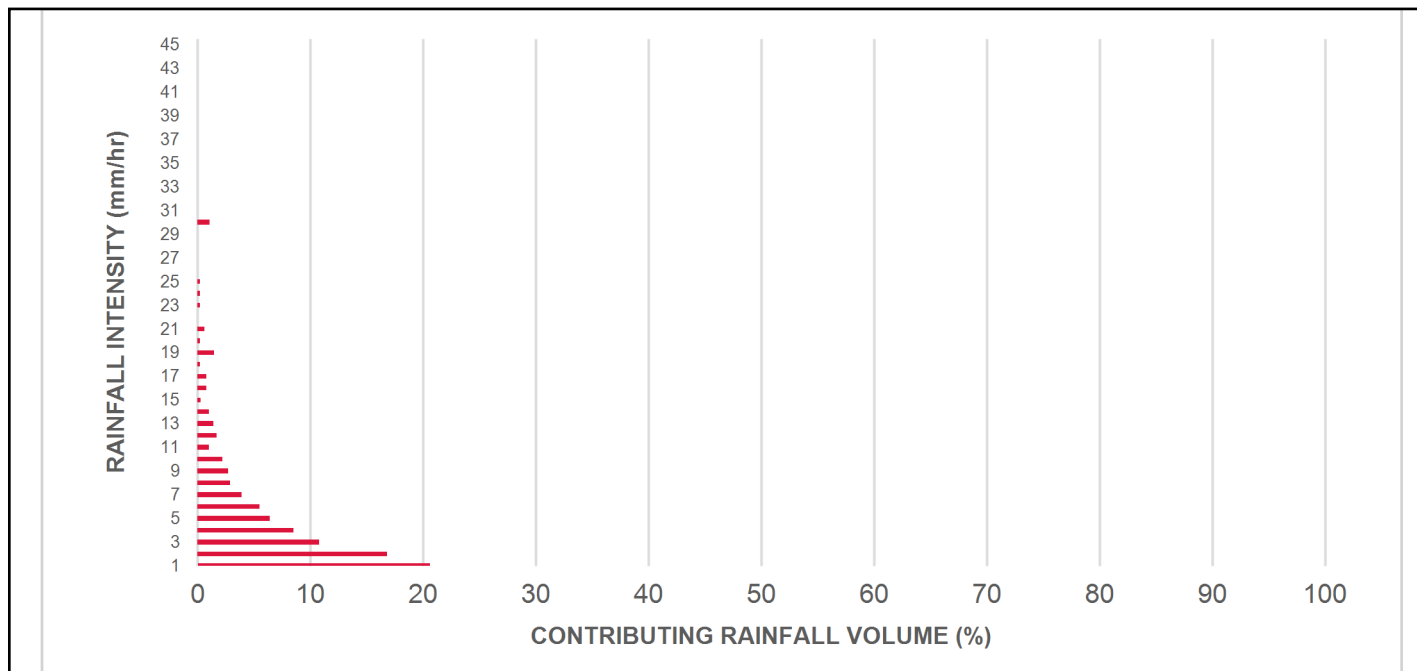
Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	2.63	158.0	34.0	100	8.5	8.5
1	20.6	29.1	5.25	315.0	67.0	100	20.6	29.1
2	16.8	45.9	10.51	631.0	134.0	92	15.5	44.6
3	10.8	56.7	15.76	946.0	201.0	83	8.9	53.5
4	8.5	65.2	21.02	1261.0	268.0	80	6.8	60.3
5	6.4	71.6	26.27	1576.0	335.0	77	4.9	65.3
6	5.5	77.0	31.53	1892.0	402.0	74	4.0	69.3
7	3.9	81.0	36.78	2207.0	470.0	73	2.9	72.2
8	2.9	83.9	42.03	2522.0	537.0	72	2.1	74.3
9	2.7	86.5	47.29	2837.0	604.0	71	1.9	76.1
10	2.2	88.7	52.54	3153.0	671.0	70	1.5	77.7
11	1.0	89.7	57.80	3468.0	738.0	70	0.7	78.4
12	1.7	91.3	63.05	3783.0	805.0	69	1.1	79.5
13	1.4	92.8	68.30	4098.0	872.0	69	1.0	80.5
14	1.0	93.7	73.56	4414.0	939.0	68	0.7	81.1
15	0.3	94.0	78.81	4729.0	1006.0	68	0.2	81.3
16	0.8	94.8	84.07	5044.0	1073.0	69	0.5	81.9
17	0.8	95.7	89.32	5359.0	1140.0	70	0.6	82.5
18	0.2	95.8	94.58	5675.0	1207.0	72	0.1	82.6
19	1.5	97.3	99.83	5990.0	1274.0	73	1.1	83.7
20	0.2	97.5	105.08	6305.0	1341.0	74	0.1	83.8
21	0.6	98.2	110.34	6620.0	1409.0	75	0.5	84.3
22	0.0	98.2	115.59	6936.0	1476.0	72	0.0	84.3
23	0.2	98.4	120.85	7251.0	1543.0	69	0.2	84.5
24	0.2	98.6	126.10	7566.0	1610.0	66	0.2	84.6
25	0.2	98.9	131.36	7881.0	1677.0	63	0.2	84.8
30	1.1	100.0	157.63	9458.0	2012.0	53	0.6	85.4
35	0.0	100.0	183.90	11034.0	2348.0	45	0.0	85.4
40	0.0	100.0	210.17	12610.0	2683.0	41	0.0	85.4
45	0.0	100.0	236.44	14186.0	3018.0	35	0.0	85.4
Estimated Net Annual Sediment (TSS) Load Reduction =								85 %

