

Prepared By:



Functional Servicing And Stormwater Management Report

Glen Allan Subdivision
Township of Mapleton

GMBP File: 317033

January 2019

TABLE OF CONTENTS

1.	INRODUCTION	1
2.	EXISTING CONDITIONS	1
3.	PROPOSED DEVELOPMENT.....	1
3.1	Grading.....	1
3.2	Water Supply.....	2
3.3	Sanitary	2
3.4	Storm	2
3.5	Stormwater Management Design.....	3
3.5.1	Identification of Drainage Outlets.....	3
3.5.2	Stormwater Management Criteria.....	3
3.5.3	Existing Condition Drainage Areas	4
3.5.4	Post-Development Condition Drainage Areas.....	5
3.5.5	Stormwater Management System Details	5
3.5.6	Routing.....	6
3.5.7	Maintenance Plan	8
3.5.8	Annual Runoff Volumes Draining to the Easterly Outlet.....	8
3.6	Roadways.....	9
3.7	Utilities and Lighting	10
3.8	Sediment and Erosion Control	10
4.	SUMMARY	11

LIST OF FIGURES

		FOLLOWING PAGE
FIGURE 1	SITE LOCATION MAP.....	1
FIGURE 2	EXISTING CONDITIONS SITE PLAN.....	1
FIGURE 3	DRAFT PLAN OF SUBDIVISION.....	1
FIGURE 4	CONCEPTUAL GRADING AND SERVICING PLAN.....	2
FIGURE 5	EXISTING CONDITION CATCHMENT AREAS.....	4
FIGURE 6	POST DEVELOPMENT CATCHMENT AREAS.....	5
FIGURE 7	TRIBUTARY AREA OF OGS UNIT.....	5

APPENDICES

APPENDIX A: STORMWATER MANAGEMENT ANALYSIS

GLEN ALLAN SUBDIVISION

FUNCTIONAL SERVICING AND STORMWATER MANAGEMENT REPORT

JANUARY 2019

GMBP FILE: 317033

1. INTRODUCTION

GM BluePlan Engineering Limited has been retained by the Glen Allan Subdivision ownership group to prepare this Functional Servicing and Stormwater Management Report in support of a Draft Plan of Subdivision application for a residential subdivision in the hamlet of Glen Allan, in the Township of Mapleton.

The subject property is located in the south side of Glen Allan, legally described as Lots 37, 38 and 39 east of Centre Street and south of Hill Street, and Lots 40 and 41 west of Centre Street and south of Hill Street, and Lots 42 and 43 west of Centre Street and north of George Street, and Lots 44, 45 and 46 east of Centre Street and north of George Street, and Lots 62, 63 and 64 west of South Mill Street and south of George Street, and Lots 65, 66 and 67 east of Centre Street and south of George Street, and Lots 68 and 69 west of Centre Street and south of George Street, and Lots 70 and 71 west of Centre Street and north of Wellesley Street, and Lots 72, 73 and 74 east of Centre Street and north of Wellesley Street, and Lots 75, 76 and 77 west of South Mill Street and north of Wellesley Street all being in Donald Sutherland's Survey and Part of Lot 5, Concession 2, Geographic Township of Peel, Township of Mapleton, County of Wellington and also includes the unopened road allowances of Hill Street, George Street, Wellesley Street, South Mill Street, Centre Street. The approximately 4.285 hectare (10.588 acre) site is situated west of South Mill Street and south of Wellington Road 45 with access from George Street. Figure No. 1, Site Location Map, shows the location of the proposed development and the surrounding area.

2. EXISTING CONDITIONS

The property is currently designated for Residential use in the Wellington County Official Plan and zoned as Unserviced Residential and Future Development in Mapleton's current Zoning By-Law. The site is bound by a mixture of land uses, including, residential to the north and east, future development to the south (currently agricultural) and agricultural to the west. The site is currently used as an actively cultivated farm field that splits the drainage so part of the site drains to the northwest and the remainder drains to the southeast. Figure No. 2, Existing Conditions Site Plan, shows the existing site conditions.

3. PROPOSED DEVELOPMENT

The proposed plan of subdivision has been prepared by MHBC Planning. The Draft Plan consists of 11 single family rural estate residential lots, a stormwater management block, a future development block (for a future cul-de-sac on Centre Street) and a road widening along South Mill Street. Approximately 200m of 20.0m wide roads will be constructed covering an area of about 0.548 hectare. It is anticipated that the development will be Draft Approved, Registered and constructed at one time. Figure No. 3 shows the Draft Plan of Subdivision.

3.1 Grading

Based on our review of the site's topography and our preliminary calculation, we consider that the development can be graded to conform to the intent of the Township of Mapleton's Development Standards. The existing

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and Stormwater
Management Report
Glen Allan Subdivision
Township of Mapleton



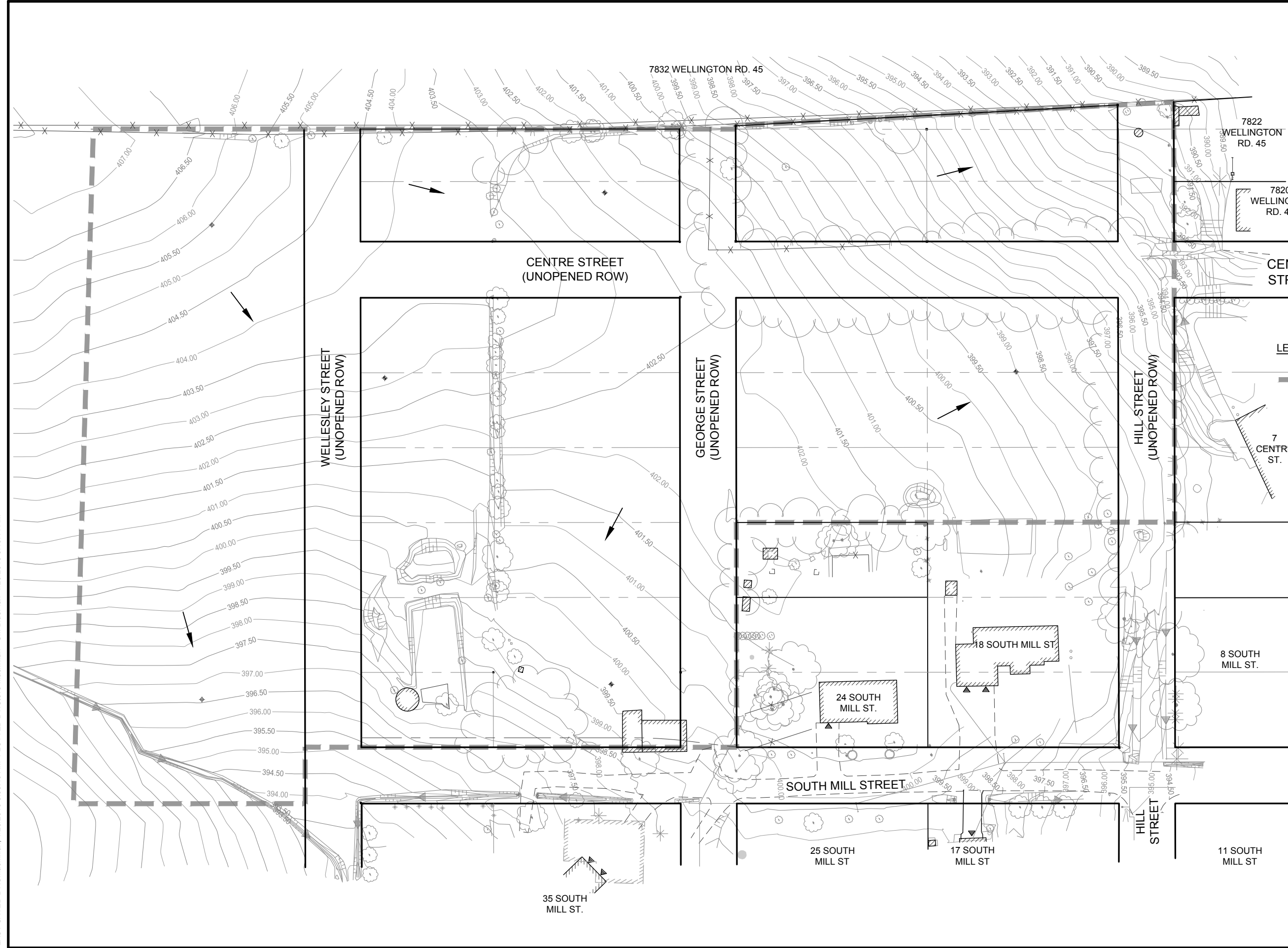
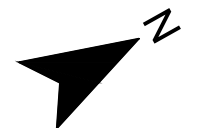
SITE
LOCATION MAP

Figure No. 1



317033
January 2019
Scale: N.T.S.

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and Stormwater
Management Report
Glen Allan Subdivision
Township of Mapleton



LEGEND:

- SITE BOUNDARY
- EXISTING DIRECTION OF SURFACE DRAINAGE

EXISTING CONDITIONS
SITE PLAN

Figure No. 2

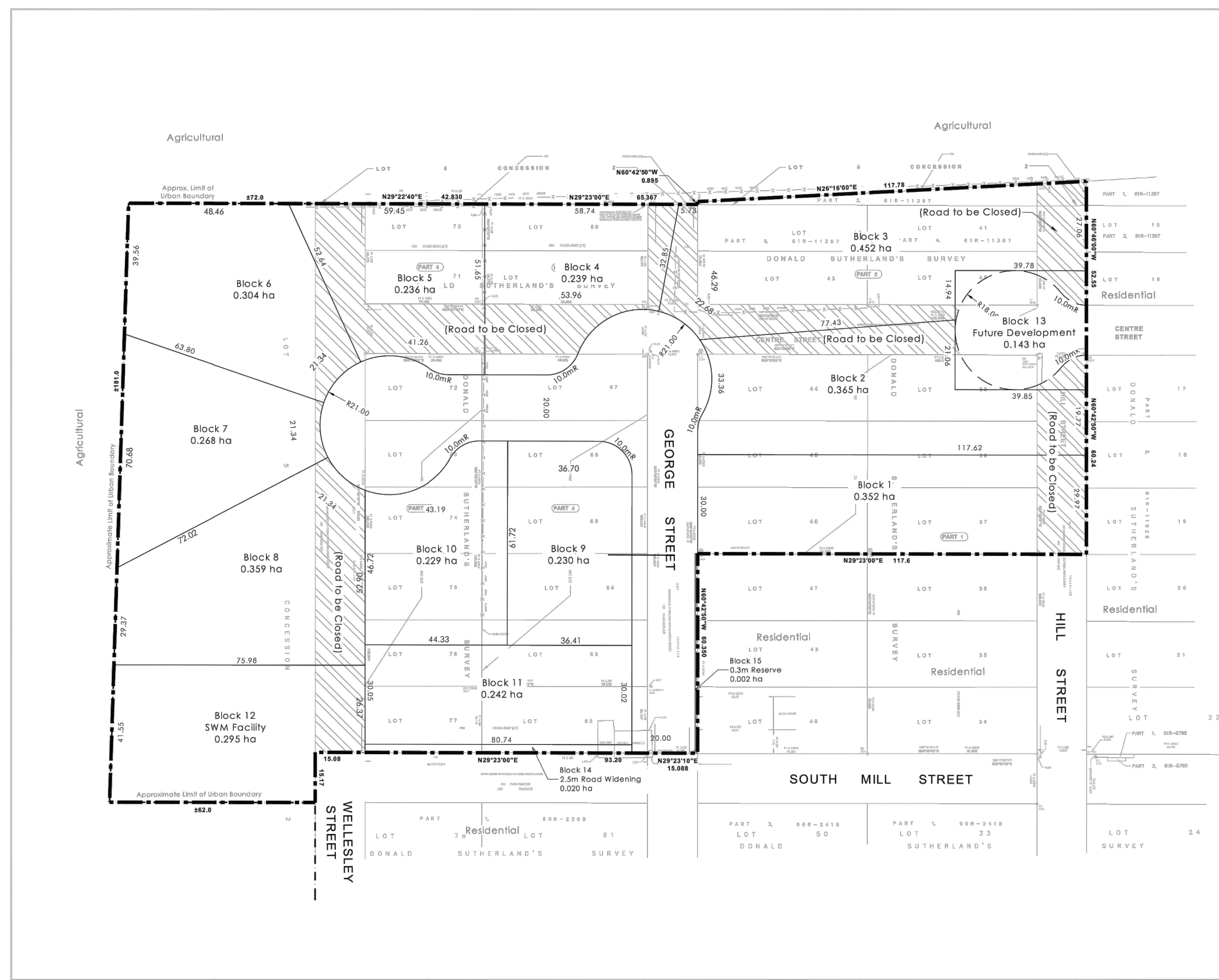


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Management Report
Glen Allan Subdivision
Township of Mapleton



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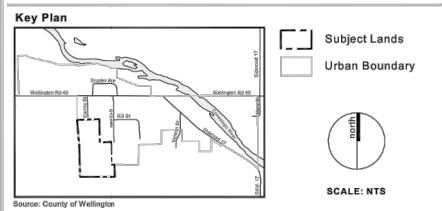


DRAFT PLAN OF SUBDIVISION

Legal Description
LOTS 34-38 WEST OF SOUTH MILL ST AND SOUTH OF HILL ST &
LOTS 37-39 EAST OF CENTRE ST AND SOUTH OF HILL ST &
LOTS 40 AND 41 WEST OF CENTRE ST AND SOUTH OF HILL ST &
LOTS 42 AND 43 WEST OF CENTRE ST AND NORTH OF GEORGE ST &
LOTS 44-48 EAST OF CENTRE ST AND NORTH OF GEORGE ST &
LOTS 62-64 WEST OF SOUTH MILL ST AND SOUTH OF GEORGE ST &
LOTS 65-67 EAST OF CENTRE ST AND SOUTH OF GEORGE ST &
LOTS 68 AND 69 WEST OF CENTRE ST AND SOUTH OF GEORGE ST &
LOTS 70 AND 71 WEST OF CENTRE ST AND NORTH OF WELLESLEY ST &
LOTS 72-74 EAST OF CENTRE ST AND NORTH OF WELLESLEY ST &
LOTS 75-77 WEST OF SOUTH MILL ST AND NORTH OF WELLESLEY ST,
ALL BEING IN DONALD SUTHERLAND'S SURVEY AND
PART OF LOT 5, CONC. 2
(GEOGRAPHIC TOWNSHIP OF PEEL)
TOWNSHIP OF MAPLETON
COUNTY OF WELLINGTON

Owner's Certificate
I HEREBY AUTHORIZE MACNAUGHTON HERMSEN BRITTON CLARKSON PLANNING LIMITED TO SUBMIT THIS PLAN FOR APPROVAL.
DATE: _____
Heather Smith & Steve Guschbaer
DATE: _____
Steve Sebben
DATE: _____
Murray Martin

Surveyor's Certificate
I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ON THIS PLAN AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.
DATE: _____
NAME, OLS (COMPANY)



Additional Information Required Under Section 51(17) of the Planning Act R.S.O. 1990, c.P.13 as Amended

A. AS SHOWN	B. AS SHOWN	C. AS SHOWN
D. RESIDENTIAL	E. AS SHOWN	F. AS SHOWN
G. AS SHOWN	H. MUNICIPAL WATER SUPPLY	I. LOAM
J. AS SHOWN	K. ALL SERVICES REQUIRED	L. AS SHOWN

Area Schedule

Description	Lots/Blocks	Units	Area (ha)
Residential	1-11	11	3.277
Storm Water Management	12		0.295
Future Development	13		0.143
Road Widening	14		0.020
0.3m Reserve	15		0.002
Roads			0.548
Total	15	11	4.285

Notes
1. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE SHOWN.
2. BOUNDARY INFORMATION FROM PLAN OF SURVEY PREPARED BY VAN HARTEN SURVEYING INC., NOV. 6, 2017

1	Dec. 13, 2018	Remove 6.0m SWM Access	CAC
Revision No.	Date	Issued / Revision	By



Approval Stamp

Date	Dec. 13, 2018
File No.	17410A
Plan Scale	1:500 (Arch D)
Drawn By	DGS/CAC
Checked By	PC
Project	Glen Allan Subdivision
Applicants	Heather Smith, Steve Guschbaer, Steve Sebben & Murray Martin Mapleton, ON.
File Name	DRAFT PLAN
Dwg No.	1 of 1



DRAFT PLAN OF
SUBDIVISION
Figure No. 3



topography provides a total vertical relief across the site of approximately 17m to the northwest and 13m to the southeast. This will provide adequate opportunity for the majority of the site to drain to the proposed stormwater management facility located in the southeast corner of the site, while maintaining existing drainage patterns and upstream flow paths. The remainder of the site, which under existing conditions drains towards the northeast, will continue to drain there under proposed conditions.

Road and lot grades will be designed to ensure proper drainage and sufficient cover over most municipal storm sewers. Road grades will range from 0.5% to 6.0% across the majority of the site. Lot grading will consist of a mixture of lot types such as back to front drainage, where the lots will drain from the rear of the lot to the front street line and split drainage (including walkout and backsplitted lots), where the front of the lots will drain to the front street line and runoff from the rear of the lots will drain towards grassed rear yard swales. Side yard swale grades will range from 2.0% to 6.0%. Houses on Blocks 1-3 are to be located at the minimum front yard setback and all roof leaders are to drain to the front yard. Runoff from roof leaders will be directed to and filtered across grassed surfaces.

The preliminary grades shown on the Conceptual Grading Plan indicate a functional design, however they have not been assessed for efficiency and cut / fill balance and may need to be adjusted accordingly, during the detailed design phase.

During site grading construction, temporary sediment and erosion control measures (such as sediment fencing, sediment ponds, straw bale check dams, etc.) will be implemented for environmental protections. Further details are discussed in Section 3.8. Figure No. 4, Conceptual Grading and Servicing Plan, shows the conceptual site grading.

3.2 Water Supply

There is no existing municipal water distribution system in Glen Allan. Therefore, each lot will have its own private drilled well. Each well is to be installed according to the Ontario Water Resources Act, Regulation 903 (O. Reg. 903) by a licenced MOE Contractor and registered with the MOE. The final location of each well will be determined and approved at the time of building permit application. A MECP D-5-5 drinking water assessment will be completed (under separate cover) to assess the supply and quality of water. Figure No. 4, Conceptual Grading and Servicing Plan, shows a potential location for each private drilled well.