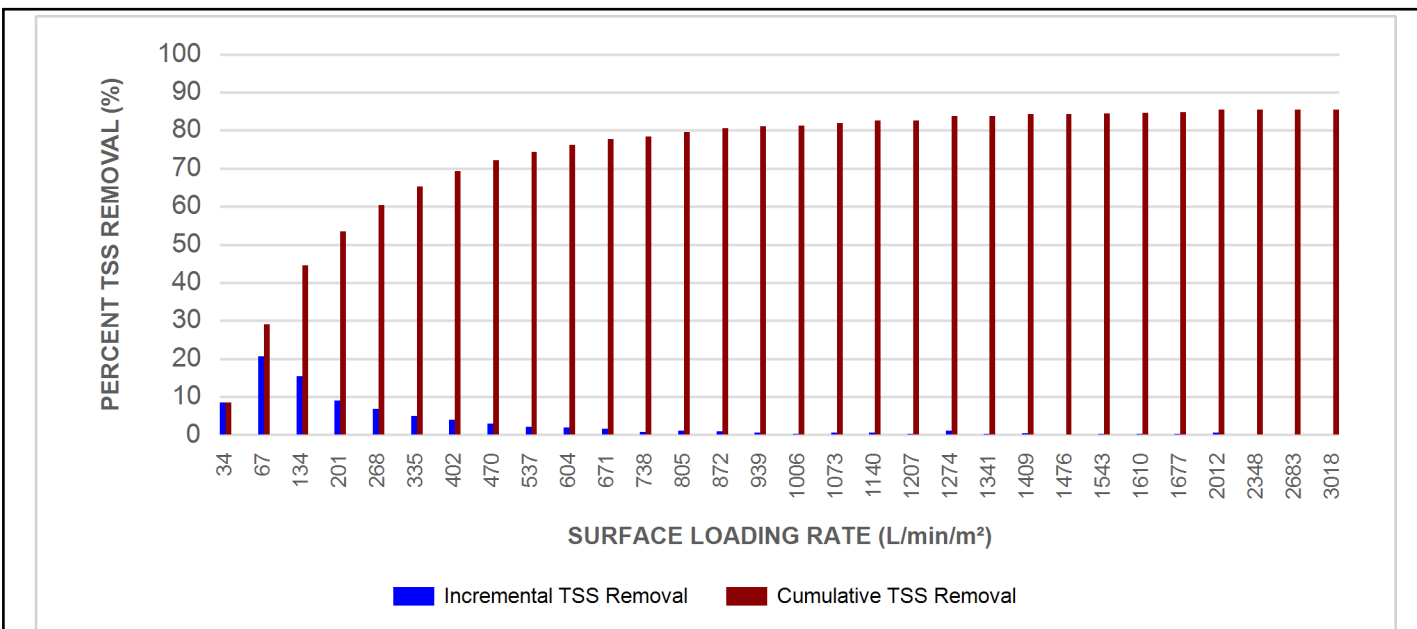
Climate Station ID: 6158731 Years of Rainfall Data: 20

Stormceptor®EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

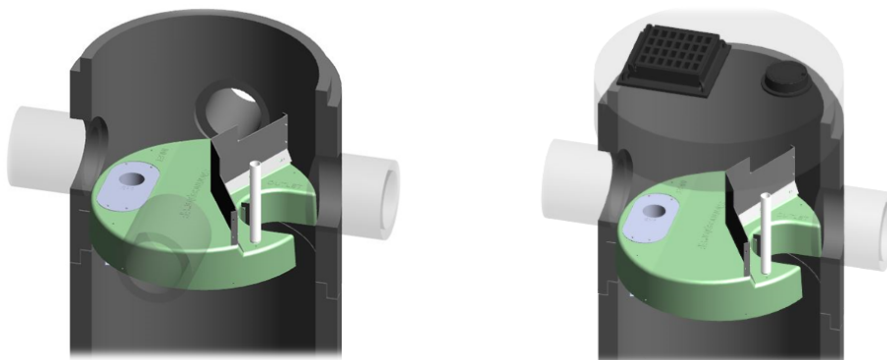
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

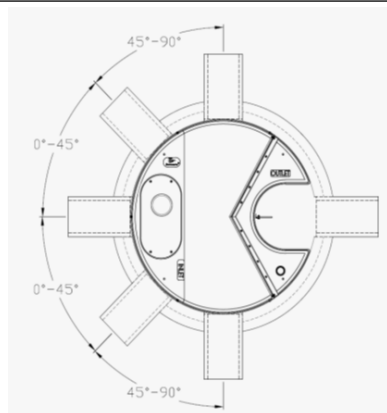
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor®EF Sizing Report

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

Stormceptor® EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

07/15/2022

Province:	Ontario	Project Name:	Ainley Farm Subdivision
City:	Elora	Project Number:	411009
Nearest Rainfall Station:	TORONTO INTL AP	Designer Name:	Patricia Wiebe
Climate Station Id:	6158731	Designer Company:	GM BluePlan Engineering Ltd.
Years of Rainfall Data:	20	Designer Email:	patricia.wiebe@gmblueplan.ca
		Designer Phone:	519-748-1440
Site Name:	SWM Pond 1, Outlet 2	EOR Name:	
		EOR Company:	
Drainage Area (ha):	2.32	EOR Email:	
% Imperviousness:	50.00	EOR Phone:	

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	43.29
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

**Net Annual Sediment
(TSS) Load Reduction
Sizing Summary**

Stormceptor Model	TSS Removal Provided (%)
EF4	72
EF6	83
EF8	88
EF10	92
EF12	95

Recommended Stormceptor EF Model: **EF6**

Estimated Net Annual Sediment (TSS) Load Reduction (%): **83**

Water Quality Runoff Volume Capture (%): **> 90**

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

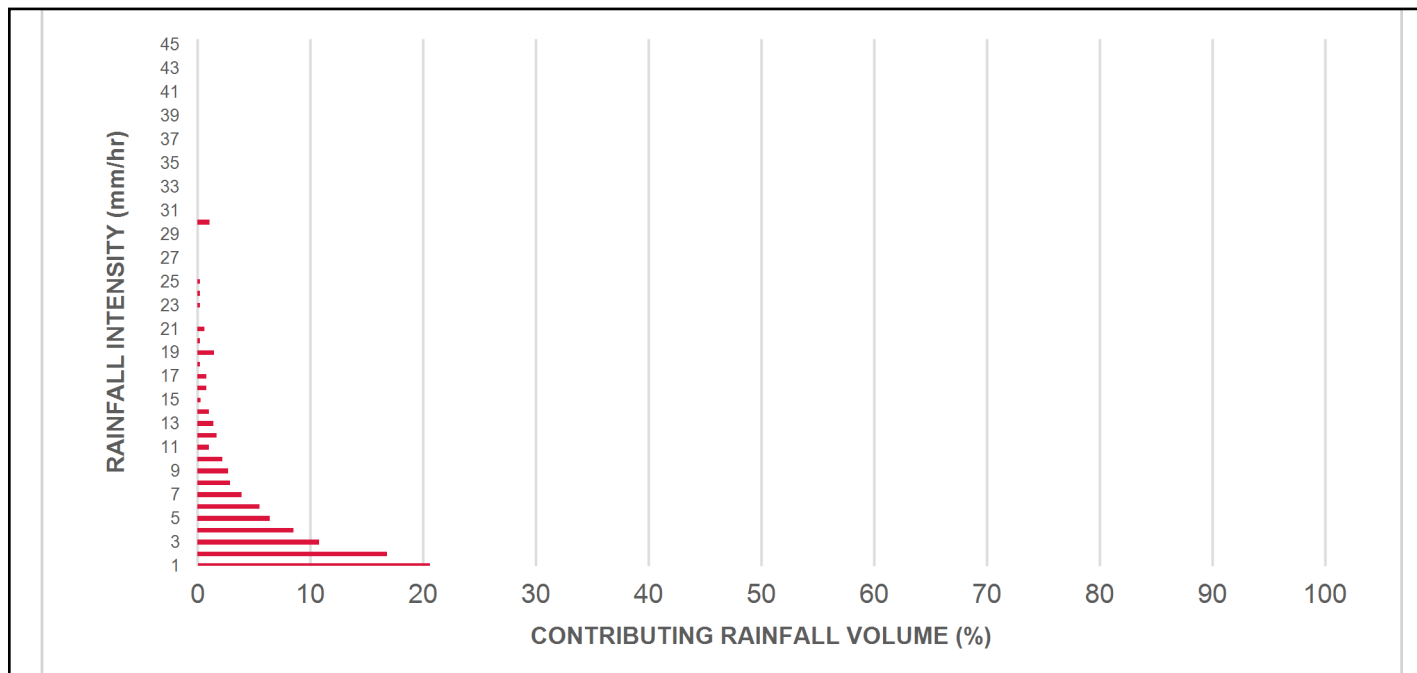
Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	1.93	116.0	44.0	100	8.5	8.5
1	20.6	29.1	3.87	232.0	88.0	98	20.3	28.8
2	16.8	45.9	7.74	464.0	177.0	87	14.6	43.4
3	10.8	56.7	11.61	697.0	265.0	80	8.6	52.1
4	8.5	65.2	15.48	929.0	353.0	76	6.4	58.5
5	6.4	71.6	19.35	1161.0	441.0	73	4.7	63.2
6	5.5	77.0	23.22	1393.0	530.0	72	3.9	67.1
7	3.9	81.0	27.09	1625.0	618.0	71	2.8	69.9
8	2.9	83.9	30.96	1857.0	706.0	70	2.0	71.9
9	2.7	86.5	34.83	2090.0	795.0	69	1.9	73.8
10	2.2	88.7	38.70	2322.0	883.0	69	1.5	75.3
11	1.0	89.7	42.57	2554.0	971.0	68	0.7	75.9
12	1.7	91.3	46.44	2786.0	1059.0	69	1.1	77.1
13	1.4	92.8	50.31	3018.0	1148.0	70	1.0	78.1
14	1.0	93.7	54.18	3251.0	1236.0	72	0.7	78.8
15	0.3	94.0	58.05	3483.0	1324.0	74	0.2	79.0
16	0.8	94.8	61.92	3715.0	1413.0	75	0.6	79.6
17	0.8	95.7	65.79	3947.0	1501.0	70	0.6	80.2
18	0.2	95.8	69.66	4179.0	1589.0	66	0.1	80.3
19	1.5	97.3	73.53	4412.0	1677.0	63	0.9	81.2
20	0.2	97.5	77.40	4644.0	1766.0	60	0.1	81.4
21	0.6	98.2	81.26	4876.0	1854.0	57	0.4	81.7
22	0.0	98.2	85.13	5108.0	1942.0	54	0.0	81.7
23	0.2	98.4	89.00	5340.0	2031.0	52	0.1	81.8
24	0.2	98.6	92.87	5572.0	2119.0	50	0.1	82.0
25	0.2	98.9	96.74	5805.0	2207.0	48	0.1	82.1
30	1.1	100.0	116.09	6966.0	2649.0	41	0.5	82.5
35	0.0	100.0	135.44	8126.0	3090.0	35	0.0	82.5
40	0.0	100.0	154.79	9287.0	3531.0	30	0.0	82.5
45	0.0	100.0	174.14	10448.0	3973.0	27	0.0	82.5
Estimated Net Annual Sediment (TSS) Load Reduction =								83 %