3.3 Sanitary

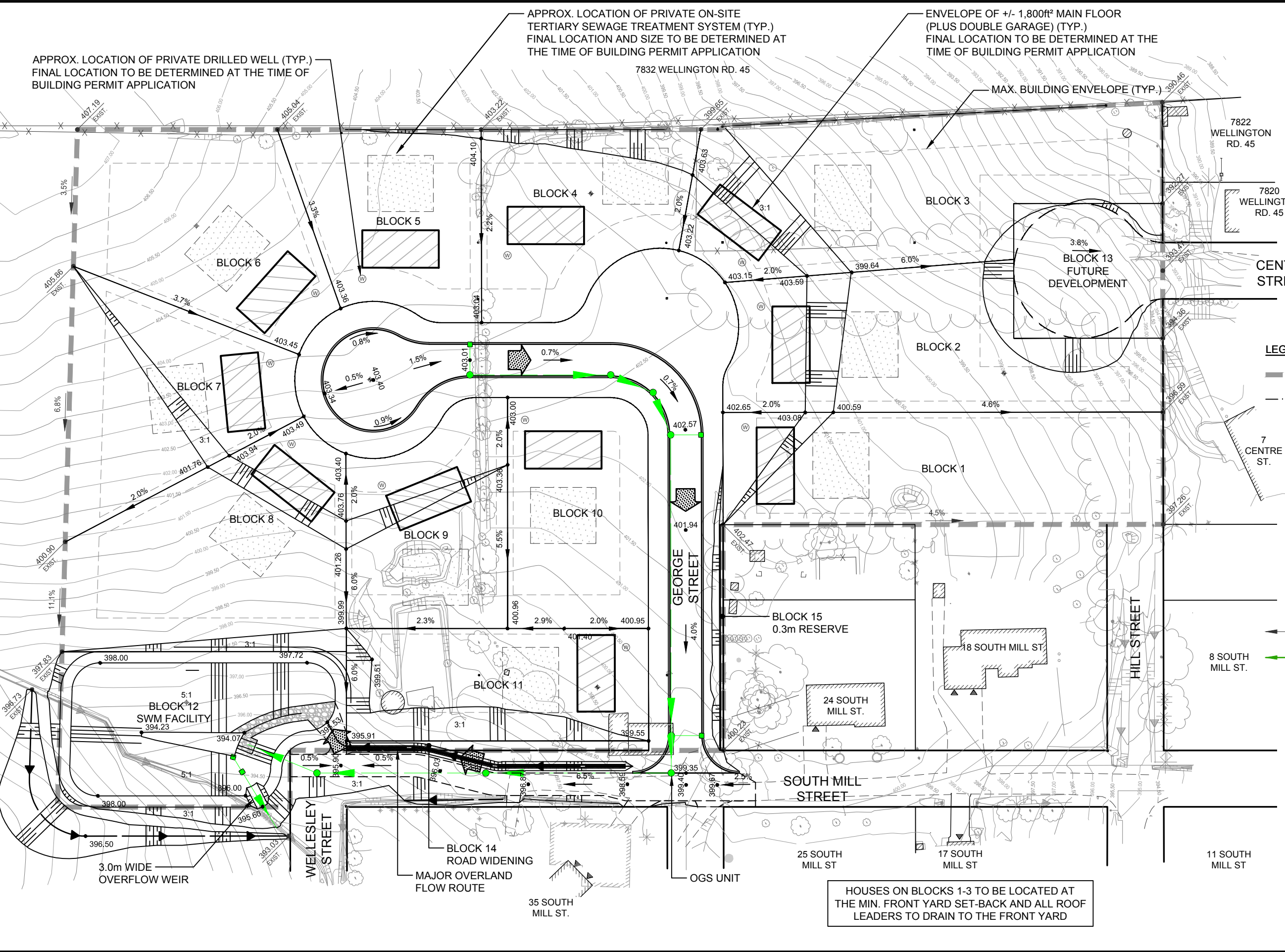
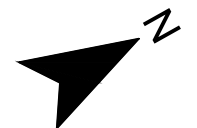
There is no existing municipal sanitary sewage collection system in Glen Allan. Therefore, each lot will have its own private tertiary on-site sewage treatment system. Each on-site sewage treatment system is to be designed and installed according to the Ontario Building Code by a licenced BCIN (Building Code Identification Number) contractor. The final location of each on-site sewage treatment system will be determined and approved at the time of building permit application. A MECP D-5-4 sewage system assessment will be completed (under separate cover) to assess the potential impact on groundwater caused by on-site sewage systems. Figure No. 4, Conceptual Grading and Servicing Plan, shows a potential location for each the on-site sewage treatment system.

3.4 Storm

There are no existing municipal storm sewers in close proximity to the site. South Mill Street has a rural cross section, using ditches and culverts to convey storm water.

To service this property, municipal storm sewers (300mm – 450mm dia.) will be extended on George Street and South Mill Street. The storm sewers will be sized to convey the runoff generated within the subdivision during a minor design storm event (1 in 5 years) and direct it to the on-site storm water management facility. Catch basins will be spaced according to the Township's standards. Due to the site's topography, it is

Functional Servicing
and Stormwater
Management Report
Glen Allan Subdivision
Township of Mapleton



- LEGEND:**
- SITE BOUNDARY
 - - - DRAINAGE BREAKLINE / DIVIDE
 - PROPOSED DIRECTION OF SURFACE DRAINAGE
 - PROPOSED OVERLAND FLOW ROUTE
 - ORIGINAL TOPOGRAPHICAL CONTOUR (M.A.S.L.)
 - 403.00 APPROX. PROPOSED ROAD CENTRELINE GRADE
 - 402.50 APPROX. PROPOSED GRADE
 - 402.50 EXIST. EXISTING GRADE
 - EXISTING STORM SEWER
 - PROPOSED STORM SEWER (3000 - 10500)

CONCEPTUAL
GRADING AND
SERVICING PLAN

Figure No. 4



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HOUSES ON BLOCKS 1-3 TO BE LOCATED AT THE MIN. FRONT YARD SET-BACK AND ALL ROOF LEADERS TO DRAIN TO THE FRONT YARD

anticipated that some storm sewers may need to be installed at relatively steep grades, however the maximum allowed full-flow velocities will not be exceeded. Major design storm flows (in excess of the 5-year design storm event) will be conveyed overland and on road surfaces to the stormwater management facility.

Further details regarding stormwater management are discussed in section 3.5. Figure No. 4, Conceptual Grading and Servicing Plan, shows the proposed storm sewer layout.

3.5 Stormwater Management Design

3.5.1 Identification of Drainage Outlets

Based on the existing drainage condition, three (3) drainage outlets were identified as receiving surface runoff from the proposed development area: the easterly, northwesterly and northeasterly outlets.

Easterly Outlet – The easterly outlet represents the confluence of the existing ditch along the east side of South Mill Street (south of George Street), and the existing ditch originating to the south of the subject property and flowing northerly to the un-opened Wellesley Street right-of-way. The confluence is located within the unopened Wellesley Street right-of-way to the east of its intersection with South Mill Street. Downstream of this outlet exists a relatively large private pond located within the 35 South Mill Street property.

Northwesterly Outlet – The northwesterly outlet represents a tributary of the Conestoga River located within the westerly adjacent property (7832 Wellington Road 45). Runoff drains to the northwesterly outlet mostly via overland flow to the westerly adjacent property. Runoff is also believed to drain to the northwesterly outlet via an existing catch basin located in the northwesterly adjacent property (7822 Wellington Road 45) which drains westerly as piped flow.

Northeasterly Outlet – The northeasterly outlet represents the existing storm sewer system located along Wellington Road 45. Runoff draining to the adjacent property to the northeast (7 Centre Street) is expected to drain to the easterly side of the Centre Street right-of-way and be conveyed northerly to the northeasterly outlet.

3.5.2 Stormwater Management Criteria

Based on the existing drainage condition and the requirements of the Township of Mapleton, the SWM criteria used to develop the appropriate SWM approach for the proposed development are as follows.:

- a) Post-development peak flow rates discharging from the site to the easterly, northwesterly and northeasterly outlets are to be attenuated to less than, or equal to, existing condition peak flow rates for all design storm events up to, and including, the 100-year design storm event.
- b) Enhanced water quality treatment (80% of TSS removal) is to be provided for runoff draining from the subject property prior to discharging from it.
- c) The annual runoff volume draining from the site to the easterly outlet under existing condition is to be maintained under post-development condition.

The most recent Mount Forest Intensity-Duration-Frequency (IDF) Curves were used to complete the 2, 5, 10, 25, 50, 100-year design storm analysis. A copy of the calculations for the existing condition and post-development condition analysis using MIDUSS for hydrologic modelling has been provided in Appendix "A".

The Mount Forest Chicago Distribution rainfall parameters for the 2, 5, 10, 25, 50, and 100-year design storm events, were calculated as follows:

$$I = A / (t+B)^C$$

Where t is the time of concentration, and A, B, and C are defined in the table below.

Table No. 1: Mount Forest Chicago Distribution Rainfall Parameters

Return Period (Year)	A	B	C
2	774.66	7.160	0.8166
5	1012.69	8.094	0.8196
10	1175.42	8.674	0.8215
25	1381.40	9.174	0.8235
50	1535.38	9.533	0.8247
100	1702.25	9.944	0.8269

The Regional Storm (Hurricane Hazel) was also modelled for post-development condition only to demonstrate that site runoff can be sufficiently conveyed to and through the proposed SWM facility.

MIDUSS was used to complete the stormwater management system design and analysis, for both the existing and post-development condition.

3.5.3 Existing Condition Drainage Areas

For the existing condition analysis, the site was analyzed as four (4) drainage catchments (see Figure No. 5, Existing Condition Storm Catchment Areas).

Catchment 10 (2.32-hectares, 0% impervious) represents the majority of the site and the portion of the subject property draining to the easterly outlet. Under existing condition, runoff generated from Catchment 10 generally sheet flows overland to the easterly outlet.

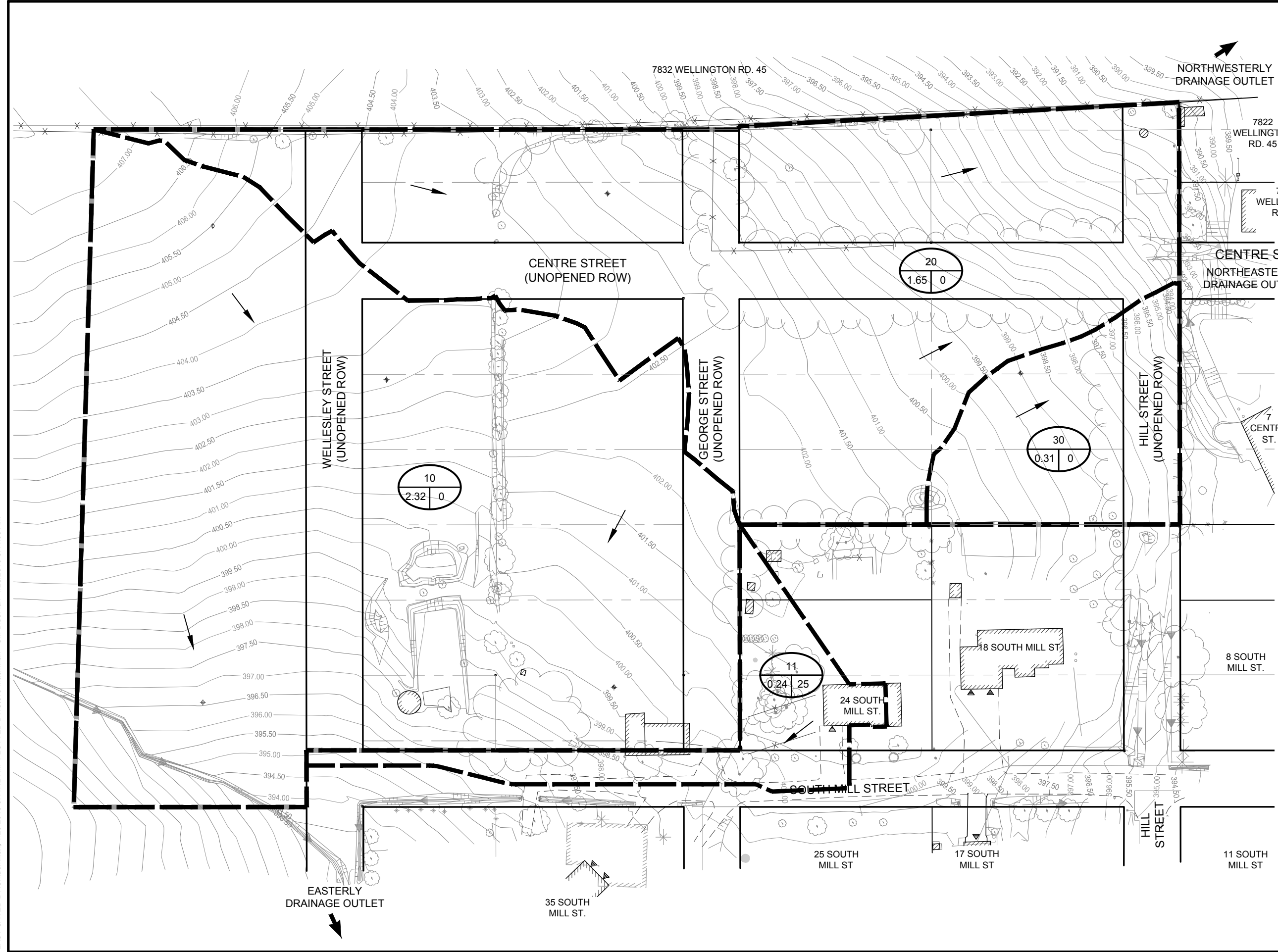
Catchment 11 (0.24-hectares, 25% impervious) represents off-site lands located along South Mill Street and a portion of the property located at 24 South Mill Street that are expected to drain to the proposed SWM facility under post-development condition. Under existing condition, runoff generated from Catchment 11 drains overland to the easterly outlet.

Catchment 20 (1.65-hectares, 0% impervious) represents the northwesterly portion of the subject property. Under existing condition, runoff generated from Catchment 20 drains to the northwesterly outlet via a combination of overland sheet flow and piped flow via the existing catch basin located within the northwesterly adjacent property.

Catchment 30 (0.31-hectares, 0% impervious) represents the northeasterly portion of the subject property. Under existing condition, runoff generated from Catchment 30 generally sheet flows overland to the northeasterly adjacent property (7 Centre Street) and Centre Street, draining ultimately to the northeasterly outlet.

The peak flow rates draining from the site to the easterly, northwesterly and northeasterly outlets under existing condition are summarized in Table No. 2 and represent the allowable peak flow rates draining from the site to the respective outlets under post-development condition.

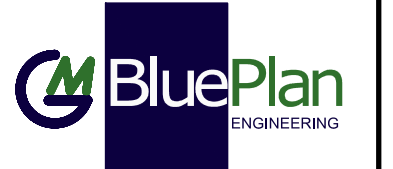
Functional Servicing
and Stormwater
Management Report
Glen Allan Subdivision
Township of Mapleton



- LEGEND:**
- SITE BOUNDARY
 - DRAINAGE AREA BOUNDARY
 - CATCHMENT NUMBER
 - % IMPERVIOUS
 - CATCHMENT AREA IN HECTARES
 - ORIGINAL TOPOGRAPHICAL CONTOUR (M.A.S.L.)
 - DIRECTION OF SURFACE DRAINAGE

EXISTING CONDITION
STORM CATCHMENT
AREAS

Figure No. 5



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Table No. 2: Existing Condition/ Allowable Flow Rates

Outlet	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Easterly Outlet – Catch. 10, 11 (m ³ /s)	0.032	0.062	0.087	0.125	0.155	0.188
Northwesterly Outlet – Catch. 20 (m ³ /s)	0.019	0.037	0.052	0.074	0.093	0.114
Northeasterly Outlet – Catch. 30 (m ³ /s)	0.005	0.010	0.013	0.019	0.023	0.028

3.5.4 Post-Development Condition Drainage Areas

For the post-development condition analysis, the site was analyzed as four (4) drainage catchments (see Figure No. 6, Post-Development Condition Drainage Area Plan).

Catchment 100 (3.08-hectares, 20% impervious) represents the majority of the site and the portion of the subject property draining to the proposed SWM facility. Under post-development condition, runoff generated from Catchment 100 will drain to the proposed SWM facility via a combination of overland flow and piped flow via the proposed storm sewer system, ultimately draining to the easterly outlet via the SWM facility.

Catchment 101 (0.24-hectares, 35% impervious) represents off-site lands located along South Mill Street and a portion of the property located at 24 South Mill Street expected to drain to the proposed SWM facility. Under post-development condition, runoff generated from Catchment 101 drains to the proposed SWM facility via a combination of overland flow and piped flow via the proposed storm sewer system, ultimately draining to the easterly outlet via the SWM facility.

Catchment 200 (0.98-hectares, 10% impervious) represents the northwesterly portion of the subject property including a portion of the future cul-de-sac planned by the Township at the end of Centre Street. Under post-development condition, runoff generated from Catchment 200 drains to the northwesterly outlet via a combination of overland sheet flow and piped flow via the existing catch basin located within the northwesterly adjacent property. A 10% imperviousness is assumed for Catchment 200 in consideration of the future cul-de-sac; approximately corresponding to a 15 m-radius cul-de-sac.