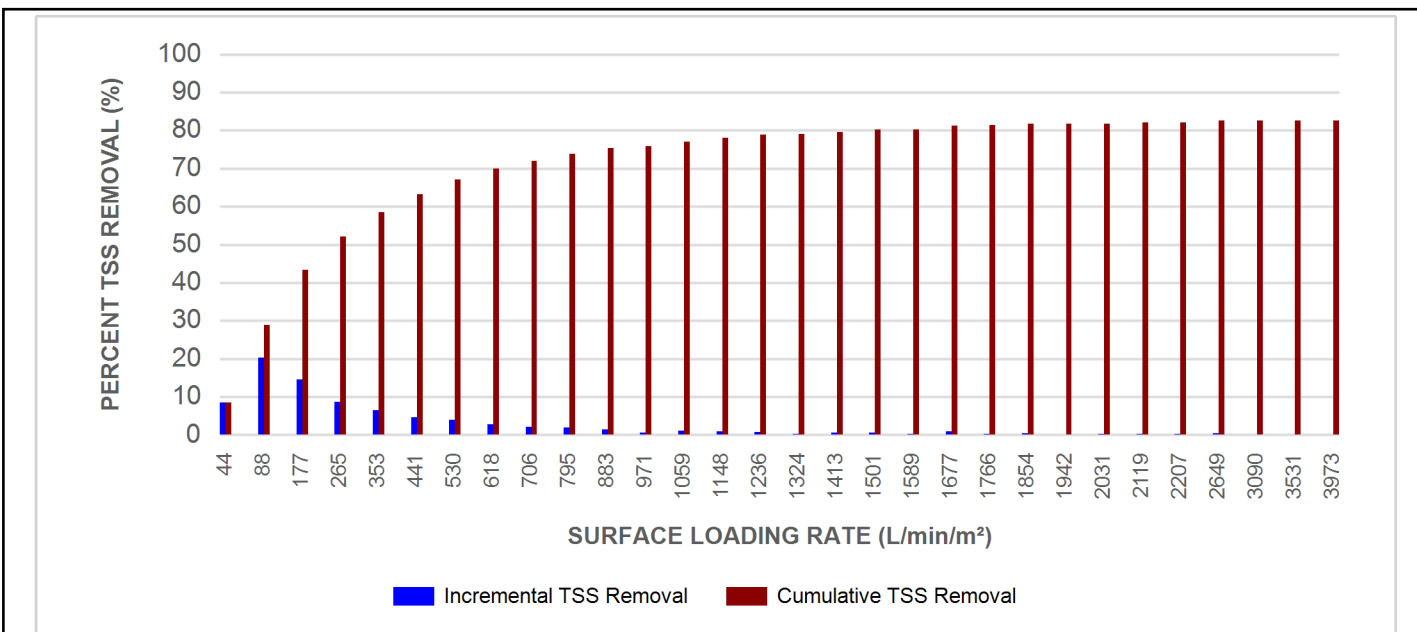
Climate Station ID: 6158731 Years of Rainfall Data: 20

Stormceptor®EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

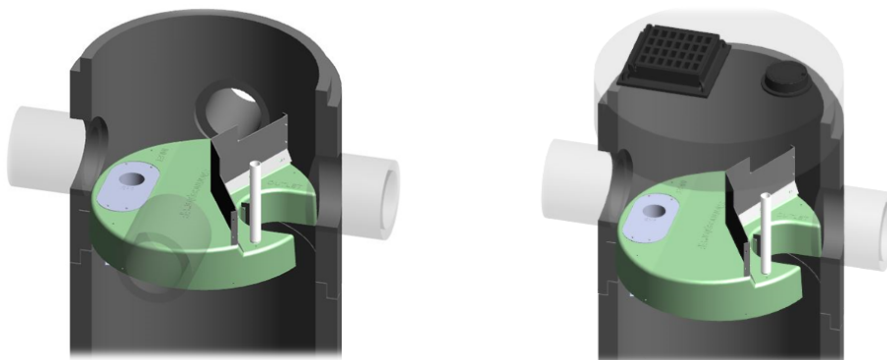
► **Stormceptor® EF and EFO** feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

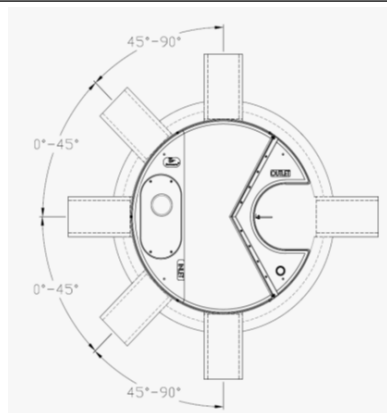
► **Stormceptor® EF and EFO** offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor®EF Sizing Report

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

Stormceptor®EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

07/15/2022

Province:	Ontario	Project Name:	Ainley Farm Subdivision
City:	Elora	Project Number:	411009
Nearest Rainfall Station:	TORONTO INTL AP	Designer Name:	Patricia Wiebe
Climate Station Id:	6158731	Designer Company:	GM BluePlan Engineering Ltd.
Years of Rainfall Data:	20	Designer Email:	patricia.wiebe@gmblueplan.ca
		Designer Phone:	519-748-1440
Site Name:	SWM Pond 2	EOR Name:	
		EOR Company:	
Drainage Area (ha):	2.75	EOR Email:	
% Imperviousness:	69.00	EOR Phone:	