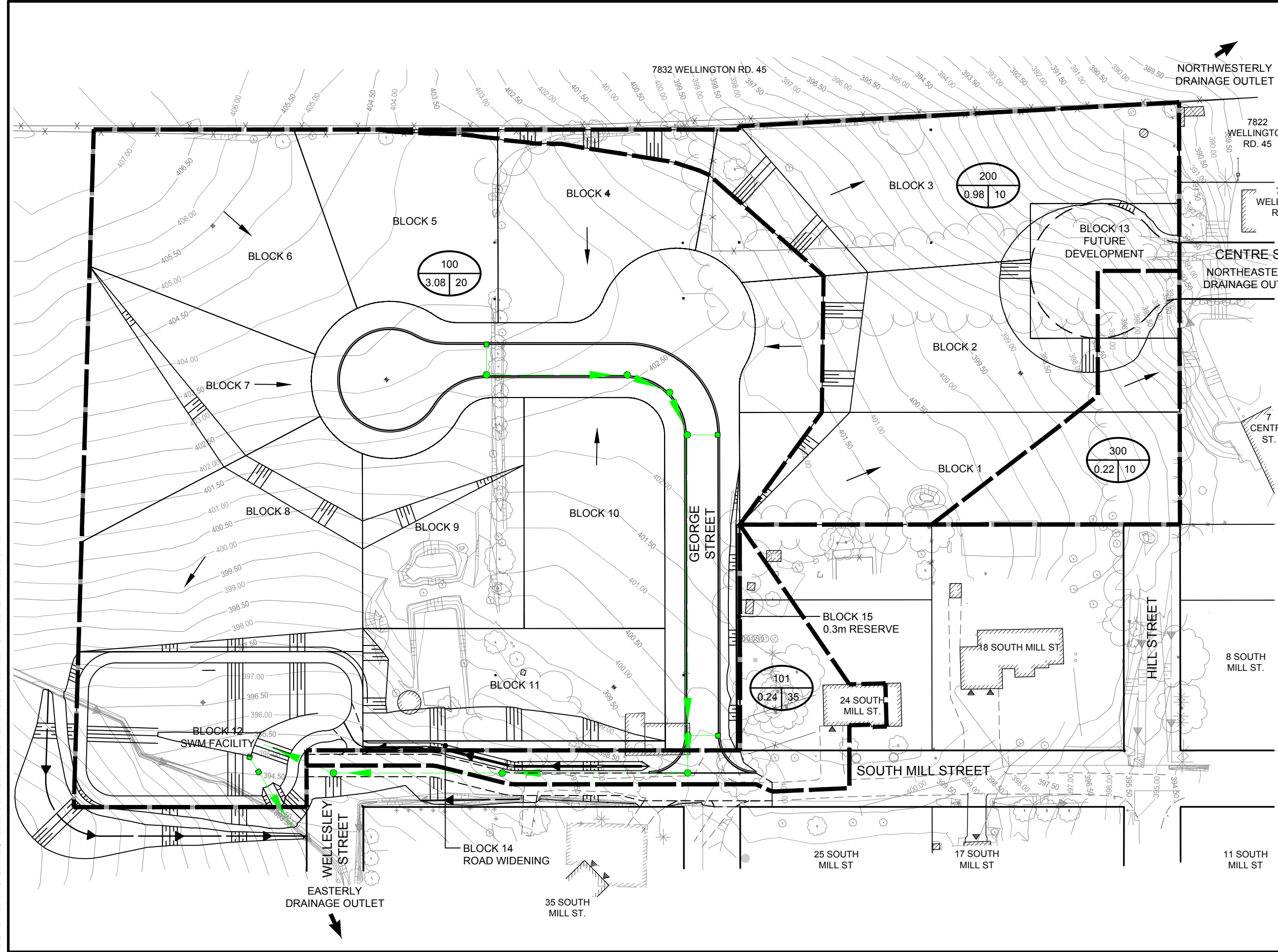
Catchment 300 (0.22-hectares, 10% impervious) represents the northeasterly portion of the subject property including a portion of the future cul-de-sac planned by the Township at the end of Centre Street. Under existing condition, runoff generated from Catchment 300 generally sheet flows overland to the northeasterly adjacent property (7 Centre Street) and Centre Street, draining ultimately to the northeasterly outlet. A 10% imperviousness is assumed for Catchment 300 in consideration of the future construction of a cul-de-sac at the end of Centre Street by the Township.

3.5.5 Stormwater Management System Details

Stormwater runoff discharging from Catchments 100 and 101 will be attenuated via the proposed SWM facility, which has been designed as a dry pond type facility. Flows discharging from the proposed SWM facility will be attenuated via two (2) outlet control structures and one (1) 3 m-wide overflow weir.

Stormwater quality control for the subject property is proposed to be provided primarily by an oil grit separator (OGS) unit installed in-line with the proposed storm sewer system at the intersection of South Mill Street and George Street. The approximately 2.03 ha portion of the subject property expected to drain to the OGS unit, as shown in Figure No. 7, Tributary Area of OGS Unit, has an imperviousness of approximately 20% which generally includes portions of rooftop surfaces within Catchment 100, all proposed driveway surfaces and the proposed George Street roadway. The proposed OGS unit will be sized to provide an Enhanced level of water quality treatment (80% TSS removal) for 90% of the annual runoff draining from the above-described tributary

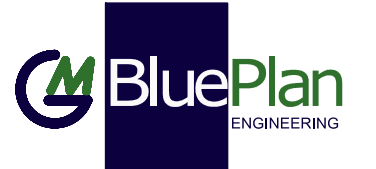
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 - DIRECTION OF SURFACE DRAINAGE

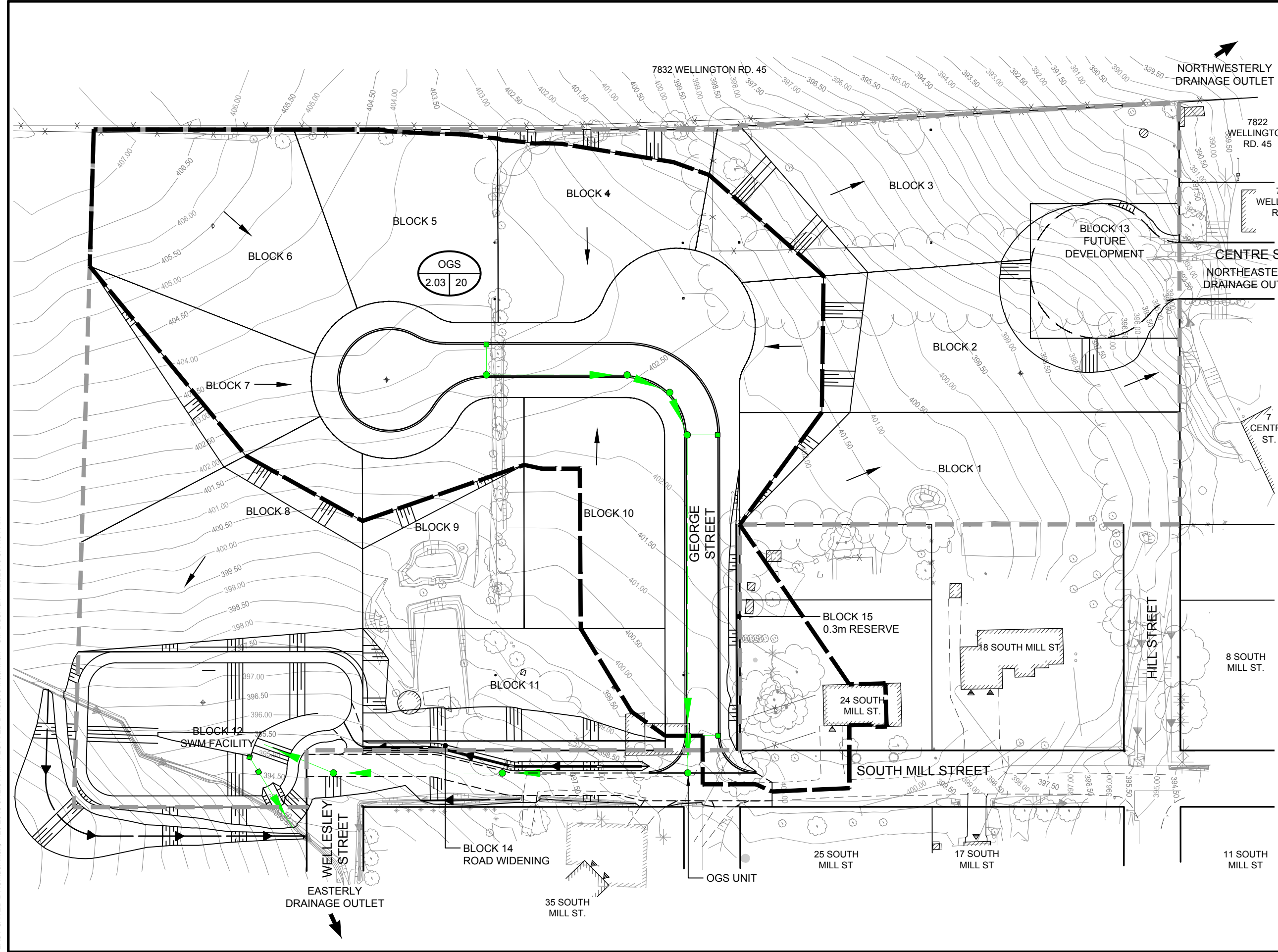
POST-DEVELOPMENT
CONDITION
STORM CATCHMENT
AREAS

Figure No. 6



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Functional Servicing
and Stormwater
Management Report
Glen Allan Subdivision
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LEGEND:

- SITE BOUNDARY
- DRAINAGE AREA BOUNDARY
- CATCHMENT NUMBER
% IMPERVIOUS
- CATCHMENT AREA
IN HECTARES
- ORIGINAL TOPOGRAPHICAL
CONTOUR (M.A.S.L.)
- DIRECTION OF
SURFACE DRAINAGE

TRIBUTARY
AREA OF
OGS UNIT
Figure No. 7



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area for a fine particle distribution. The Hydro International FD-6HC unit is sufficiently sized to achieve these quality control objectives and its supporting sizing calculations are included in Appendix 'A'.

Within the subject property, the impervious surfaces not expected to drain to the OGS unit consist of portions of proposed rooftop surfaces. Runoff draining from these rooftop surfaces is generally considered to be "clean and receives further filtration as it is conveyed overland to the SWM facility.

As an overland flow route for a majority of the site, the capacity of the proposed westerly roadside swale along South Mill Street must be considered in relation to expected major peak flows. From the MIDUSS modelling included in Appendix A, the 100-year peak flow draining from Catchments 100 and 101 is expected to be approximately 0.404 m³/s prior to attenuation by the proposed SWM facility; during a Regional storm, an approximately 0.390 m³/s peak flow is expected prior to attenuation. The proposed roadside swale is conservatively considered to provide sufficient capacity to convey the entirety of this approximately 0.404 m³/s 100-year peak flow. With the minimum slope of the roadside swale proposed at 0.5%, a minimum cross-sectional flow area of 0.48 m² is proposed to ensure a sufficient capacity of approximately 0.409 m³/s (assuming Manning's n = 0.027 for a maintained grass swale).

Along the westerly side of South Mill Street, south of George Street, the minimum width between the edge of the proposed extension of the South Mill Street roadway and the subject property's easterly boundary is approximately 3.5 m. Assuming that the top of the roadside swale's bank is setback 1 m from the edge of the roadway, a swale having a 0.5 m bottom width, a 0.33 m depth, and 3:1 side slopes is expected to fit within the minimum available width (2.5 m swale top width + 1 m setback from road). These minimum swale dimensions yield a flow area of approximately 0.49 m² and a resulting capacity of approximately 0.426 m³/s.

Under realistic conditions, a portion of this 0.404 m³/s flow would be expected to drain as piped flow via the proposed storm sewer system and not as overland flow. Additionally, the entirety of Catchment 100 is not expected to drain via this overland flow route as a portion of the peak flow would drain overland to the SWM facility as sheet flow. Therefore, the proposed minimum 0.48 m² cross-sectional flow area of the proposed roadside swale is expected to provide sufficient capacity as an overland flow and is considered viable given the minimum available width along South Mill Street.

3.5.6 Routing

The hydrologic model MIDUSS was used to create the design storm runoff hydrographs and to route the hydrographs. A copy of the final printout of the hydrologic modelling is appended in Appendix "A".

The results of the routing analysis are as follows:

Table No. 3: Stormwater Management Facility - Stage/Storage/Discharge Capacities

	Available Capacity			Actual Capacity Used		
	Peak Flow (m ³ /s)	Storage Volume (m ³)	Storage Elevation (m)	Peak Flow (m ³ /s)	Storage Volume (m ³)	Storage Elevation (m)
Bottom of Pond & T/G Primary Outlet Control Structure (100 mm Ø orifice)	0	0	394.00	---	---	---
2-Year	---	---	---	0.024	198	394.94
T/G Secondary Outlet Control Structure (150 mm Ø orifice)	0.024	204	394.95	---	---	---
5-Year	---	---	---	0.057	275	395.10
10-Year	---	---	---	0.060	360	395.26
25-Year	---	---	---	0.063	486	395.45
50-Year	---	---	---	0.066	596	395.60
Overflow Weir	0.066	600	395.60	---	---	---
100-Year	---	---	---	0.122	640	395.65
Regional	---	---	---	0.364	734	395.76
Top of Bank	0.493	766	395.79	---	---	---

Table No. 4: Post-Development Condition Flow Rates

Outlet	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	Regional
Easterly Outlet – Catch. 100, 101 (controlled; m ³ /s)	0.024	0.057	0.060	0.063	0.066	0.122	0.364
Northwesterly Outlet – Catch. 200 (uncontrolled; m ³ /s)	0.016	0.031	0.041	0.058	0.072	0.086	0.106
Northeasterly Outlet – Catch. 300 (uncontrolled; m ³ /s)	0.005	0.009	0.013	0.018	0.021	0.025	0.025

The following table compares the post-development condition flow rates to the allowable release rates for the full range of design storm events.

Table No. 5: Comparison of Allowable Release Rates and Post-Development Condition Flow Rates

Outlet	Condition	2-Yr.	5-Yr.	10-Yr.	25-Yr.	50-Yr.	100-Yr.	Reg.
Easterly Outlet	Allowable Release Rates (m ³ /s)	0.032	0.062	0.087	0.125	0.155	0.188	N/A
	Post-Development (m ³ /s)	0.024	0.057	0.060	0.063	0.066	0.122	0.364
Northwesterly Outlet	Allowable Release Rates (m ³ /s)	0.019	0.037	0.052	0.074	0.093	0.114	N/A
	Post-Development (m ³ /s)	0.016	0.031	0.041	0.058	0.072	0.086	0.106
Northeasterly Outlet	Allowable Release Rates (m ³ /s)	0.005	0.010	0.013	0.019	0.023	0.028	N/A
	Post-Development (m ³ /s)	0.005	0.009	0.013	0.018	0.021	0.025	0.025

Therefore, the post-development peak flow rates from the site to the easterly, northwesterly and northeasterly outlets are expected to be attenuated to less than, or equal to, the allowable release rates for all design storm events up to, and including, the 100-year design storm event.

3.5.7 Maintenance Plan

To ensure that the stormwater management system continues to function as designed and constructed, we recommend that the following inspections and maintenance activities be completed on an annual basis:

1. Is there any noticeable damage to the structures (i.e. outlet structures, overflow weirs)? If yes, complete any necessary repairs and/or installation of replacement structures.
2. Is there any noticeable damage to the asphalt/gravel and grassed swales (i.e. erosion, blockages)? If yes, complete any necessary repairs.
3. Is there any indication of a spill (i.e. frothy water, oily sheen on the water)? If yes, investigate, inform the appropriate agencies and complete the necessary clean-up and restoration.
4. Inspect all catch basins, and manholes. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).
5. Inspect all swales and overflow locations. Remove and dispose of any accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.).

Please note that any structures identified during the annual inspection to be worn, missing or damaged are to be repaired or replaced within 48 hours.

3.5.8 Annual Runoff Volumes Draining to the Easterly Outlet

A water balance analysis was completed for lands draining to the easterly outlet to confirm that the existing condition annual runoff volume draining to it is maintained in post-development condition. From the proposed development area, Catchments 10 and 11, and Catchments 100 and 101 are considered to contribute runoff volume to the easterly outlet under existing and post-development conditions, respectively. In addition, with the use of the Ontario Flow Assessment Tool provided by the Ministry of Natural Resources and Forestry, an approximately 6.24 ha portion of southerly off-site lands is expected to drain to the easterly outlet in addition to portions of the proposed development site.

With reference to values provided in Table 3.1 of the Stormwater Management Planning and Design (SWMPD) Guide, published by the Ministry of Environment, Conservation and Parks, existing condition ground cover of the on-site and off-site lands draining to the easterly outlet are considered to be “moderately rooted crops”. Under post-development condition, on-site lands are considered to be “urban lawns” and off-site lands are considered to be maintained as “moderately rooted crops”. The soil type within the subject property is generally characterized as Huron loam, as per the Wellington County Soils Map (Ontario Soil Survey Report No. 35) published by the Department of Agriculture. Huron loam is known to be of the Hydrological Soil Group (HSG) BC. Therefore, the average of the values provided for an HSG B and C soil in Table 3.1 of the SWMPD Guide is considered for analysis purposes.

The values and methodology outlined by the SWMPD Guide provide the runoff volume expected to drain from a given area generally without consideration of possible runoff volume reduction via conveyance measures.

Under post-development condition, the majority of the proposed surface grading within the portion of the site proposed to drain to the easterly outlet meets the requirements of a “vegetated filter strip” as defined by the Low Impact Development Stormwater Management Planning and Design (LIDSWMP) Guide published by the Credit Valley and Toronto and Region Conservation Authorities. From the LIDSWMP Guide, “a conservative runoff reduction rate for vegetated filter strips is 25% for HSG C and D soils and 50% for HSG A and B soils”. As an HSG BC soil, Huron loam is considered to provide a conservative runoff reduction rate of 37.5%. Under post-development condition, approximately 3.01 ha of the site is expected to be conveyed by vegetated filter strips prior to discharging to the easterly outlet.

In consideration of the above discussion, the water balance calculations were completed and are attached in Appendix “A” and as summarized in Table No. 6 below.

Table No. 6: Water Balance Analysis Results

Outlet	Hydrological Component	Annual Existing Volume (m ³)	Annual Post-Development Volume (m ³)	Annual Net Change (m ³)	Annual Net Change as a Percentage of Ex. Condition
Easterly Outlet	Runoff	27,616	28,784	+ 1,168	+ 4%
	Recharge	7,691	7,640	- 52	- 1%
<p><i>Note: The sum of existing and post-development annual recharge and runoff volumes are not equal as evapotranspiration is not shown.</i></p>					

From Table No. 6, for the portion of on-site and off-site lands expected to drain to the easterly outlet, it is expected that there will be an increase of approximately 4%, or 1,168 m³, in annual runoff volume under post-development condition from existing condition. Therefore, a minor increase in the annual runoff volume draining to the easterly outlet is expected which is considered to be relatively negligible given that this volume drains over an annual basis.

3.6 Roadways

George Street will be constructed in general conformance with the Township’s typical road cross section as shown in STD.DWG. No 101 of the Township’s Standards, including a 20.0m right-of-way, 8.5m wide asphalt carriageway, 13.5m asphalt radius within the cul-de-sac and mountable curb and gutter as per OPSD 600.100. The 8.5m wide asphalt roadway will provide enough room to accommodate parking along one side the street. Typical road grades are anticipated to range from 0.65% to 4.0% with a standard roadway cross fall of 2.0%.

South Mill Street, south of the intersection of George Street, will be reconstructed to match existing conditions as a 6.0m wide gravel roadway. This gravel roadway will act as a maintenance access road to the stormwater management facility as well as the major overland flow route for storms in excess of the 5-year design storm. Due to the existing topography typical road grades are anticipated to range from 0.5% to 6.5% with a standard roadway cross fall of 2.0%. The existing ditch on the east side of the street is to be maintained.

A Future Development Block (Block 13) is shown at the end of Centre Street to allow for the future construction of a cul-de-sac at the end of Centre Street by the Township. The existing topography and vegetation in this area may make construction difficult.

3.7 Utilities and Lighting

Local utility providers include Hydro One Networks, and Bell Canada. Both were contacted for comment regarding the general location and capacity / size of their existing plant at the limits or boundaries of this development and / or the general vicinity, and to provide preliminary comments regarding their desire to service this subdivision and any anticipated opportunities or constraints associated with such servicing.

Hydro One Networks confirmed that there is 3-phase (8.32kV) and 1-phase (4.8kV) power readily available to supply this development and that they don't foresee any issues regarding the station and feeder capacity. That being said, this information is based on the current condition of the distribution system, and can change without notice, so a thorough study will be completed once this development has officially been put into the process.

Bell Canada has existing copper plant along South Mill Street, at the intersection of South Mill Street and Hill Street. Since Bell doesn't place copper cable in new subdivisions any more, it would be fibre to the home or nothing. Bell's process for servicing would be later, following the Draft plan application, and as detailed design proceeds. Bell would then submit a Greenfield Template to the Bell Governance Board to seek approval. Their decision at that time would be to either serve, to not serve, or to serve with a contribution from the Developer. Bell also made note of another small development in a rural community where because the Developer's contribution to install fibre to the home was so high the Developer declined Bell and the subdivision proceeded with no wireline telecom provider.

The Township of Mapleton Development Standards include provisions for street lighting on all new internal streets, at the Developer's cost, which would be coordinated with Hydro One. The Township indicated that in a rural setting the street lighting may not need to be to full urban standards as long as there is sufficient / safe lighting at intersections and dead ends.

All lighting and utility infrastructure located within the cul-de-sac is to maintain a constant offset from the curb, similar to the straight section of road, and not protrude into the street to ensure they are not struck by fire vehicles.

3.8 Sediment and Erosion Control

A silt fence will generally be installed along the property boundary and along any environmentally sensitive buffers/areas of the site. The silt fence will serve to minimize the opportunity for water borne sediments to be transported from the site to the adjacent properties. Temporary straw bale check dams will be installed in swales after the initial grading has been completed to slow flow rates and promote the settlement of water borne sediments before they reach the silt fence.

Upon completion of area grading, any area not subject to active construction within an agreed timeframe will be topsoiled and seeded as per OPSS 570 and 572.

Inspection and maintenance of all silt fencing and straw bale check dams will start after installation is complete. The silt fence and check dams will be inspected on a weekly basis during active construction or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the features found to need repair.

Once construction has been substantially completed and the ground surface is stabilized, the silt fence and straw bale check dams will be removed, any accumulated sediment within the limits of the development will be collected and removed and the site restoration will be completed.

After construction of the development, erosion and sediment transport will be minimal.

4. SUMMARY

In our opinion, the foregoing adequately demonstrates that development of the subject property as an urban residential subdivision serviced with private drilled wells, private on-site sewage treatment facilities, municipal storm sewers, a storm water management facility and utility services is achievable.

All of which is respectfully submitted.

GM BLUEPLAN ENGINEERING LIMITED

Per:

GM BLUEPLAN ENGINEERING LIMITED

Per:

Ian Eriksen, P.Eng.

Project Engineer

Steven MacMillan, C.E.T.

Senior Technical Specialist

**APPENDIX A:
STORMWATER MANAGEMENT ANALYSIS**

MIDUSS Output
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Job's used: \\os-2k8\users-private\awilkinson\Documents\le METRIC
Job folder: MIDUSS\317033
Output filename: 317033 - Pre 2 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Company
Date & Time last used: 1/7/2019 at 6:42:19 PM
TIME PARAMETERS
31 10.000 Max. Storm length"
360.000 Max. Hydrograph"
2400.000 Max. Chicago storm"
32 774.890 Coefficient A"
0.160 Constant B"
0.817 Exponent C"
0.375 Fraction R"
360.000 Duration
1.000 Time step multiplier"
74.888 mm/hr"
37.317 mm"
6 002hyd Hydrograph extension used in this file"
33 CATCHMENT 10"
1 Triangular SCS"
1 Equal length"
1 SCS method"
1 Southerly Portion of Subject Property"
% Impervious"
0.000 Total Area"
2.320 Flow length"
7.500 Overland Slope"
2.320 Pervious Area"
160.000 Pervious length"
7.500 Pervious slope"
0.000 Impervious Area"
160.000 Impervious length"
7.500 Impervious slope"
0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
0.239 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
7.164 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"

5.000 Overland slope"
0.180 Pervious Area"
10.000 Pervious length"
5.000 Pervious slope"
0.060 Impervious Area"
10.000 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
0.238 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
7.164 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
CATCHMENT 11
Surface Area Pervious Total Area "
0.180 0.060 0.240 hectare"
Time of concentration 10.572 1.012 5.576 minutes"
Rainfall depth 204.984 169.794 186.594 mm"
Rainfall volume 37.317 37.317 37.317 mm"
Rainfall losses 67.17 22.39 89.56 mm"
Runoff depth 28.427 8.125 23.352 mm"
Runoff volume 8.889 29.192 13.965 mm"
Runoff coefficient 16.00 17.52 33.52 mm"
Maximum flow 0.238 0.782 0.374 C.m"
HYDROGRAPH Add Runoff " 0.005 0.010 0.012 C.m/sec"
4 Add Runoff " 0.012 0.032 0.000
HYDROGRAPH Copy to OutFlow" 0.000 0.000
8 Copy to OutFlow" 0.032 0.032 0.000"
6 Combine " Combine 1"
1 Node #"
Easterly outlet"
Maximum flow volume 0.032 C.m/sec"
Hydrograph volume 240.536 C.m"
0.012 0.032 0.032"
2 Start - New Tributary"
0.012 0.000 0.032 0.032"
33 CATCHMENT 20"
1 Triangular SCS"
1 Equal length"
1 SCS method"
20 Northwesterly Portion of Subject Property"
% Impervious"
0.000 Total Area"
200.000 Flow length"
7.500 Overland Slope"
1.650 Pervious Area"
200.000 Pervious length"
7.500 Pervious slope"
0.000 Impervious Area"
200.000 Impervious length"
7.500 Impervious slope"
0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
0.239 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
7.164 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
CATCHMENT 20
Surface Area Pervious Total Area "
0.000 0.000 0.032 C.m/sec"
1.650 1.650 1.650 hectare"
0.032 0.032 0.032 C.m/sec"

317033 - Pre 2 yr - Jan19
 Hydrograph volume 56,484
 START/RE-START TOTALS 30" 27.648
 Runoff Totals on EXIT" 0.005
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT" hectare"

317033 - Pre 2 yr - Jan19
 Time of concentration 56.485
 Time to Centroid 265.130
 Rainfall depth 37.317
 Rainfall volume 615.773
 Rainfall losses 28.393
 Runoff depth 8.924
 Runoff volume 147.25
 Runoff coefficient 0.239
 Maximum flow 0.000
 Maximum flow Add Runoff " 0.019
 HYDROGRAPH Add Runoff " 0.019
 Add Runoff " 0.019
 HYDROGRAPH Copy to Outflow" 0.032
 Copy to Outflow" 0.019
 HYDROGRAPH " Combine 2" 0.019
 Combine " 2" 0.019
 Node #"
 Northwesterly Outlet"
 Maximum flow 0.019
 Hydrograph volume 147.248
 HYDROGRAPH Start - New Tributary" 0.016
 Start - New Tributary" 0.019
 Start - New Tributary" 0.019
 CATCHMENT 30"
 Triangular SCS"
 Equal length"
 SCS method"
 30 Northeastly Portion of Subject Property"
 % Impervious"
 Total Area"
 Flow length"
 90.000 Overland Slope"
 8.000 Pervious Area"
 0.310 Pervious length"
 8.000 Pervious slope"
 0.000 Impervious Area"
 90.000 Impervious length"
 8.000 Impervious slope"
 8.000 Pervious Manning "n"
 0.250 Pervious SCS Curve No. "
 0.239 Pervious runoff coefficient"
 7.164 Pervious Ia/S coefficient"
 0.015 Pervious Initial abstraction"
 98.000 Impervious Manning "n"
 0.000 Impervious SCS Curve No. "
 0.000 Impervious runoff coefficient"
 0.100 Impervious Ia/S coefficient"
 0.518 Impervious Initial abstraction"
 Catchment 30 Pervious
 Surface Area 0.310
 Time of concentration 34.312
 Time to Centroid 236.029
 Rainfall depth 37.317
 Rainfall volume 115.68
 Rainfall losses 28.398
 Runoff depth 8.919
 Runoff volume 27.65
 Runoff coefficient 0.239
 Maximum flow 0.000
 Maximum flow Add Runoff " 0.005
 HYDROGRAPH Add " 0.005
 Add Runoff " 0.019
 HYDROGRAPH Copy to Outflow" 0.019
 Copy to Outflow" 0.005
 HYDROGRAPH " Combine 3" 0.005
 Combine " 3" 0.005
 Node #"
 Northwesterly Outlet"
 Maximum flow 0.005

Page 3

317033 - Pre 2 yr - Jan19
 Hydrograph volume 56,484
 START/RE-START TOTALS 30" 27.648
 Runoff Totals on EXIT" 0.005
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT" hectare"

317033 - Pre 2 yr - Jan19
 Time of concentration 56.485
 Time to Centroid 265.130
 Rainfall depth 37.317
 Rainfall volume 615.773
 Rainfall losses 28.393
 Runoff depth 8.924
 Runoff volume 147.25
 Runoff coefficient 0.239
 Maximum flow 0.000
 Maximum flow Add Runoff " 0.019
 HYDROGRAPH Add Runoff " 0.019
 Add Runoff " 0.019
 HYDROGRAPH Copy to Outflow" 0.032
 Copy to Outflow" 0.019
 HYDROGRAPH " Combine 2" 0.019
 Combine " 2" 0.019
 Node #"
 Northwesterly Outlet"
 Maximum flow 0.019
 Hydrograph volume 147.248
 HYDROGRAPH Start - New Tributary" 0.016
 Start - New Tributary" 0.019
 Start - New Tributary" 0.019
 CATCHMENT 30"
 Triangular SCS"
 Equal length"
 SCS method"
 30 Northeastly Portion of Subject Property"
 % Impervious"
 Total Area"
 Flow length"
 90.000 Overland Slope"
 8.000 Pervious Area"
 0.310 Pervious length"
 8.000 Pervious slope"
 0.000 Impervious Area"
 90.000 Impervious length"
 8.000 Impervious slope"
 8.000 Pervious Manning "n"
 0.250 Pervious SCS Curve No. "
 0.239 Pervious runoff coefficient"
 7.164 Pervious Ia/S coefficient"
 0.015 Pervious Initial abstraction"
 98.000 Impervious Manning "n"
 0.000 Impervious SCS Curve No. "
 0.000 Impervious runoff coefficient"
 0.100 Impervious Ia/S coefficient"
 0.518 Impervious Initial abstraction"
 Catchment 30 Pervious
 Surface Area 0.310
 Time of concentration 34.312
 Time to Centroid 236.029
 Rainfall depth 37.317
 Rainfall volume 115.68
 Rainfall losses 28.398
 Runoff depth 8.919
 Runoff volume 27.65
 Runoff coefficient 0.239
 Maximum flow 0.000
 Maximum flow Add Runoff " 0.005
 HYDROGRAPH Add " 0.005
 Add Runoff " 0.019
 HYDROGRAPH Copy to Outflow" 0.019
 Copy to Outflow" 0.005
 HYDROGRAPH " Combine 3" 0.005
 Combine " 3" 0.005
 Node #"
 Northwesterly Outlet"
 Maximum flow 0.005

Page 4

MIDUSS Output
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Output filename: 317033 - Pre 5 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Date & Time last used: 1/7/2019 at 6:50:57 PM

TIME PARAMETERS

31 10.000 Time Step
360.000 Max. Storm Length
2400.000 Max. Hydrograph
1 CHICAGO STORM
1012.690 Coefficient A
8.094 Constant B
0.820 Exponent C
0.375 Fraction R
360.000 Duration
1.000 Time step multiplier
93.011 mm/hr
47.926 mm

6 005Hyd Hydrograph extension used in this file

33 CATCHMENT 10
1 Triangular SCS
1 Equal length
1 SCS method
1 Southerly Portion of Subject Property

0.000 Total Area
2.320 Total Area
160.000 Flow Length
7.500 Overland Slope
2.320 Pervious Area
160.000 Pervious length
7.500 Pervious slope
0.000 Impervious Area
160.000 Impervious length
7.500 Impervious slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.308 Pervious Runoff coefficient
7.164 Pervious Ia/S coefficient
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.000 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
0.057 0.000

Catchment 10 Pervious 0.000 c.m./sec
Surface Area 2.320 Total Area
Time of concentration 39.192 39.192
Rainfall depth 240.443 240.443
Rainfall volume 47.926 47.926
Rainfall losses 1111.87 1111.88
Runoff depth 33.168 33.168
Runoff volume 342.38 342.38
Maximum flow 0.308 0.308
HYDROGRAPH Add Runoff 0.057 0.057

4 Add Runoff 0.057 0.000 0.000

CATCHMENT 11

1 Triangular SCS
1 Equal length
1 SCS method
1 Easterly Off-Site Lands
25.000 % Impervious
0.240 Total Area
10.000 Flow Length

5.000 Overland slope
0.180 Pervious Area
10.000 Pervious length
5.000 Pervious slope
0.060 Pervious Area
10.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.308 Pervious Runoff coefficient
7.164 Pervious Ia/S coefficient
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.803 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
0.057 0.000
Catchment 11 Pervious 0.000 c.m./sec
Surface Area 0.180 Total Area
Time of concentration 8.386 8.386
Rainfall depth 199.210 199.210
Rainfall volume 86.27 86.27
Rainfall losses 33.304 33.304
Runoff depth 14.621 14.621
Runoff volume 26.32 26.32
Maximum flow 0.305 0.305
HYDROGRAPH Add Runoff 0.009 0.009

4 Add Runoff 0.017 0.062 0.000

HYDROGRAPH Copy to Outflow 0.000 0.000

8 Copy to Outflow 0.017 0.062 0.000

HYDROGRAPH Combine 1

1 Node #

Easterly Outlet

Maximum flow 0.062 0.062 c.m./sec
Hydrograph volume 391.801 391.801

HYDROGRAPH Start - New Tributary 0.062 0.062

2 Start - New Tributary 0.017 0.000 0.062

CATCHMENT 20

1 Triangular SCS

1 Equal length

1 SCS method

20 Northwesternly Portion of Subject Property

0.000 % Impervious

1.650 Total Area

200.000 Flow Length

7.500 Overland Slope

1.650 Pervious Area

200.000 Pervious length

7.500 Pervious slope

0.000 Impervious Area

200.000 Impervious length

7.500 Impervious slope

0.250 Pervious Manning 'n'

78.000 Pervious SCS Curve No.

0.308 Pervious Runoff coefficient

7.164 Pervious Ia/S coefficient

0.015 Impervious Manning 'n'

98.000 Impervious SCS Curve No.

0.000 Impervious Runoff coefficient

0.100 Impervious Ia/S coefficient

0.518 Impervious Initial abstraction

0.037 0.000 0.062

Catchment 20 Pervious 0.062 c.m./sec
Surface Area 1.650 Total Area
Flow Length 200.000

Impervious Total Area 1.650 hectare
Impervious Total Area 1.650 hectare

317033 - Pre 5 yr - Jan19
 Hydrograph volume 44,807 c.m.
 0.010 0.010
 START/RE-START TOTALS 30"
 3 Runoff Totals on EXIT" 4.520
 Total Catchment area 0.060
 Total Impervious area 1.328"
 Total % Impervious
 EXIT"

317033 - Pre 5 yr - Jan19
 Time of concentration 44,807 minutes"
 Time to Centroid 4,803 minutes"
 Rainfall depth 248,009 mm
 Rainfall volume 47,926 c.m.
 Rainfall losses 790,177 mm
 Runoff depth 33,170 mm
 Runoff volume 14,756 mm
 Runoff coefficient 243.47
 Maximum flow 0.308 c.m./sec
 HYDROGRAPH Add Runoff " 0.037
 4 Add Runoff " 0.037
 HYDROGRAPH Copy to Outflow" 0.062
 8 Copy to Outflow" 0.062
 HYDROGRAPH " Combine 2" 0.037
 6 Combine " Combine 2"
 2 Node #"
 Northwesterly Outlet"
 Maximum flow 0.037 c.m./sec
 Hydrograph volume 243,469 c.m.
 0.037 0.037
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary"
 0.037 0.037
 CATCHMENT 30"
 33 CATCHMENT 30"
 1 Triangular SCS"
 1 Equal length"
 1 SCS method"
 30 Northeastly Portion of Subject Property"
 0.000 % Impervious"
 90.000 Total Area"
 8.000 Flow length"
 8.000 Overland Slope"
 0.310 Pervious Area"
 90.000 Pervious length"
 8.000 Pervious slope"
 0.000 Impervious Area"
 90.000 Impervious length"
 8.000 Impervious slope"
 0.250 Pervious Manning "n"
 78.000 Pervious SCS Curve No."
 0.307 Pervious Runoff coefficient"
 0.100 Pervious Ia/S coefficient"
 7.164 Pervious Initial abstraction"
 0.015 Impervious Manning "n"
 98.000 Impervious SCS Curve No."
 0.000 Impervious Runoff coefficient"
 0.100 Impervious Ia/S coefficient"
 0.518 Impervious Initial abstraction"
 0.010 0.000 0.037
 Catchment 30 Pervious Impervious Total Area "
 Surface Area 0.310 Hectare"
 Time of concentration 27,219 minutes"
 Time to Centroid 2,978 minutes"
 Rainfall depth 224,392 mm
 Rainfall volume 47,926 c.m.
 Rainfall losses 148,57 mm
 Runoff depth 33,190 mm
 Runoff volume 14,735 mm
 Runoff coefficient 45.68 c.m./sec
 Maximum flow 0.307 c.m./sec
 HYDROGRAPH Add Runoff " 0.010
 4 Add Runoff " 0.010
 HYDROGRAPH Copy to Outflow" 0.037
 8 Copy to Outflow" 0.037
 HYDROGRAPH " Combine 3" 0.010
 6 Combine " Combine 3"
 3 Node #"
 Northwesterly Outlet"
 Maximum flow 0.010 c.m./sec