Runoff Coefficient 'c': 0.71

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	61.06
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary

Stormceptor Model	TSS Removal Provided (%)
EF4	66
EF6	78
EF8	85
EF10	89
EF12	92

Recommended Stormceptor EF Model: EF8
Estimated Net Annual Sediment (TSS) Load Reduction (%): 85
Water Quality Runoff Volume Capture (%): > 90

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

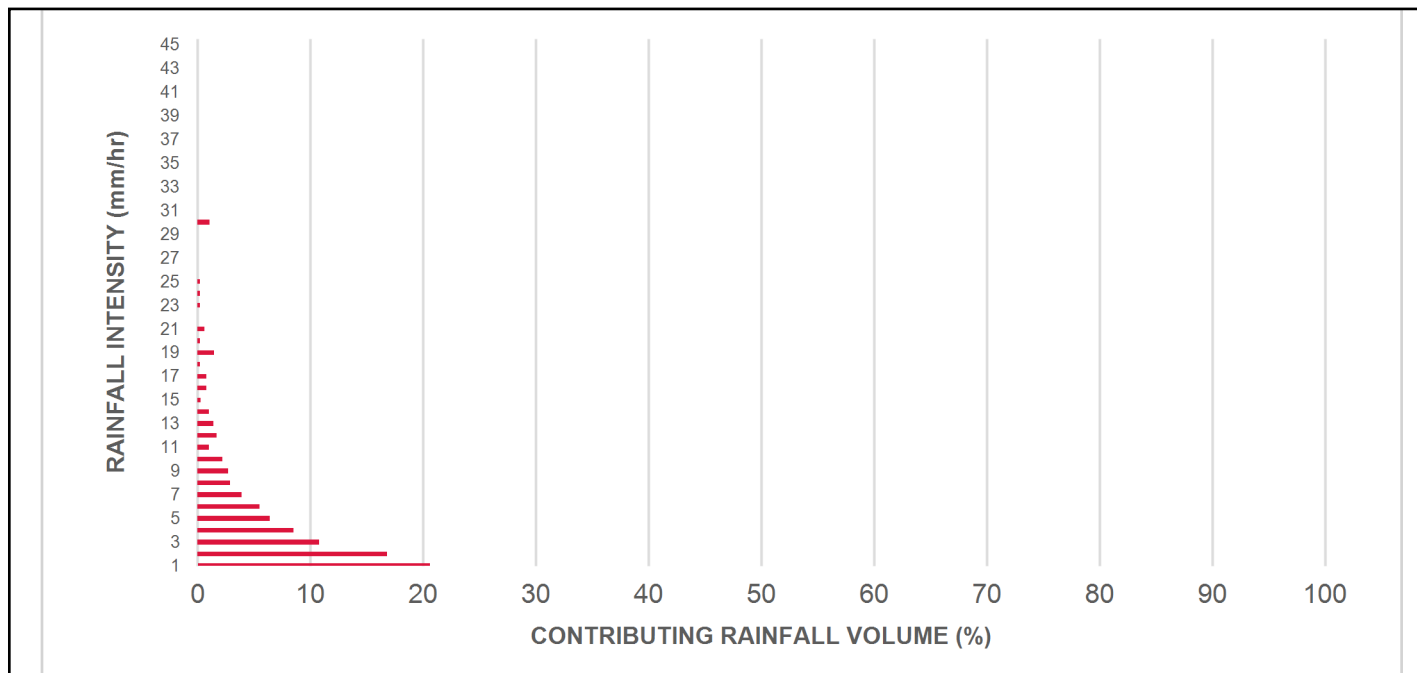
Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	2.73	164.0	35.0	100	8.5	8.5
1	20.6	29.1	5.46	328.0	70.0	100	20.6	29.1
2	16.8	45.9	10.92	655.0	139.0	91	15.3	44.4
3	10.8	56.7	16.38	983.0	209.0	83	8.9	53.3
4	8.5	65.2	21.83	1310.0	279.0	80	6.7	60.0
5	6.4	71.6	27.29	1638.0	348.0	77	4.9	64.9
6	5.5	77.0	32.75	1965.0	418.0	74	4.0	69.0
7	3.9	81.0	38.21	2293.0	488.0	73	2.9	71.8
8	2.9	83.9	43.67	2620.0	557.0	72	2.1	73.9
9	2.7	86.5	49.13	2948.0	627.0	71	1.9	75.8
10	2.2	88.7	54.59	3275.0	697.0	70	1.5	77.3
11	1.0	89.7	60.04	3603.0	767.0	70	0.7	78.0
12	1.7	91.3	65.50	3930.0	836.0	69	1.1	79.1
13	1.4	92.8	70.96	4258.0	906.0	68	1.0	80.1
14	1.0	93.7	76.42	4585.0	976.0	68	0.7	80.8
15	0.3	94.0	81.88	4913.0	1045.0	68	0.2	81.0
16	0.8	94.8	87.34	5240.0	1115.0	70	0.6	81.5
17	0.8	95.7	92.80	5568.0	1185.0	71	0.6	82.1
18	0.2	95.8	98.25	5895.0	1254.0	73	0.1	82.2
19	1.5	97.3	103.71	6223.0	1324.0	74	1.1	83.3
20	0.2	97.5	109.17	6550.0	1394.0	75	0.2	83.5
21	0.6	98.2	114.63	6878.0	1463.0	72	0.4	83.9
22	0.0	98.2	120.09	7205.0	1533.0	69	0.0	83.9
23	0.2	98.4	125.55	7533.0	1603.0	66	0.1	84.1
24	0.2	98.6	131.00	7860.0	1672.0	63	0.2	84.2
25	0.2	98.9	136.46	8188.0	1742.0	61	0.1	84.4
30	1.1	100.0	163.76	9825.0	2091.0	50	0.6	85.0
35	0.0	100.0	191.05	11463.0	2439.0	43	0.0	85.0
40	0.0	100.0	218.34	13100.0	2787.0	39	0.0	85.0
45	0.0	100.0	245.63	14738.0	3136.0	34	0.0	85.0
Estimated Net Annual Sediment (TSS) Load Reduction =								85 %