MIDUSS output -----
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Jobs used: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
Job folder: MIDUSS\317033
Output filename: 317033 - Pre 10 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Company gmbp
Date & Time last used: 1/7/2019 at 7:01:57 PM

TIME PARAMETERS

31 10.000 Time Step
360.000 Max. Storm length"
2400.000 Max. Hydrograph"
32 STORM Chicago storm"
1 Chicago storm"
8.674 Coefficient A"
0.872 Constant B"
0.375 Exponent C"
1.000 Duration
Maximum intensity 104.616 mm/hr"
Total depth 54.935 mm"
6 0.010Hyd Hydrograph extension used in this file"

CATCHMENT 10

1 Triangular SCS"
1 Equal length"
1 SCS method"
10 Southerly Portion of Subject Property"
% Impervious"
2.320 Total Area"
160.000 Flow length"
7.500 Overland Slope"
2.320 Pervious Area"
160.000 Pervious length"
7.500 Pervious slope"
0.000 Impervious Area"
7.500 Impervious length"
0.250 Impervious slope"
78.000 Pervious Manning "n"
0.348 Pervious SCS Curve No."
0.100 Pervious Runoff coefficient"
7.164 Pervious Ia/S coefficient"
0.015 Pervious Manning "n"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.080 Pervious
0.080 Total Area"
2.320 Pervious
2.320 Impervious
4.073 Total Area"
35.033 hectare"
Time to centroid 35.033 minutes"
171.373 233.645
54.935 54.935
1274.49 1274.49
5.941 35.833
18.102 48.954
443.16 443.16
0.000 0.348
0.080 0.080
4 Add Runoff 0.080 0.000 0.000

CATCHMENT 11

1 Triangular SCS"
1 Equal length"
1 SCS method"
11 Easterly Off-Site Lands"
% Impervious"
25.000
0.240 Total Area"
10.000 Flow length"

Overland Slope"
Pervious Area"
Pervious length"
Pervious slope"
Impervious Area"
Impervious length"
Impervious slope"
Pervious Manning "n"
Pervious SCS Curve No."
Pervious Runoff coefficient"
Pervious Ia/S coefficient"
Pervious Initial abstraction"
Impervious Manning "n"
Impervious SCS Curve No."
Impervious Runoff coefficient"
Impervious Ia/S coefficient"
Impervious Initial abstraction"
0.021 0.080 0.000 c.m/sec"
0.060 0.240
0.872 4.561
167.676 183.849
54.935 54.935
32.96 131.84
10.323 29.758
44.612 25.177
26.77 60.42
0.812 0.458
0.014 0.021
4 Add Runoff 0.087 0.000 0.000

HYDROGRAPH COPY TO OUTFLOW

8 Copy to Outflow 0.087 0.087 0.000"
HYDROGRAPH " Combine 1"
1 Node #"
Easterly Outlet"
Maximum Flow volume 0.087 c.m/sec"
503.384
0.087 0.087"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary 0.000 0.087 0.087"
0.021 0.000

CATCHMENT 20

1 Triangular SCS"
1 Equal length"
1 SCS method"
20 Northwesterly Portion of Subject Property"
% Impervious"
0.000 Total Area"
1.650 Flow length"
7.500 Overland Slope"
1.650 Pervious Area"
7.500 Pervious length"
7.500 Pervious slope"
0.000 Impervious Area"
7.500 Impervious length"
0.250 Impervious slope"
78.000 Pervious Manning "n"
0.348 Pervious SCS Curve No."
0.100 Pervious Runoff coefficient"
7.164 Pervious Ia/S coefficient"
0.015 Pervious Manning "n"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.052 0.000 0.087 c.m/sec"
0.087 0.087
0.000 0.000
1.650 1.650
0.000 1.650
0.000 1.650
4 Add Runoff 0.087 0.000 0.000

317033 - Pre 10 yr - Jan19
 Hydrograph volume 0.013 c.m.
 START/RE-START TOTALS 30" 0.013
 3 Runoff Totals on EXIT" 0.013
 Total Catchment area 4.520 hectare"
 Total Impervious area 0.060 hectare"
 Total % impervious 1.328"
 EXIT"

317033 - Pre 10 yr - Jan19
 Hydrograph volume 0.013 c.m.
 START/RE-START TOTALS 30" 0.013
 3 Runoff Totals on EXIT" 0.013
 Total Catchment area 4.520 hectare"
 Total Impervious area 0.060 hectare"
 Total % impervious 1.328"
 EXIT"

317033 - Pre 10 yr - Jan19
 Hydrograph volume 0.013 c.m.
 START/RE-START TOTALS 30" 0.013
 3 Runoff Totals on EXIT" 0.013
 Total Catchment area 4.520 hectare"
 Total Impervious area 0.060 hectare"
 Total % impervious 1.328"
 EXIT"

40	Time of concentration	40.052	minutes"
40	Time to Centroid	240.513	minutes"
40	Rainfall depth	54.935	mm."
40	Rainfall volume	906.442	C.m."
40	Rainfall losses	35.838	mm."
40	Runoff depth	19.097	mm."
40	Runoff volume	315.09	C.m."
40	Runoff coefficient	0.348	
40	Maximum flow	0.000	C.m./sec"
40	HYDROGRAPH Add Runoff "	0.052	
40	4 Add Runoff "	0.052	
40	HYDROGRAPH Copy to Outflow"	0.087	
40	8 Copy to Outflow"	0.052	
40	HYDROGRAPH " Combine 2"	0.087	
40	6 Combine " Combine 2"		
40	2 Node #"		
40	Northwesterly Outlet"		
40	Maximum flow	0.052	C.m./sec"
40	Hydrograph volume	315.093	C.m."
40	0.052	0.052	0.052"
40	HYDROGRAPH Start - New Tributary"		
40	2 Start - New Tributary"	0.052	0.052"
40	0.052	0.052	0.052"
40	CATCHMENT 30"		
40	1 Triangular SCS"		
40	1 Equal length"		
40	1 SCS method"		
40	30 Northeastern Portion of subject Property"		
40	0.000 % Impervious"		
40	0.310 Total Area"		
40	90.000 Flow length"		
40	8.000 Overland Slope"		
40	0.310 Pervious Area"		
40	90.000 Pervious length"		
40	8.000 Pervious slope"		
40	0.000 Pervious Area"		
40	90.000 Impervious length"		
40	8.000 Impervious slope"		
40	0.250 Pervious Manning "n" "		
40	78.000 Pervious SCS Curve No. "		
40	0.347 Pervious Runoff coefficient"		
40	0.100 Pervious Ia/S coefficient"		
40	7.164 Pervious Initial abstraction"		
40	0.015 Impervious Manning "n" "		
40	98.000 Impervious SCS Curve No. "		
40	0.000 Impervious Runoff coefficient"		
40	0.100 Impervious Ia/S coefficient"		
40	0.518 Impervious Initial abstraction"		
40	0.013 Impervious Initial abstraction"		
40	Catchment 30	0.052	C.m./sec"
40	Surface Area	0.310	hectare "
40	Time of concentration	2.829	minutes"
40	Time to Centroid	219.107	minutes"
40	Rainfall depth	54.935	mm."
40	Rainfall volume	170.30	C.m."
40	Rainfall losses	35.857	mm."
40	Runoff depth	19.078	mm."
40	Runoff volume	59.14	C.m."
40	Runoff coefficient	0.347	
40	Maximum flow	0.000	C.m./sec"
40	HYDROGRAPH Add Runoff "	0.013	
40	4 Add Runoff "	0.013	
40	HYDROGRAPH Copy to Outflow"	0.052	
40	8 Copy to Outflow"	0.013	
40	HYDROGRAPH " Combine 3"	0.052	
40	6 Combine " Combine 3"		
40	3 Node #"		
40	Northwesterly Outlet"		
40	Maximum flow	0.013	C.m./sec"

MIDUSS Output -----3
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Jobs Used: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
Job Folder: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
Output filename: 317033 - Pre 25 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Company: gmbp
Date & Time last used: 1/7/2019 at 7:05:29 PM
TIME PARAMETERS

31 10.000 Time Step
360.000 Max. Storm length
2400.000 Max. Hydrograph
STORM Chicago storm
1 Chicago storm
Coefficient A
0.174 Constant B
0.823 Exponent C
0.375 Fraction R
360.000 Duration
1.000 Time step multiplier
Maximum intensity 119.599 mm/hr
Total depth 63.732 mm
6 025hyd Hydrograph extension used in this file
CATCHMENT 10
1 Triangular SCS
1 Equal length
1 SCS method
10 Southerly Portion of Subject Property
% Impervious
2.320 Total Area
160.000 Flow length
7.500 Overland Slope
2.320 Pervious Area
160.000 Pervious length
7.500 Pervious Slope
0.000 Impervious Area
160.000 Impervious length
0.250 Pervious Slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.391 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.000 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 10
Surface Area 2.320 hectare
Time of concentration 31.185 minutes
Time to Centroid 227.186 minutes
Rainfall depth 63.732 mm
Rainfall volume 1478.58 C.m.
Rainfall losses 38.837 mm
Runoff depth 57.610 mm
Runoff volume 277.56 C.m.
Runoff coefficient 0.391 C.m./sec
Maximum flow 0.114
HYDROGRAPH Add Runoff 0.114
4 Add Runoff 0.114 0.000 0.000

CATCHMENT 20
1 Triangular SCS
1 Equal length
1 SCS method
20 Northwesterly Portion of Subject Property
% Impervious
1.650 Total Area
200.000 Flow length
7.500 Overland Slope
1.650 Pervious Area
200.000 Pervious length
7.500 Pervious Slope
0.000 Impervious Area
200.000 Impervious length
7.500 Impervious Slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.391 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.000 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 11
Surface Area 0.180 hectare
Time of concentration 6.673 minutes
Time to Centroid 193.860 minutes
Rainfall depth 63.732 mm
Rainfall volume 114.72 C.m.
Rainfall losses 39.317 mm
Runoff depth 24.414 mm
Runoff volume 43.95 C.m.
Runoff coefficient 0.383 C.m./sec
Maximum flow 0.016
HYDROGRAPH Add Runoff 0.016
4 Add Runoff 0.125 0.125 0.000

HYDROGRAPH Copy to Outflow
8 Copy to Outflow 0.125 0.125 0.000
HYDROGRAPH Combine 1
6 Combine 1
1 Node #
Easterly Outlet
Maximum flow 0.125 C.m./sec
Hydrograph volume 652.851 C.m.
0.027 0.125 0.125 0.125
HYDROGRAPH Start - New Tributary
2 Start - New Tributary 0.027 0.000 0.125 0.125

CATCHMENT 20
1 Triangular SCS
1 Equal length
1 SCS method
20 Northwesterly Portion of Subject Property
% Impervious
1.650 Total Area
200.000 Flow length
7.500 Overland Slope
1.650 Pervious Area
200.000 Pervious length
7.500 Pervious Slope
0.000 Impervious Area
200.000 Impervious length
7.500 Impervious Slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.391 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.000 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 20
Surface Area 1.650 hectare
Time of concentration 6.673 minutes
Time to Centroid 193.860 minutes
Rainfall depth 63.732 mm
Rainfall volume 114.72 C.m.
Rainfall losses 39.317 mm
Runoff depth 24.414 mm
Runoff volume 43.95 C.m.
Runoff coefficient 0.383 C.m./sec
Maximum flow 0.017
HYDROGRAPH Add Runoff 0.017
4 Add Runoff 0.125 0.125 0.000

HYDROGRAPH Copy to Outflow
8 Copy to Outflow 0.125 0.125 0.000
HYDROGRAPH Combine 1
6 Combine 1
1 Node #
Easterly Outlet
Maximum flow 0.125 C.m./sec
Hydrograph volume 652.851 C.m.
0.027 0.125 0.125 0.125
HYDROGRAPH Start - New Tributary
2 Start - New Tributary 0.027 0.000 0.125 0.125

MIDUSS Output -----
MIDUSS Version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Units Used: \\\os-2k8\users\private\awilkinson\Documents\MIDUSS\317033
Job Folder: \\os-2k8\users\private\awilkinson\Documents\MIDUSS\317033
Output filename: 317033 - Pre 50 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Company: gmbp
Date & Time last used: 1/7/2019 at 7:07:44 PM
TIME PARAMETERS
31 10.000 Time Step
360.000 Max. Storm length
2400.000 Max. Hydrograph
32 STORM Chicago storm
1 Chicago storm
1535.380 Coefficient A
9.533 Constant B
0.825 Exponent C
0.375 Fraction R
360.000 Duration
1.000 Time step multiplier
Maximum intensity 130.451 mm/hr
70.279 mm
6 OSOHyd Hydrograph extension used in this file
33 CATCHMENT 10
1 Triangular SCS
1 Equal length
1 SCS method
10 Southerly Portion of Subject Property
% Impervious
0.000 Total Area
2.320 Flow length
160.000 Overland Slope
2.320 Pervious Area
160.000 Pervious length
7.500 Pervious slope
0.000 Impervious area
160.000 Impervious length
7.500 Impervious slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.420 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.000 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

5.000 Overland Slope
0.180 Pervious Area
10.000 Pervious length
5.000 Pervious slope
10.000 Impervious Area
10.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.412 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.824 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
Catchment 11 Pervious 0.000 c.m/sec
Surface Area 0.180 hectares
Time of Concentration 6.209 minutes
Time to Centroid 182.164 4.042
Rainfall depth 70.279 161.647
Rainfall volume 42.177 70.279
Rainfall losses 41.353 168.167
Runoff depth 28.926 34.114
Runoff volume 52.07 36.185
Runoff coefficient 0.412 86.79
Maximum Flow 0.019 0.824
HYDROGRAPH Add Runoff " 0.018 0.515 c.m/sec
4 Add Runoff " 0.032 0.155 0.000 0.000
8 HYDROGRAPH Copy to Outflow" 0.032 0.155 0.000 0.000
8 Copy to Outflow" 0.032 0.155 0.155 0.000
40 HYROGRAPH " Combine 1"
6 Combine " Combine 1"
1 Node #"
1 Easterly Outlet"
Maximum Flow 0.155 c.m/sec
Hydrograph volume 772.261 c.m
0.032 0.155 0.155 0.155
HYROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.000 0.155 0.155 0.155
33 CATCHMENT 20
1 Triangular SCS"
1 Equal length"
1 SCS method"
20 Northwesterly Portion of Subject Property"
% Impervious"
0.000 Total Area"
1.650 Flow length"
200.000 Overland Slope"
7.500 Pervious Area"
1.650 Pervious length"
7.500 Pervious slope"
0.000 Impervious Area"
200.000 Impervious length"
7.500 Impervious slope"
0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
0.420 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
7.164 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
Catchment 20 Pervious 0.093 0.155 0.155 0.155 c.m/sec
Surface Area 1.650 hectares
Impervious Total Area 0.000 hectare

MIDUSS Output
 MIDUSS version Version 2.25 rev. 475
 MIDUSS created Sunday, February 07, 2010
 Job used: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
 Job folder: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
 Output filename: 317033 - Pre 100 yr - Jan19.out
 License name: Hewlett-Packard Company
 Company: gmbp
 Date & Time last used: 1/7/2019 at 7:08:38 PM

TIME PARAMETERS
 10.000 Time Step
 360.000 Max. Storm length
 2400.000 Max. Hydrograph
 1 STORM Chicago storm
 1702.250 Coefficient A
 0.827 Coefficient B
 0.375 Exponent C
 360.000 Fraction R
 1.000 Duration
 Maximum intensity 141.234 mm/hr
 Total depth 76.839 mm
 6 100hyd Hydrograph extension used in this file
 CATCHMENT 10
 1 Triangular SCS
 1 Equal length
 1 SCS method
 10 Southerly Portion of Subject Property
 0.000 % Impervious
 2.320 Total Area
 160.000 Flow length
 7.500 Overland Slope
 2.320 Pervious Area
 160.000 Pervious length
 7.500 Pervious slope
 0.000 Impervious Area
 160.000 Impervious length
 7.500 Impervious slope
 0.250 Pervious Manning 'n'
 78.000 Pervious SCS Curve No.
 0.446 Pervious Runoff coefficient
 0.100 Pervious Ia/S coefficient
 7.164 Pervious Initial abstraction
 0.015 Impervious Manning 'n'
 98.000 Impervious SCS Curve No.
 0.000 Impervious Runoff coefficient
 0.100 Impervious Ia/S coefficient
 0.518 Impervious Initial abstraction

Catchment 10 Pervious 0.000 c.m./sec
 Surface Area 2.320 hectare
 Time of concentration 27.236 minutes
 Rainfall depth 76.839 mm
 Rainfall volume 1782.67 C.m.
 Rainfall losses 42.579 mm
 Runoff depth 34.260 mm
 Runoff volume 794.84 C.m.
 Runoff coefficient 0.446
 Maximum flow 0.172 c.m./sec
 HYDROGRAPH Add Runoff 0.172
 4 Add Runoff 0.172 0.000 0.000
 CATCHMENT 11
 1 Triangular SCS
 1 Equal length
 1 SCS method
 11 Easterly Off-Site Lands
 25.000 % Impervious
 0.240 Total Area
 10.000 Flow length