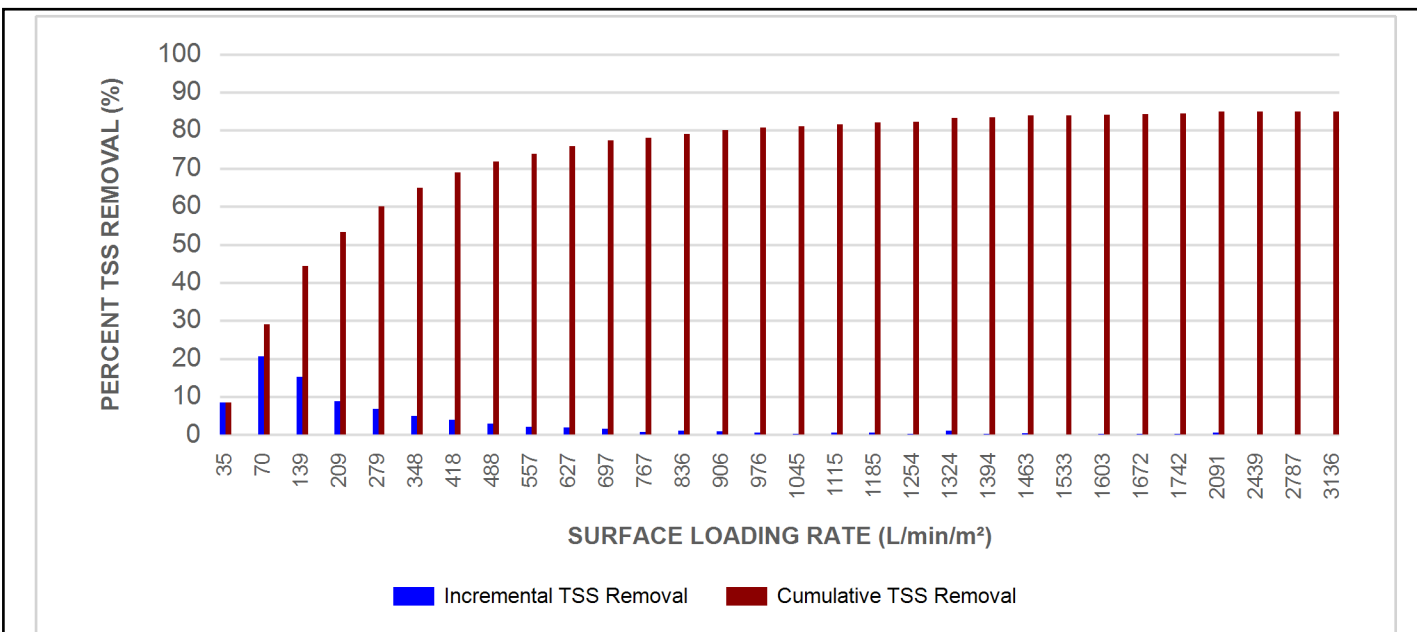
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Stormceptor®EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® EF Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

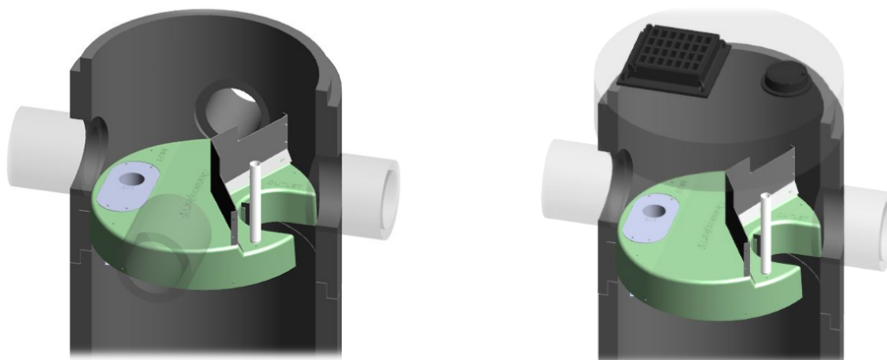
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DESIGN FLEXIBILITY

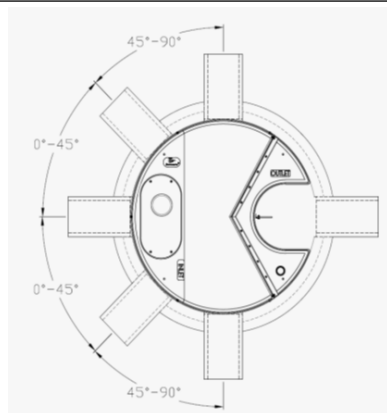
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OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, **Stormceptor® EFO** has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

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0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

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HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

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STANDARD STORMCEPTOR EF/EFO SPECIFICATION

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Stormceptor®EF Sizing Report

STANDARD PERFORMANCE SPECIFICATION FOR “OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

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The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².



APPENDIX E

WATER BUDGET ANALYSES

EXISTING CONDITION

Contributing Catchments:
Contributing Area =
Percent Impervious =

22.62 ha
0% %

All Soil Type: Clay Loam
Vegetation: Shallow-rooted crops
Root Zone Depth = 0.40m
Soil Moisture Retention Capacity = 100mm

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
Monthly Water Balance (Thornthwaite and Mather Method)
Date: July 2022

Runoff Factor =
Evapotranspiration Factor for
Impervious Surfaces =

0.84

0.36

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspir ation	Correction Factors	Adjusted Potential Evapotrans piration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotrans- piration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Runoff Volume	Recharge Volume
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m ³)	(m ³)
Jan	-7.40	0.00	0.00	24.3	0.00	67.90	67.9		236.5	0.0		0.0	0.0	0.0	0.0	9.9	0.0	9.9	8.3	1,887	351
Feb	-6.30	0.00	0.00	24.6	0.00	55.90	55.9		292.4	0.0		0.0	0.0	0.0	0.0	4.9	0.0	4.9	4.2	944	176
Mar	-1.90	0.00	0.00	30.6	0.00	59.60	59.6		352.0	0.0		0.0	0.0	0.0	0.0	2.5	0.0	2.5	2.1	472	88
Apr	5.70	1.22	0.90	33.6	30.24	74.10	43.9		100.0	0.0	30.2	30.2	0.0	0.0	43.9	21.9	25.2	47.1	39.7	8,988	1,673
May	12.20	3.86	2.00	37.8	75.60	86.90	11.3		100.0	0.0	75.6	75.6	0.0	0.0	11.3	16.6	113.4	130.0	109.6	24,795	4,614
Jun	17.50	6.66	2.90	38.4	111.36	83.80	-27.6	-27.6	75.0	-25.0	108.8	108.8	0.0	2.6	-2.6	7.0	56.7	63.7	53.7	12,154	2,262
Jul	20.00	8.16	3.40	38.7	131.58	89.20	-42.4	-69.9	49.0	-26.0	115.2	115.2	0.0	16.4	-16.4	-4.7	28.4	23.7	20.0	4,515	840
Aug	19.00	7.55	3.20	36.0	115.20	96.60	-18.6	-88.5	40.0	-9.0	105.6	105.6	0.0	9.6	-9.6	-7.1	14.2	7.0	5.9	1,342	250
Sep	14.90	5.22	2.50	31.2	78.00	93.10	15.1		55.1	15.1	78.0	78.0	0.0	0.0	0.0	-3.6	7.7	4.1	3.5	788	147
Oct	8.30	2.15	1.30	28.5	37.05	77.20	40.2		95.3	40.2	37.1	37.1	0.0	0.0	0.0	-1.8	4.0	2.2	1.9	423	79
Nov	2.10	0.27	0.30	24.3	7.29	93.00	85.7		100.0	4.8	7.3	7.3	0.0	0.0	81.0	39.6	2.5	42.1	35.5	8,027	1,494
Dec	-3.90	0.00	0.00	23.1	0.00	68.60	68.6		168.6	0.0		0.0	0.0	0.0	0.0	19.8	0.0	19.8	16.7	3,775	702
Total		35.1				945.9	359.6				557.8	557.8		28.5	107.6	105.1	252.0	357.1	301.1	68,109	12,674

Notes:

Precipitation and Temperature data from Environment Canada Climate Normals 1981-2010 for Fergus Shand Dam

Monthly water balance strategy as outlined in the document *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thornthwaite and Mather, 1957)*

Monthly Heat Index (I) from Table 2 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*

Correction Factors from Table 6 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*

Evaporation Factor for Impervious Surfaces = Average Annual Evapotranspiration for Impervious Surfaces (200mm/year) / Average Annual Evapotranspiration for Pervious Surfaces (558mm/year) = 0.36

Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces (745.9mm/year)) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces (301.1 mm/year))]/ Total Annual Recharge & Runoff

POST-DEVELOPMENT CONDITIONS - TO WETLAND

Contributing Catchments: All

Contributing Area = 22.62 ha

Percent Impervious = 36%

Soil Type: Clay Loam

Vegetation: Shallow-rooted crops

Root Zone Depth = 0.40m

Soil Moisture Retention Capacity = 100mm

Ainley Farm Subdivision

Township of Centre Wellington (Elora)

Monthly Water Balance (Thornthwaite and Mather Method)