5.000 Overland Slope
 0.180 Pervious Area
 10.000 Pervious length
 5.000 Pervious slope
 0.060 Impervious Area
 10.000 Impervious length
 5.000 Impervious slope
 0.250 Pervious Manning 'n'
 78.000 Pervious SCS Curve No.
 0.437 Pervious Runoff coefficient
 0.100 Pervious Ia/S coefficient
 7.164 Pervious Initial abstraction
 0.015 Impervious Manning 'n'
 98.000 Impervious SCS Curve No.
 0.827 Impervious Runoff coefficient
 0.100 Impervious Ia/S coefficient
 0.518 Impervious Initial abstraction

Catchment 11 Pervious 0.000 c.m./sec
 Surface Area 0.180 hectare
 Time of concentration 3.828 minutes
 Rainfall depth 76.839 mm
 Rainfall volume 138.31 C.m.
 Rainfall losses 43.234 mm
 Runoff depth 33.606 mm
 Runoff volume 60.49 C.m.
 Runoff coefficient 0.437
 Maximum flow 0.827 c.m./sec
 HYDROGRAPH Add Runoff 0.020
 4 Add Runoff 0.188 0.000 0.000
 HYDROGRAPH Copy to Outflow 0.000
 8 Copy to Outflow 0.188 0.188 0.000
 HYDROGRAPH 1
 6 Combine Combine 1
 1 Node #
 Easterly outlet
 Maximum flow 0.188 c.m./sec
 Hydrograph volume 893.439 C.m.
 0.037 0.188 0.188 0.188
 HYDROGRAPH Start - New Tributary
 2 Start - New Tributary 0.037 0.000 0.188 0.188
 0.037 0.000 0.188 0.188

CATCHMENT 20
 1 Triangular SCS
 1 Equal length
 1 SCS method
 20 Northwesterly Portion of Subject Property
 0.000 % Impervious
 1.650 Total Area
 200.000 Flow length
 7.500 Overland Slope
 1.650 Pervious Area
 200.000 Pervious length
 7.500 Pervious slope
 0.000 Impervious Area
 200.000 Impervious length
 7.500 Impervious slope
 0.250 Pervious Manning 'n'
 78.000 Pervious SCS Curve No.
 0.446 Pervious Runoff coefficient
 0.100 Pervious Ia/S coefficient
 7.164 Pervious Initial abstraction
 0.015 Impervious Manning 'n'
 98.000 Impervious SCS Curve No.
 0.000 Impervious Runoff coefficient
 0.100 Impervious Ia/S coefficient
 0.518 Impervious Initial abstraction

317033 - Pre 100 yr - Jan19
 Hydrograph volume 0.028 0.028
 START/RE-START TOTALS 30"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT"

" " 38
 " " 19

317033 - Pre 100 yr - Jan19
 Hydrograph volume 0.028 0.028
 START/RE-START TOTALS 30"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT"

" " 38
 " " 19

317033 - Pre 100 yr - Jan19
 Hydrograph volume 0.028 0.028
 START/RE-START TOTALS 30"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT"

" " 38
 " " 19

317033 - Pre 100 yr - Jan19
 Hydrograph volume 0.028 0.028
 START/RE-START TOTALS 30"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520
 Total Impervious area 0.060
 Total % impervious 1.328"
 EXIT"

" " 38
 " " 19

40	Time of concentration	31.138	minutes"
"	Time to centroid	225.503	minutes"
"	Rainfall depth	76.839	mm"
"	Rainfall volume	1267.85	C.m"
"	Rainfall losses	42.575	mm"
"	Runoff depth	70.462	mm"
"	Runoff volume	565.36	C.m"
"	Runoff coefficient	0.446	"
"	Maximum flow	0.000	C.m/sec"
"	HYDROGRAPH Add Runoff "	0.114	
4	Add Runoff "	0.114	
"	HYDROGRAPH Copy to Outflow"	0.188	
8	Copy to Outflow"	0.114	
"	HYDROGRAPH " Combine 2"	0.188"	
6	Combine " "		
2	Node #"		
"	Northwesterly Outlet"		
"	Maximum flow	0.114	C.m/sec"
"	Hydrograph volume	565.358	C.m"
"	0.114	0.114	0.114"
40	HYDROGRAPH Start - New Tributary"		
2	Start, New Tributary"	0.114	0.114"
"	0.114	0.000	0.114
"	CATCHMENT 30"		
33	1 Triangular SCS"		
"	1 Equal length"		
"	1 SCS method"		
"	30 Northeastly Portion of Subject Property"		
"	0.000 % Impervious"		
"	0.310 Total Area"		
"	90.000 Flow length"		
"	8.000 Overland Slope"		
"	0.310 Pervious Area"		
"	90.000 Pervious length"		
"	8.000 Pervious slope"		
"	0.000 Impervious Area"		
"	90.000 Impervious length"		
"	8.000 Impervious slope"		
"	0.250 Pervious Manning "n" "		
"	78.000 Pervious SCS Curve No. "		
"	0.445 Pervious Runoff coefficient"		
"	0.100 Pervious Ia/S coefficient"		
"	7.164 Pervious Initial abstraction"		
"	0.015 Impervious Manning "n" "		
"	98.000 Impervious SCS Curve No. "		
"	0.000 Impervious Runoff coefficient"		
"	0.100 Impervious Ia/S coefficient"		
"	0.518 Impervious Initial abstraction"		
"	0.028 0.000 0.114		
"	Catchment 30		
"	Surface Area	0.310	Impervious Total Area "
"	Time of concentration	2.490	0.310 hectare "
"	Rainfall depth	167.633	18.915 minutes"
"	Rainfall volume	76.839	208.520 minutes"
"	Rainfall losses	42.575	76.839 mm"
"	Runoff depth	34.222	238.20 C.m"
"	Runoff volume	106.09	42.617 mm"
"	Runoff coefficient	0.445	34.222 mm"
"	Maximum flow	0.000	106.09 C.m"
"	HYDROGRAPH Add Runoff "	0.028	0.445 C.m/sec"
4	Add Runoff "	0.028	0.028 C.m/sec"
"	HYDROGRAPH Copy to Outflow"	0.114	
8	Copy to Outflow"	0.028	0.114"
"	HYDROGRAPH " Combine 3"	0.028	0.114"
6	Combine " "		
3	Node #"		
"	Northwesterly Outlet"		
"	Maximum flow	0.028	C.m/sec"

GLEN ALLAN SUBDIVISION
TOWNSHIP OF MAPLETON
OUR FILE: 317033
JANUARY 2019

SWM FACILITY
DRY POND

ELEV (m)	INC DEPTH (m)	SURFACE AREA (sq m)	AVERAGE AREA (sq m)	INCR. VOL (cu m)	ACCUM STORAGE VOL (cu m)	
394.00	0.00	0	0	0	0	Bottom of Pond; T/G of DICB-12
394.05	0.05	13	7	0	0.3	
394.10	0.10	49	31	2	2	
394.15	0.15	81	65	3	5	
394.20	0.20	111	96	5	10	
394.25	0.25	139	125	6	16	
394.30	0.30	155	147	7	24	
394.35	0.35	172	164	8	32	
394.40	0.40	189	181	9	41	
394.45	0.45	206	198	10	51	
394.50	0.50	225	216	11	61	
394.55	0.55	243	234	12	73	
394.60	0.60	263	253	13	86	
394.65	0.65	284	274	14	99	
394.70	0.70	305	295	15	114	
394.75	0.75	327	316	16	130	
394.80	0.80	348	338	17	147	
394.85	0.85	371	360	18	165	
394.90	0.90	394	383	19	184	
394.95	0.95	419	407	20	204	T/G of DICB-13
395.00	1.00	445	432	22	226	
395.05	1.05	473	459	23	249	
395.10	1.10	501	487	24	273	
395.15	1.15	530	516	26	299	
395.20	1.20	560	545	27	326	
395.25	1.25	590	575	29	355	
395.30	1.30	620	605	30	385	
395.35	1.35	651	636	32	417	
395.40	1.40	682	667	33	450	
395.45	1.45	714	698	35	485	
395.50	1.50	746	730	37	522	
395.55	1.55	779	763	38	560	
395.60	1.60	812	796	40	600	Weir
395.65	1.65	845	829	41	641	Overflow
395.70	1.70	879	862	43	684	Overflow
395.75	1.75	913	896	45	729	Overflow
395.79	1.79	941	927	37	766	Overflow; Min. T/ Pond Elev.

Primary Outlet Orifice		Secondary Outlet Orifice		Overflow Weir	
Orifice Dia.:	100 mm	Orifice Dia.:	150 mm	T/ Bank	395.79 m
Orifice Area:	0.008 m ²	Orifice Area:	0.018 m ²	Overflow Elv	395.60 m
Coefficient:	0.6	Coefficient:	0.6	Height of Overflow	0.19 m
Invert Elev:	393.60 m	Invert Elev:	393.55 m	2g =	19.612 m/s ²
				Length =	3 m
				Flow =	0.424 m ³ /s

ELEV (m)	STAGE (m)	STORAGE VOLUME (cu m)	DISCHARGE			TOTAL FLOW (cu m/s)	
			PRIMARY ORIFICE FLOW (cu m/s)	SECONDARY ORIFICE FLOW (cu m/s)	OVERFLOW WEIR FLOW (cu m/s)		
394.00	0.00	0	0.000	0.000	0.000	0.000	Bottom of Pond; T/G of DICB-12
394.05	0.05	0.3	0.013	0.000	0.000	0.013	
394.10	0.10	2	0.014	0.000	0.000	0.014	
394.15	0.15	5	0.015	0.000	0.000	0.015	
394.20	0.20	10	0.015	0.000	0.000	0.015	
394.25	0.25	16	0.016	0.000	0.000	0.016	
394.30	0.30	24	0.017	0.000	0.000	0.017	
394.35	0.35	32	0.017	0.000	0.000	0.017	
394.40	0.40	41	0.018	0.000	0.000	0.018	
394.45	0.45	51	0.019	0.000	0.000	0.019	
394.50	0.50	61	0.019	0.000	0.000	0.019	
394.55	0.55	73	0.020	0.000	0.000	0.020	
394.60	0.60	86	0.020	0.000	0.000	0.020	
394.65	0.65	99	0.021	0.000	0.000	0.021	
394.70	0.70	114	0.021	0.000	0.000	0.021	
394.75	0.75	130	0.022	0.000	0.000	0.022	
394.80	0.80	147	0.022	0.000	0.000	0.022	
394.85	0.85	165	0.023	0.000	0.000	0.023	
394.90	0.90	184	0.023	0.000	0.000	0.023	
394.95	0.95	204	0.024	0.000	0.000	0.024	T/G of DICB-13
395.00	1.00	226	0.000	0.055	0.000	0.055	
395.05	1.05	249	0.000	0.056	0.000	0.056	
395.10	1.10	273	0.000	0.057	0.000	0.057	
395.15	1.15	299	0.000	0.058	0.000	0.058	
395.20	1.20	326	0.000	0.059	0.000	0.059	
395.25	1.25	355	0.000	0.060	0.000	0.060	
395.30	1.30	385	0.000	0.061	0.000	0.061	
395.35	1.35	417	0.000	0.062	0.000	0.062	
395.40	1.40	450	0.000	0.063	0.000	0.063	
395.45	1.45	485	0.000	0.063	0.000	0.063	
395.50	1.50	522	0.000	0.064	0.000	0.064	
395.55	1.55	560	0.000	0.065	0.000	0.065	
395.60	1.60	600	0.000	0.066	0.000	0.066	Weir
395.65	1.65	641	0.000	0.067	0.057	0.124	Overflow
395.70	1.70	684	0.000	0.068	0.162	0.229	Overflow
395.75	1.75	729	0.000	0.068	0.297	0.366	Overflow
395.79	1.79	766	0.000	0.069	0.424	0.493	Overflow; Min. T/ Pond Elev.

MIDUSS Output ----->
MIDUSS version Version 2.25 rev. 472
MIDUSS created Sunday, February 07, 2010
Units used: The METRIC
Job Folder: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033
Output filename: 317033 - Post 2 yr - Jan19.out
Licensee name: Hewlett-Packard Company
Company gmbp
Date & Time last used: 1/22/2019 at 9:47:39 AM
TIME PARAMETERS
31 10.000 Time Step
360.000 Max. Storm Length
2400.000 Max. Hydrograph
STORM Chicago storm
1 Chicago storm
Coefficient A
774.890
Constant B
7.160
Exponent C
0.817
Fraction R
0.375
360.000 Duration
1.000 Time step multiplier
Maximum intensity 74.888 mm/hr
Total depth 37.317 mm
6 CATCHMENT 100 Hydrograph extension used in this file
1 Triangular SCS
1 SCS method
1 Equal length
100 Southernly Portion of Subject Property
20.000 % Impervious
3.080 Total Area
30.000 Flow Length
5.000 Overland Slope
2.464 Pervious Area
30.000 Pervious Length
5.000 Pervious Slope
0.616 Impervious Area
30.000 Impervious Length
5.000 Impervious Slope
0.250 Pervious Manning 'n'
78.200 Pervious SCS Curve No.
0.238 Pervious Runoff coefficient
7.164 Pervious Ia/S coefficient
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.831 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
0.109 0.000

Catchment 100 Pervious 0.000 c.m./sec
Surface Area 2.464 hectare
Time of concentration 20.437 minutes
Rainfall depth 37.317 mm
Rainfall volume 217.904 mm
Rainfall losses 28.421 mm
Runoff depth 8.896 mm
Runoff volume 219.20 mm
Runoff coefficient 0.338
Maximum flow 191.13 c.m.
HYDROGRAPH Add, Runoff " 0.031 0.098 0.109 c.m./sec
4 Add Runoff 0.109 0.000 0.000 0.000

CATCHMENT 101
1 Triangular SCS
1 Equal length
1 SCS method
101 Easterly Off-Site Lands
35.000 % Impervious
0.240 Total Area
10.000 Flow Length

Catchment 101 Pervious 0.000 c.m./sec
Surface Area 3.080 hectare
Time of concentration 20.437 minutes
Rainfall depth 37.317 mm
Rainfall volume 217.904 mm
Rainfall losses 28.421 mm
Runoff depth 8.896 mm
Runoff volume 219.20 mm
Runoff coefficient 0.338
Maximum flow 191.13 c.m.
HYDROGRAPH Add, Runoff " 0.031 0.098 0.109 c.m./sec
4 Add Runoff 0.109 0.000 0.000 0.000

CATCHMENT 101
1 Triangular SCS
1 Equal length
1 SCS method
101 Easterly Off-Site Lands
35.000 % Impervious
0.240 Total Area
10.000 Flow Length

Overland Slope
Pervious Area
Pervious Length
Pervious Slope
Impervious Area
Impervious Length
Impervious Slope
Pervious Manning 'n'
Pervious SCS Curve No.
Pervious Ia/S coefficient
Pervious Runoff coefficient
Impervious Manning 'n'
Impervious SCS Curve No.
Impervious Runoff coefficient
Impervious Ia/S coefficient
Impervious Initial abstraction
0.015 0.109 0.000
Catchment 101 Pervious 0.000 c.m./sec
Surface Area 0.156 hectare
Time of concentration 10.572 minutes
Rainfall depth 37.317 mm
Rainfall volume 204.984 mm
Rainfall losses 28.421 mm
Runoff depth 8.889 mm
Runoff volume 13.87 mm
Runoff coefficient 0.238
Maximum flow 0.004 c.m.
HYDROGRAPH Add Runoff " 0.004 0.014 0.014 c.m./sec
4 Add Runoff 0.015 0.124 0.000 0.000

POND DESIGN
0.124 Current peak flow c.m./sec
0.001 Target outflow c.m./sec
448.7 Hydrograph volume c.m.
37 Number of stages
394.000 Minimum water level metre
395.790 Maximum water level metre
394.000 Startling water level metre
0 Keep Design Data: I = True; O = False
Level Discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.9000
394.150 0.01480 5.1000
394.200 0.01550 9.9000
394.250 0.01620 16.2000
394.300 0.01680 23.5000
394.350 0.01750 31.7000
394.400 0.01810 40.7000
394.450 0.01870 50.6000
394.500 0.01920 61.4000
394.550 0.01980 73.1000
394.600 0.02030 85.7000
394.650 0.02090 99.4000
394.700 0.02140 114.1000
394.750 0.02190 129.8000
394.800 0.02240 146.8000
394.850 0.02290 164.8000
394.900 0.02350 183.9000
394.950 0.02380 204.2000
395.000 0.02400 225.8000
395.050 0.02420 248.8000
395.100 0.02430 273.1000
395.150 0.02440 298.9000
395.200 0.02450 326.1000
395.250 0.02460 354.9000
395.300 0.02470 385.2000
395.350 0.02480 416.9000
395.400 0.02490 450.2000
395.450 0.02500 485.1000
395.500 0.02510 521.7000

40
54
0.124 Current peak flow c.m./sec
0.001 Target outflow c.m./sec
448.7 Hydrograph volume c.m.
37 Number of stages
394.000 Minimum water level metre
395.790 Maximum water level metre
394.000 Startling water level metre
0 Keep Design Data: I = True; O = False
Level Discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.9000
394.150 0.01480 5.1000
394.200 0.01550 9.9000
394.250 0.01620 16.2000
394.300 0.01680 23.5000
394.350 0.01750 31.7000
394.400 0.01810 40.7000
394.450 0.01870 50.6000
394.500 0.01920 61.4000
394.550 0.01980 73.1000
394.600 0.02030 85.7000
394.650 0.02090 99.4000
394.700 0.02140 114.1000
394.750 0.02190 129.8000
394.800 0.02240 146.8000
394.850 0.02290 164.8000
394.900 0.02350 183.9000
394.950 0.02380 204.2000
395.000 0.02400 225.8000
395.050 0.02420 248.8000
395.100 0.02430 273.1000
395.150 0.02440 298.9000
395.200 0.02450 326.1000
395.250 0.02460 354.9000
395.300 0.02470 385.2000
395.350 0.02480 416.9000
395.400 0.02490 450.2000
395.450 0.02500 485.1000
395.500 0.02510 521.7000