Date: July 2022

Runoff Factor = 0.95

Evapotranspiration Factor for Impervious 0.36

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspiration	Correction Factors	Adjusted Potential Evapotranspiration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotranspiration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Runoff Volume	Recharge Volume	Enhanced Recharge
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m³)	(m³)	(m³)
Jan	-7.40	0.00	0.00	24.3	0.00	67.90	67.9		236.5	0.0		0.0	0.0	0.0	0.0	11.9	0.0	11.9	11.3	2,381	135	184
Feb	-6.30	0.00	0.00	24.6	0.00	55.90	55.9		292.4	0.0		0.0	0.0	0.0	0.0	6.0	0.0	6.0	5.7	1,190	67	92
Mar	-1.90	0.00	0.00	30.6	0.00	59.60	59.6		352.0	0.0		0.0	0.0	0.0	0.0	3.0	0.0	3.0	2.8	595	34	46
Apr	5.70	1.22	0.90	33.6	30.24	74.10	43.9		100.0	0.0	30.2	23.3	7.0	7.0	50.8	25.4	25.2	50.6	48.1	10,095	571	785
May	12.20	3.86	2.00	37.8	75.60	86.90	11.3		100.0	0.0	75.6	58.1	17.5	17.5	28.8	27.1	113.4	140.5	133.5	28,773	1,584	1,421
Jun	17.50	6.66	2.90	38.4	111.36	83.80	-27.6	-27.6	75.0	-25.0	108.8	83.7	25.1	27.7	22.6	24.8	56.7	81.5	77.5	16,455	919	1,067
Jul	20.00	8.16	3.40	38.7	131.58	89.20	-42.4	-69.9	49.0	-26.0	115.2	88.6	26.6	43.0	10.2	17.5	28.4	45.9	43.6	9,165	517	695
Aug	19.00	7.55	3.20	36.0	115.20	96.60	-18.6	-88.5	40.0	-9.0	105.6	81.2	24.4	34.0	14.8	16.2	14.2	30.3	28.8	6,066	342	452
Sep	14.90	5.22	2.50	31.2	78.00	93.10	15.1		55.1	15.1	78.0	60.0	18.0	18.0	18.0	17.1	7.7	24.8	23.5	4,958	279	368
Oct	8.30	2.15	1.30	28.5	37.05	77.20	40.2		95.3	40.2	37.1	28.5	8.6	8.6	8.6	12.8	4.0	16.8	16.0	3,366	190	249
Nov	2.10	0.27	0.30	24.3	7.29	93.00	85.7		100.0	4.8	7.3	5.6	1.7	1.7	82.6	47.7	2.5	50.2	47.7	10,021	566	775
Dec	-3.90	0.00	0.00	23.1	0.00	68.60	68.6		168.6	0.0		0.0	0.0	0.0	0.0	23.9	0.0	23.9	22.7	4,762	269	368
Total		35.1				945.9	359.6				557.8	429.0	128.8	157.3	236.4	233.4	252.0	485.4	461.2	97,828	5,473	6,502
Total Recharge Volume																					11,974	
Total Enhanced Recharge Surplus (post-development volume - pre-development volume)																					-700	

Notes: Precipitation and Temperature data from Environment Canada Climate Normals 1981-2010 for Fergus Shand Dam

Monthly water balance strategy as outlined in the document *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thornthwaite and Mather, 1957)*

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Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces (745.9mm/year)) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces (301.1 mm/year))]/ Total Annual Recharge &

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
Monthly Water Balance (Thorntwaite and Mather Method)
Date: July 2022

Catchment 1200 - Stormwater Management Pond 1 Infiltration Gallery

Infiltration Gallery No. 1

Area = 540 m²
Depth= 0.70 m
Perforated Pipe Diameter= 0.450 m
No. of Pipes= 4 - 6

Volume of Perforated Pipe = 100.52 cu m
Volume of Clear Stone = 277.41 cu m
Clear Stone Void Ratio= 0.33

Total Storage Volume of Structure = 192.06 cu m

A = contact area of structure = 540 sq m
V = runoff volume to be infiltrated = 192.06 cu m
P = percolation rate of native soils = 4.00 mm/h
n = porosity of storage media (weighted) = 0.51
T = retention time = Solve for T

$T = (1000 \times V) / (P \times n \times A) = 175.00$ hours or 7.3 day draindown period

Contributing Area 1.14 ha
Recharge Time 175.0 hours / 7.3 days
Recharge Volume Potential 192.06 m³

Month	Total Runoff from Contributing Area (mm)	No. of days	Max Potential Recharge (m ³)	Available Recharge (m ³)	Enhanced Recharge (m ³)
Jan	12.4	31	817	142	135
Feb	6.2	28	738	71	67
Mar	3.1	31	817	35	34
Apr	50.8	30	790	579	551
May	141.4	31	817	1,612	776
Jun	86.5	30	790	986	751
Jul	53.3	31	817	608	577
Aug	38.5	31	817	439	417
Sep	32.1	30	790	366	348
Oct	22.0	31	817	251	238
Nov	52.2	30	790	595	566
Dec	24.9	31	817	284	270
Total	523.5	365	9,614	5,968	4,728

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
Monthly Water Balance (Thorntwaite and Mather Method)
Date: July 2022

Catchment 1400 - Park Infiltration Gallery

Infiltration Gallery No. 3

Length = 79.05 m
Width = 10.00 m
Depth = 0.70 m
Perforated Pipe Diameter = 0.30 m
No. of Pipes = 1.00

Area of Material = 790.50 sq m

Volume of Perf. Pipe = 11.03
Volume of Clear Stone = 542.32 cu m
Clear Stone Void Ratio = 0.33

Total Storage Volume of Structure = 191.80 cu m

A = contact area of structure = 790.50 sq m
V = runoff volume to be infiltrated = 191.80 cu m
P = percolation rate of native soils = 4.00 mm/h
n = porosity of storage media (weighted) = 0.34
T = retention time = Solve for T

$T = (1000 \times V) / (P \times n \times A) = 176.67$ hours or 7.4 day draindown period

Contributing Area 0.62 ha
Recharge Time 176.7 hours / 7.4 days
Recharge Volume Potential 191.80 m³

Month	Total Runoff from Contributing Area	No. of days	Max Potential Recharge	Available Recharge	Enhanced Recharge
	(mm)		(m ³)	(m ³)	(m ³)
Jan	8.3	31	808	52	49
Feb	4.2	28	730	26	25
Mar	2.1	31	808	13	12
Apr	39.7	30	782	246	234
May	109.6	31	808	680	646
Jun	53.7	30	782	333	316
Jul	20.0	31	808	124	118
Aug	5.9	31	808	37	35
Sep	3.5	30	782	22	21
Oct	1.9	31	808	12	11
Nov	35.5	30	782	220	209
Dec	16.7	31	808	103	98
Total	301.1	365	9,511	1,867	1,773