Catchment 101 Pervious 0.000 c.m./sec
Surface Area 0.156 hectare
Time of concentration 10.572 minutes
Rainfall depth 37.317 mm
Rainfall volume 204.984 mm
Rainfall losses 28.421 mm
Runoff depth 8.889 mm
Runoff volume 13.87 mm
Runoff coefficient 0.238
Maximum flow 0.004 c.m.
HYDROGRAPH Add Runoff " 0.004 0.014 0.014 c.m./sec
4 Add Runoff 0.015 0.124 0.000 0.000

Catchment 101 Pervious 0.000 c.m./sec
Surface Area 0.156 hectare
Time of concentration 10.572 minutes
Rainfall depth 37.317 mm
Rainfall volume 204.984 mm
Rainfall losses 28.421 mm
Runoff depth 8.889 mm
Runoff volume 13.87 mm
Runoff coefficient 0.238
Maximum flow 0.004 c.m.
HYDROGRAPH Add Runoff " 0.004 0.014 0.014 c.m./sec
4 Add Runoff 0.015 0.124 0.000 0.000

MIDUSS Output
 MIDUSS version Version 2.25 rev. 473
 MIDUSS created Sunday, February 07, 2010
 Units used: \\os-2k8\users_private\awilkinson\Documents\1a\METRIC.MIDUSS\317033
 Job folder: \\os-2k8\users_private\awilkinson\Documents\1a\METRIC.MIDUSS\317033
 Output filename: 317033 - Post 5 yr - Jan19.out
 Licensee name: Hewlett-Packard Company
 Company gmbp
 Date & Time last used: 1/22/2019 at 9:51:33 AM

31 10.000 Time step 93.011 mm/hr
 360.000 Max. storm length 47.926 mm
 2400.000 Max. Hydrograph 93.011 mm/hr
 32 STORM Chicago storm
 1 Chicago storm
 1012.690 Coefficient A
 8.094 Constant B
 0.820 Exponent C
 0.375 Fraction R
 360.000 Duration
 1.000 Time step multiplier
 6 .005hyd Hydrograph extension used in this file
 33 CATCHMENT 100
 1 Triangular SCS
 1 SCS method
 100 Southerly Portion of subject Property
 3.080 Total Area
 30.000 Flow length
 5.000 Overland Slope
 2.464 Pervious Area
 30.000 Pervious length
 5.000 Pervious slope
 0.616 Pervious Area
 30.000 Pervious length
 5.000 Pervious slope
 0.250 Pervious Manning "n"
 78.000 Pervious SCS Curve No.
 0.307 Pervious Runoff coefficient
 0.100 Pervious Ia/S coefficient
 7.164 Pervious Initial abstraction
 0.015 Pervious Manning "n"
 98.000 Pervious SCS Curve No.
 0.852 Pervious Runoff coefficient
 0.100 Pervious Ia/S coefficient
 0.518 Pervious Initial abstraction
 0.162 0.000
 Catchment 100 Pervious 0.000 c.m./sec
 Surface Area 0.156 Hectare
 Time of concentration 8.386 0.240
 Rainfall depth 47.926 4.006
 Rainfall volume 199.210 181.145
 Rainfall losses 74.76 47.926
 Runoff volume 33.304 24.946
 Runoff depth 14.621 9.425
 Runoff coefficient 22.81 38.501
 Maximum flow 0.305 55.15
 HYDROGRAPH Add Runoff " 0.008 0.803
 4 Add Runoff " 0.177 0.000 0.000
 54 POND DESIGN
 0.177 Current peak flow c.m./sec
 0.001 Target outflow c.m./sec
 669.3 Hydrograph volume c.m.
 37 Number of stages metre
 394.000 Minimum water level metre
 395.790 Maximum water level metre
 394.000 Starting water level metre
 0 Keep Design Data: I = True; O = False
 Level Discharge Volume
 394.000 0.000
 394.050 0.01320 0.500
 394.100 0.01400 1.900
 394.150 0.01480 5.100
 394.200 0.01550 9.900
 394.250 0.01620 16.200
 394.300 0.01680 23.500
 394.350 0.01750 31.700
 394.400 0.01810 40.700
 394.450 0.01870 50.600
 394.500 0.01920 61.400
 394.550 0.01980 73.100
 394.600 0.02030 85.700
 394.650 0.02090 99.400
 394.700 0.02140 114.100
 394.750 0.02190 129.900
 394.800 0.02240 146.800
 394.850 0.02290 164.800
 394.900 0.02330 183.800
 394.950 0.02380 204.200
 395.000 0.02510 225.800
 395.050 0.02610 248.800
 395.100 0.02700 273.100
 395.150 0.02790 298.900
 395.200 0.02890 326.100
 395.250 0.02990 354.900
 395.300 0.03090 385.200
 395.350 0.03170 416.900
 395.400 0.03260 450.200
 395.450 0.03340 485.100
 395.500 0.03430 521.700

" 33

395.550 0.06510 0.057 c.m/sec
 559.800 0.06600 395.103 metre
 599.600 0.12400 274.588 c.m.
 395.650 0.12400 4.581 hours
 641.000 0.177 0.057 0.000 c.m/sec
 395.700 0.177 0.057 0.000 c.m/sec
 395.750 0.177 0.057 0.000 c.m/sec
 728.500 0.177 0.057 0.000 c.m/sec
 395.780 0.177 0.057 0.000 c.m/sec
 766.000 0.177 0.057 0.000 c.m/sec

" 40

Peak outflow 0.057 c.m/sec
 Maximum level 395.103 metre
 Maximum storage 274.588 c.m.
 Centroidal lag 4.581 hours
 0.022 0.177 0.057 0.000 c.m/sec
 HYDROGRAPH " Combine 1"
 6 " Combine " Combine 1"
 1 Node #"
 Easterly Outlet" 0.057 c.m/sec
 Maximum flow 671.464 c.m.
 Hydrograph volume 0.057 0.057"
 0.022 0.177 0.057 0.057"
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary" 0.057 0.057"
 0.022 0.000 0.057 0.057"
 CATCHMENT "200"
 1 Triangular SCS"
 1 Equal length"
 1 SCS method"
 200 Northwesternly Portion of Subject Property"
 10.000 % Impervious
 0.980 Total Area"
 120.000 Flow length"
 10.000 Overland Slope"
 0.882 Pervious Area"
 120.000 Pervious length"
 10.000 Pervious slope"
 0.098 Pervious Area"
 120.000 Impervious length"
 10.000 Impervious slope"
 0.250 Pervious Manning "n"
 78.000 Pervious SCS Curve No."
 0.308 Pervious Runoff coefficient"
 0.100 Pervious Ia/S coefficient"
 7.164 Pervious Initial abstraction"
 0.015 Pervious Manning "n"
 98.000 Impervious SCS Curve No."
 0.879 Impervious Runoff coefficient"
 0.100 Impervious Ia/S coefficient"
 0.518 Impervious Initial abstraction"

" 40

Catchment 200 Pervious 0.057 c.m/sec
 Surface Area 0.980 Total Area " hectare"
 Time of concentration 3.310 0.980 23.766 0.980
 0.882 3.310 23.766 minutes
 30.252 170.715 214.517 minutes
 228.405 47.926 47.926 mm
 Rainfall depth 469.67 469.67 c.m.
 422.70 469.67 469.67 mm
 Rainfall volume 5.777 30.417 30.417 mm
 33.155 30.417 30.417 mm
 Rainfall losses 14.771 42.149 42.149 mm
 Runoff depth 130.28 41.31 41.31 mm
 Runoff volume 0.308 171.58 171.58 c.m.
 Runoff coefficient 0.879 0.365 0.365 c.m.
 Maximum flow 0.025 0.018 0.018 c.m/sec
 HYDROGRAPH Add Runoff " 0.031
 4 Add Runoff " 0.031
 0.031 0.031 0.057 0.057"
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.031 0.031 0.057"
 0.031 0.031 0.031 0.057"
 HYDROGRAPH " Combine 2"
 6 " Combine " Combine 2"
 2 Node #"
 Northwesternly Outlet" 0.031 c.m/sec
 Maximum flow 171.582 c.m.
 Hydrograph volume 0.031 0.031
 0.031 0.031 0.031 0.031"
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary"

" 40

Catchment 300 Pervious 0.031 c.m/sec
 Surface Area 0.198 Total Area " hectare"
 Time of concentration 17.891 0.198 14.118 minutes
 0.958 17.891 14.118 minutes
 211.730 168.723 201.547 mm
 Rainfall depth 47.926 47.926 47.926 mm
 94.89 10.54 105.44 c.m.
 Rainfall losses 33.193 6.792 30.553 mm
 Runoff length 14.732 41.133 17.373 mm
 Runoff volume 29.17 9.05 38.22 c.m.
 Runoff coefficient 0.307 0.858 0.858 c.m.
 Maximum flow 0.007 0.004 0.009 c.m/sec
 HYDROGRAPH Add Runoff " 0.031
 4 Add Runoff " 0.031
 0.009 0.009 0.009 0.031"
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.009 0.009 0.031"
 0.009 0.009 0.009 0.031"
 HYDROGRAPH " Combine 3"
 6 " Combine " Combine 3"
 3 Node #"
 Northeastly Outlet" 0.009 c.m/sec
 Maximum flow 38.220 c.m.
 Hydrograph volume 0.009 0.009
 0.009 0.009 0.009 0.009"
 START/RE-START TOTALS 300"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520 hectare"
 Total Impervious area 0.820 hectare"
 Total % Impervious 18.142
 EXIT"

" 19

" 33

395.550 0.06510 0.057 c.m/sec
 559.800 0.06600 395.103 metre
 599.600 0.12400 274.588 c.m.
 395.650 0.12400 4.581 hours
 641.000 0.177 0.057 0.000 c.m/sec
 395.700 0.177 0.057 0.000 c.m/sec
 395.750 0.177 0.057 0.000 c.m/sec
 728.500 0.177 0.057 0.000 c.m/sec
 395.780 0.177 0.057 0.000 c.m/sec
 766.000 0.177 0.057 0.000 c.m/sec

" 40

Peak outflow 0.057 c.m/sec
 Maximum level 395.103 metre
 Maximum storage 274.588 c.m.
 Centroidal lag 4.581 hours
 0.022 0.177 0.057 0.000 c.m/sec
 HYDROGRAPH " Combine 1"
 6 " Combine " Combine 1"
 1 Node #"
 Easterly Outlet" 0.057 c.m/sec
 Maximum flow 671.464 c.m.
 Hydrograph volume 0.057 0.057"
 0.022 0.177 0.057 0.057"
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary" 0.057 0.057"
 0.022 0.000 0.057 0.057"
 CATCHMENT "200"
 1 Triangular SCS"
 1 Equal length"
 1 SCS method"
 200 Northwesternly Portion of Subject Property"
 10.000 % Impervious
 0.980 Total Area"
 120.000 Flow length"
 10.000 Overland Slope"
 0.882 Pervious Area"
 120.000 Pervious length"
 10.000 Pervious slope"
 0.098 Pervious Area"
 120.000 Impervious length"
 10.000 Impervious slope"
 0.250 Pervious Manning "n"
 78.000 Pervious SCS Curve No."
 0.308 Pervious Runoff coefficient"
 0.100 Pervious Ia/S coefficient"
 7.164 Pervious Initial abstraction"
 0.015 Pervious Manning "n"
 98.000 Impervious SCS Curve No."
 0.879 Impervious Runoff coefficient"
 0.100 Impervious Ia/S coefficient"
 0.518 Impervious Initial abstraction"

" 40

Catchment 200 Pervious 0.057 c.m/sec
 Surface Area 0.882 Total Area " hectare"
 Time of concentration 3.310 0.980 23.766 0.980
 0.882 3.310 23.766 minutes
 30.252 170.715 214.517 minutes
 228.405 47.926 47.926 mm
 Rainfall depth 469.67 469.67 c.m.
 422.70 469.67 469.67 mm
 Rainfall volume 5.777 30.417 30.417 mm
 33.155 30.417 30.417 mm
 Rainfall losses 14.771 42.149 42.149 mm
 Runoff depth 130.28 41.31 41.31 mm
 Runoff volume 0.308 171.58 171.58 c.m.
 Runoff coefficient 0.879 0.365 0.365 c.m.
 Maximum flow 0.025 0.018 0.018 c.m/sec
 HYDROGRAPH Add Runoff " 0.031
 4 Add Runoff " 0.031
 0.031 0.031 0.057 0.057"
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.031 0.031 0.057"
 0.031 0.031 0.031 0.057"
 HYDROGRAPH " Combine 2"
 6 " Combine " Combine 2"
 2 Node #"
 Northwesternly Outlet" 0.031 c.m/sec
 Maximum flow 171.582 c.m.
 Hydrograph volume 0.031 0.031
 0.031 0.031 0.031 0.031"
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary"

" 40

Catchment 300 Pervious 0.031 c.m/sec
 Surface Area 0.198 Total Area " hectare"
 Time of concentration 17.891 0.198 14.118 minutes
 0.958 17.891 14.118 minutes
 211.730 168.723 201.547 mm
 Rainfall depth 47.926 47.926 47.926 mm
 94.89 10.54 105.44 c.m.
 Rainfall losses 33.193 6.792 30.553 mm
 Runoff length 14.732 41.133 17.373 mm
 Runoff volume 29.17 9.05 38.22 c.m.
 Runoff coefficient 0.307 0.858 0.858 c.m.
 Maximum flow 0.007 0.004 0.009 c.m/sec
 HYDROGRAPH Add Runoff " 0.031
 4 Add Runoff " 0.031
 0.009 0.009 0.009 0.031"
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.009 0.009 0.031"
 0.009 0.009 0.009 0.031"
 HYDROGRAPH " Combine 3"
 6 " Combine " Combine 3"
 3 Node #"
 Northeastly Outlet" 0.009 c.m/sec
 Maximum flow 38.220 c.m.
 Hydrograph volume 0.009 0.009
 0.009 0.009 0.009 0.009"
 START/RE-START TOTALS 300"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520 hectare"
 Total Impervious area 0.820 hectare"
 Total % Impervious 18.142
 EXIT"

" 19

MIDUSS Output ----- Version 2.25 Rev. 473
MIDUSS version Sunday, February 07, 2010
Units created: \\os-2k8\users_private\awilkinson\Documents\1e METRIC
Job folder: MIDUSS\317033
Output filename: 317033 - Post 10 yr - Jan19.out
Company: Hewlett-Packard Company
Date & Time last used: 1/22/2019 at 10:27:30 AM
License name: gmpb

31 TIME PARAMETERS
10.000 Time Step
360.000 Max. Storm length
2400.000 Max. Hydrograph
STORM Chicago storm
1 Chicago storm
Coefficient A
8.674 Constant B
0.822 Exponent C
0.375 Fraction R
360.000 Duration
1.000 Time step multiplier
Maximum intensity 104.616 mm/hr
Total depth 34.935 mm
6 CATCHMENT 100 Hydrograph extension used in this file

33 1 Triangular SCS
1 SCS method
20.000 Southernly portion of Subject Property
3.080 Total Area
30.000 Flow length
5.000 Overland Slope
2.464 Pervious Area
30.000 Pervious length
5.000 Pervious slope
0.616 Impervious Area
30.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning "n"
78.200 Pervious SCS Curve No.
0.346 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.860 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

40 Catchment 100 Pervious 0.000 c.m/sec
Surface Area 2.464 Impervious Total Area 0.000 c.m/sec
Time of concentration 14.491 Pervious 3.080 hectare
1.685 9.581
167.663 191.260
54.935 54.935
338.40 1691.99
7.671 30.282
47.264 24.652
291.14 759.30
0.860 0.449
0.145 0.209
4 Add Runoff 0.209 0.000 0.000

33 CATCHMENT 101
1 Triangular SCS
1 Equal length
SCS method
20.000 Southernly portion of Subject Property
3.080 Total Area
30.000 Flow length
5.000 Overland Slope
2.464 Pervious Area
30.000 Pervious length
5.000 Pervious slope
0.616 Impervious Area
30.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning "n"
78.200 Pervious SCS Curve No.
0.346 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.860 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

40 Catchment 101 Pervious 0.000 c.m/sec
Surface Area 2.464 Impervious Total Area 0.000 c.m/sec
Time of concentration 14.491 Pervious 3.080 hectare
1.685 9.581
167.663 191.260
54.935 54.935
338.40 1691.99
7.671 30.282
47.264 24.652
291.14 759.30
0.860 0.449
0.145 0.209
4 Add Runoff 0.209 0.000 0.000

101 Easterly Off-Site Lands
35.000 % Impervious
0.240 Total Area
10.000 Flow length

5.000 Overland Slope
0.156 Pervious Area
10.000 Pervious length
5.000 Pervious slope
0.084 Impervious Area
10.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning "n"
78.000 Pervious SCS Curve No.
0.340 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.812 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 101 Pervious 0.000 c.m/sec
Surface Area 2.464 Impervious Total Area 0.000 c.m/sec
Time of concentration 14.491 Pervious 3.080 hectare
1.685 9.581
167.663 191.260
54.935 54.935
338.40 1691.99
7.671 30.282
47.264 24.652
291.14 759.30
0.860 0.449
0.145 0.209
4 Add Runoff 0.209 0.000 0.000

40 Catchment 101 Pervious 0.000 c.m/sec
Surface Area 2.464 Impervious Total Area 0.000 c.m/sec
Time of concentration 14.491 Pervious 3.080 hectare
1.685 9.581
167.663 191.260
54.935 54.935
338.40 1691.99
7.671 30.282
47.264 24.652
291.14 759.30
0.860 0.449
0.145 0.209
4 Add Runoff 0.209 0.000 0.000

54 POND DESIGN
0.227 Current peak flow c.m/sec
0.001 Target outflow c.m/sec
825.9 Hydrograph volume c.m
37 Number of stages metre
394.000 Minimum water level metre
395.700 Maximum water level metre
394.000 Starting water level metre
0 Keep Design data: I = True; O = False
Level Discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.900
394.150 0.01480 5.100
394.200 0.01550 9.900
394.250 0.01620 16.200
394.300 0.01680 23.500
394.350 0.01750 31.700
394.400 0.01810 40.700
394.450 0.01870 50.600
394.500 0.01920 61.400
394.550 0.01980 73.100
394.600 0.02030 85.700
394.650 0.02090 99.400
394.700 0.02140 114.100
394.750 0.02190 129.900
394.800 0.02240 146.800
394.850 0.02290 164.800
394.900 0.02330 183.900
394.950 0.02360 204.200
395.000 0.02380 225.800
395.050 0.02400 248.800
395.100 0.02420 273.100
395.150 0.02440 298.900
395.200 0.02460 326.100
395.250 0.02480 354.900
395.300 0.02500 385.200
395.350 0.02520 416.900
395.400 0.02540 450.200
395.450 0.02560 485.100
395.500 0.02580 521.700

40 Catchment 101 Pervious 0.000 c.m/sec
Surface Area 2.464 Impervious Total Area 0.000 c.m/sec
Time of concentration 14.491 Pervious 3.080 hectare
1.685 9.581
167.663 191.260
54.935 54.935
338.40 1691.99
7.671 30.282
47.264 24.652
291.14 759.30
0.860 0.449
0.145 0.209
4 Add Runoff 0.209 0.000 0.000

54 POND DESIGN
0.227 Current peak flow c.m/sec
0.001 Target outflow c.m/sec
825.9 Hydrograph volume c.m
37 Number of stages metre
394.000 Minimum water level metre
395.700 Maximum water level metre
394.000 Starting water level metre
0 Keep Design data: I = True; O = False
Level Discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.900
394.150 0.01480 5.100
394.200 0.01550 9.900
394.250 0.01620 16.200
394.300 0.01680 23.500
394.350 0.01750 31.700
394.400 0.01810 40.700
394.450 0.01870 50.600
394.500 0.01920 61.400
394.550 0.01980 73.100
394.600 0.02030 85.700
394.650 0.02090 99.400
394.700 0.02140 114.100
394.750 0.02190 129.900
394.800 0.02240 146.800
394.850 0.02290 164.800
394.900 0.02330 183.900
394.950 0.02360 204.200
395.000 0.02380 225.800
395.050 0.02400 248.800
395.100 0.02420 273.100
395.150 0.02440 298.900
395.200 0.02460 326.100
395.250 0.02480 354.900
395.300 0.02500 385.200
395.350 0.02520 416.900
395.400 0.02540 450.200
395.450 0.02560 485.100
395.500 0.02580 521.700

MIDUSS Output ----- Version 2.25 rev. 473
MIDUSS version Sunday, February 07, 2010
MIDUSS created 1e METRIC
Units used: \\os-2k8\users_private\awitkinson\Documents\MIDUSS\317033
Job folder: 317033 - Post 25 yr - Jan19.out
Output filename: Hewlett-Packard Company
Licensee name: gmbp
Date & Time last used: 1/22/2019 at 10:40:03 AM

TIME PARAMETERS

31 10.000 Time Step
360.000 Max. Storm length
2400.000 Max. Hydrograph
STORM Chicago storm
1 1381.400 Coefficient A
9.174 Constant B
0.823 Exponent C
0.373 Fraction R
360.000 Duration
1.000 Time step multiplier
Maximum intensity 119.599 mm/hr
Total depth 63.732 mm
6 0.25hyd Hydrograph extension used in this file

CATCHMENT 100

1 Equal length
1 SCS method
100 Southernly Portion of subject property
20.000 % Impervious
3.080 Total Area
5.000 Flow length
2.464 Pervious Area
30.000 Pervious length
5.000 Pervious slope
0.616 Impervious Area
30.000 Impervious length
5.000 Impervious slope
78.230 Pervious Manning 'n'
0.388 Pervious SCS Curve No.
0.100 Pervious Runoff coefficient
7.164 Pervious Ia/S coefficient
0.015 Impervious Manning 'n'
98.000 Impervious SCS Curve No.
0.868 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
0.265 Impervious Initial abstraction
0.000 Pervious Total Area
2.464 Surface Area
12.899 Time of concentration
202.348 Target outflow
189.647 Hydrograph volume
63.732 Rainfall depth
1570.35 Rainfall volume
8.402 Rainfall losses
39.004 Runoff depth
24.728 Runoff volume
609.30 Runoff coefficient
340.83 Maximum flow
0.868 Maximum flow
0.182 HYDROGRAPH Add Runoff

CATCHMENT 101

1 Triangular SCS
1 Equal length
1 SCS method
101 Easterly Off-Site Lands
35.000 % Impervious
0.240 Total Area
10.000 Flow length

5.000 Overland Slope
0.156 Pervious Area
10.000 Pervious length
3.000 Pervious slope
0.084 Impervious Area
10.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning 'n'
78.000 Pervious SCS Curve No.
0.383 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Manning 'n'
98.000 Pervious SCS Curve No.
0.820 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
0.518 Impervious Initial abstraction
0.032 Impervious Initial abstraction
0.265 Impervious Initial abstraction
0.000 Pervious Total Area
0.139 Surface Area
0.084 Imperious Total Area
9.673 Time of concentration
193.860 Target outflow
63.732 Rainfall depth
63.732 Rainfall volume
99.42 Rainfall losses
39.317 Runoff depth
24.414 Runoff volume
38.09 Runoff coefficient
0.383 Maximum flow
0.014 HYDROGRAPH Add Runoff

4 Add Runoff

0.032 0.288 0.000 0.000
POND DESIGN
0.288 Current peak flow c.m/sec
0.001 Target outflow c.m/sec
1032.1 Hydrograph volume c.m
37 Number of stages
394.000 Minimum water level metre
395.790 Maximum water level metre
394.000 Starting water level metre
0 Keep Design data: 1 = True; 0 = False

Level Discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.900
394.150 0.01480 5.100
394.200 0.01550 9.900
394.250 0.01620 16.200
394.300 0.01680 23.500
394.350 0.01750 31.700
394.400 0.01810 40.700
394.450 0.01870 50.600
394.500 0.01920 61.400
394.550 0.01980 73.100
394.600 0.02030 85.700
394.650 0.02090 99.400
394.700 0.02140 114.100
394.750 0.02190 129.900
394.800 0.02240 146.800
394.850 0.02290 164.800
394.900 0.02330 183.900
394.950 0.02380 204.200
395.000 0.05510 225.800
395.050 0.05610 248.800
395.100 0.05700 273.100
395.150 0.05800 298.900
395.200 0.05890 326.100
395.250 0.05990 354.900
395.300 0.06080 385.200
395.350 0.06170 416.900
395.400 0.06260 450.200
395.450 0.06340 485.100
395.500 0.06430 521.700

"	"	395.550	0.06510	559.800"	0.058	0.000	0.058"
"	"	395.600	0.06600	599.600"			
"	"	395.650	0.1340	641.000"			
"	"	395.700	0.2194	684.100"			
"	"	395.750	0.3656	728.500"			
"	"	395.790	0.4927	766.000"			
"	"	Peak outflow					
"	"	Maximum level	0.063	395.451			
"	"	Maximum storage	0.063	486.134			
"	"	Centroidal lag	0.063	4.764			
"	"	0.032	0.288	0.000			
"	40	HYDROGRAPH Combine	1"				
"	"	6 Combine "					
"	"	1 Node #"					
"	"	Easterly Outlet"					
"	"	Maximum flow	0.063	1040.417			
"	"	Hydrograph volume	0.063	0.063"			
"	40	HYDROGRAPH Start - New Tributary"					
"	"	2 Start - New Tributary"	0.000	0.063			
"	33	CATCHMENT 200"					
"	"	1 Triangular SCS"					
"	"	1 SCS method"					
"	"	200 Northwestery Portion of Subject Property"					
"	"	% Impervious"					
"	"	10.000 Total Area"					
"	"	120.000 Flow Length"					
"	"	0.882 Overland Slope"					
"	"	120.000 Pervious Area"					
"	"	10.000 Pervious length"					
"	"	0.098 Pervious slope"					
"	"	120.000 Impervious Area"					
"	"	10.000 Impervious length"					
"	"	0.250 Impervious slope"					
"	"	78.000 Pervious Manning "n" "					
"	"	0.391 Pervious SCS Curve No. "					
"	"	0.100 Pervious Ia/S coefficient"					
"	"	7.164 Pervious Initial abstraction"					
"	"	0.015 Pervious Manning "n" "					
"	"	98.000 Impervious SCS Curve No. "					
"	"	0.901 Impervious Runoff coefficient"					
"	"	0.100 Impervious Ia/S coefficient"					
"	"	0.518 Impervious Initial abstraction"					
"	"	Catchment 200					
"	"	Surface Area	0.882	19.769			
"	"	Time of concentration	24.071	207.475			
"	"	Time to Centroid	217.346	624.57			
"	"	Rainfall depth	63.732	6.291			
"	"	Rainfall volume	562.11	57.441			
"	"	Rainfall losses	38.811	28.173			
"	"	Runoff depth	24.921	276.10			
"	"	Runoff volume	219.81	0.901			
"	"	Runoff coefficient	0.391	0.442			
"	"	Maximum flow	0.030	0.025			
"	40	HYDROGRAPH Add. Runoff "					
"	"	4 Add Runoff "	0.058	0.063			
"	"	HYDROGRAPH Copy to. Outflow"					
"	40	8 Copy to Outflow"	0.058	0.058			
"	"	HYDROGRAPH Combine	2"	0.058			
"	40	6 Combine "					
"	"	2 Node #"					
"	"	Northwesterly Outlet"					
"	"	Maximum flow	0.058	276.097			
"	"	Hydrograph volume	0.058	0.058			
"	40	HYDROGRAPH Start - New Tributary"					
"	"	2 Start - New Tributary"	0.058	0.058			

"	"	33	CATCHMENT 300"				
"	"	1 Triangular SCS"					
"	"	1 SCS method"					
"	"	300 Northeastery Portion of Subject Property"					
"	"	% Impervious"					
"	"	10.000 Total Area"					
"	"	50.000 Flow Length"					
"	"	10.000 Overland Slope"					
"	"	0.198 Pervious Area"					
"	"	50.000 Pervious length"					
"	"	10.000 Pervious slope"					
"	"	10.000 Impervious Area"					
"	"	50.000 Impervious length"					
"	"	10.000 Impervious slope"					
"	"	0.250 Pervious Manning "n" "					
"	"	78.000 Pervious SCS Curve No. "					
"	"	0.389 Pervious Runoff coefficient"					
"	"	0.100 Pervious Ia/S coefficient"					
"	"	7.164 Pervious Initial abstraction"					
"	"	0.015 Pervious Manning "n" "					
"	"	98.000 Impervious SCS Curve No. "					
"	"	0.875 Impervious Runoff coefficient"					
"	"	0.100 Impervious Ia/S coefficient"					
"	"	0.518 Impervious Initial abstraction"					
"	"	Catchment 300					
"	"	Surface Area	0.198	11.739			
"	"	Time of concentration	14.235	167.139			
"	"	Time to Centroid	204.078	63.732			
"	"	Rainfall depth	63.732	14.02			
"	"	Rainfall volume	126.19	7.945			
"	"	Rainfall losses	38.947	55.787			
"	"	Runoff depth	24.785	12.27			
"	"	Runoff volume	49.07	0.875			
"	"	Runoff coefficient	0.389	0.438			
"	"	Maximum flow	0.015	0.006			
"	40	HYDROGRAPH Add. Runoff "					
"	"	4 Add Runoff "	0.018	0.058			
"	"	HYDROGRAPH Copy to. Outflow"					
"	40	8 Copy to Outflow"	0.018	0.018			
"	"	HYDROGRAPH Combine	3"	0.018			
"	40	6 Combine "					
"	"	3 Node #"					
"	"	Northeasterly Outlet"					
"	"	Maximum flow	0.018	61.347			
"	"	Hydrograph volume	0.018	0.018			
"	38	3 START/RE-START TOTALS 300"					
"	"	Runoff Totals on EXIT"					
"	"	Total Catchment area					
"	"	Total Impervious area					
"	"	Total % Impervious					
"	19	EXIT"					

MIDUSS Output -----
 MIDUSS version Version 2.25 rev. 473
 MIDUSS created Sunday, February 07, 2010
 Units used: in METRIC
 Job Folder: \\os-2k8\users_private\awitkinson\Documents\MIDUSS\317033
 Output filename: 317033 - Post 50 yr - Jan19.out
 Licensee name: Hewlett-Packard Company
 Date & Time last used: 1/22/2019 at 10:41:33 AM

31
 10.000
 360.000
 2400.000

32
 1535.380
 9.533
 0.825
 0.373
 360.000
 1.000

33
 130.451
 70.279
 130.451
 70.279

34
 0.037
 0.344
 0.000
 0.000

35
 0.344
 0.001
 1194.8
 394.000
 395.700
 394.000

36
 394.000
 394.050
 394.100
 394.150
 394.200
 394.250
 394.300
 394.350
 394.400
 394.450
 394.500
 394.550
 394.600
 394.650
 394.700
 394.750
 394.800
 394.850
 394.900
 394.950
 395.000
 395.050
 395.100
 395.150
 395.200
 395.250
 395.300
 395.350
 395.400
 395.450
 395.500

37
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

38
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

39
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

40
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

Overland Slope
 Pervious Area
 Pervious length
 Pervious slope
 Impervious Area
 Impervious slope
 Impervious length
 Impervious Manning 'n'
 Pervious SCS Curve No.
 Pervious Ia/S coefficient
 Pervious Initial abstraction
 Pervious Manning 'n'
 Pervious SCS Curve No.
 Pervious Ia/S coefficient
 Pervious Initial abstraction
 Impervious Manning 'n'
 Impervious SCS Curve No.
 Impervious Ia/S coefficient
 Impervious Initial abstraction
 Catchment 101
 Surface Area
 Time of concentration
 Rainfall depth
 Rainfall volume
 Rainfall losses
 Runoff depth
 Runoff volume
 Runoff coefficient
 Maximum flow
 HYDROGRAPH Add Runoff

0.000
 0.156
 10.000
 5.000
 0.084
 10.000
 5.000
 0.250
 78.000
 0.412
 0.100
 7.164
 0.015
 98.000
 0.824
 0.100
 0.518
 0.037
 0.317
 0.000
 0.156
 6.209
 192.164
 70.279
 109.63
 41.353
 28.926
 45.12
 0.412
 0.017
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

4
 Add Runoff
 0.037
 0.344
 0.000
 0.000

POND DESIGN
 Current peak flow
 Target outflow
 Hydrograph volume
 Number of stages
 Minimum water level
 Maximum water level
 Starting water level
 Keep Design Data: I = True; O = False
 Level Discharge
 Volume
 394.000
 0.000
 394.050
 0.01320
 0.3000
 394.100
 0.01400
 1.900
 394.150
 0.01480
 5.100
 394.200
 0.01550
 9.900
 394.250
 0.01620
 16.200
 394.300
 0.01680
 23.500
 394.350
 0.01750
 31.700
 394.400
 0.01810
 40.700
 394.450
 0.01870
 50.600
 394.500
 0.01920
 61.400
 394.550
 0.01980
 73.100
 394.600
 0.02030
 85.700
 394.650
 0.02090
 99.400
 394.700
 0.02140
 114.100
 394.750
 0.02190
 129.900
 394.800
 0.02240
 146.800
 394.850
 0.02290
 164.800
 394.900
 0.02330
 184.900
 394.950
 0.02380
 204.200
 395.000
 0.02430
 225.800
 395.050
 0.02480
 248.800
 395.100
 0.02530
 273.100
 395.150
 0.02580
 298.900
 395.200
 0.02630
 326.100
 395.250
 0.02680
 354.900
 395.300
 0.02730
 385.200
 395.350
 0.02780
 416.900
 395.400
 0.02830
 450.200
 395.450
 0.02880
 485.100
 395.500
 0.02930
 521.700

40
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

41
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

42
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

43
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

44
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

45
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

46
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

MIDUSS Output -----
 MIDUSS version Version 2.25 rev. 473
 MIDUSS created Sunday, February 07, 2010
 Units used: in METRIC
 Job Folder: \\os-2k8\users_private\awitkinson\Documents\MIDUSS\317033
 Output filename: 317033 - Post 50 yr - Jan19.out
 Licensee name: Hewlett-Packard Company
 Date & Time last used: 1/22/2019 at 10:41:33 AM

31
 10.000
 360.000
 2400.000

32
 1535.380
 9.533
 0.825
 0.373
 360.000
 1.000

33
 130.451
 70.279
 130.451
 70.279

34
 0.037
 0.344
 0.000
 0.000

35
 0.344
 0.001
 1194.8
 394.000
 395.700
 394.000

36
 394.000
 394.050
 394.100
 394.150
 394.200
 394.250
 394.300
 394.350
 394.400
 394.450
 394.500
 394.550
 394.600
 394.650
 394.700
 394.750
 394.800
 394.850
 394.900
 394.950
 395.000
 395.050
 395.100
 395.150
 395.200
 395.250
 395.300
 395.350
 395.400
 395.450
 395.500

37
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

38
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

39
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

40
 0.000
 0.084
 0.240
 3.400
 178.791
 70.279
 168.67
 31.219
 57.881
 93.74
 0.556
 0.037

MIDUSS Output ----- Version 2.25 rev. 473
MIDUSS version Sunday, February 07, 2010
MIDUSS created \\os-2k8\users_private\awilkinson\Documents\le METRIC
Units used: MIDUSS\317033
Job folder: 317033 - Post 100 yr - Jan19.out
Output filename: Hewlett-Packard Company
Company gmpb
Date & Time last used: 1/22/2019 at 9:53:30 AM
License name: 317033 - Post 100 yr - Jan19

31 TIME PARAMETERS
10.000 Time Storm
360.000 Max. Storm length
2400.000 Max. Hydrograph
32 STORM Chicago storm
1 Chicago storm
Coefficient A
9.944 Constant B
0.827 Exponent C
0.375 Fraction R
360.000 Duration
1.000 Time step multiplier
Maximum intensity 141.234 mm/hr
Total depth 76.839 mm
6 100hyd Hydrograph extension used in this file
33 CATCHMENT 100
1 Triangular SCS
1 Equal length
1 SCS method
20.000 Southerly portion of Subject Property
3.080 Total Area
30.000 Flow length
5.000 Overland Slope
2.464 Pervious Area
30.000 Pervious length
5.000 Pervious slope
0.616 Impervious Area
30.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning "n"
78.000 Pervious SCS Curve No.
0.444 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.876 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 100 Pervious 0.000 c.m./sec
Surface Area 2.464 Pervious Total Area hectare
Time of concentration 11.266 Pervious 0.616
Target outflow 197.993 Pervious 1.483
Hydrograph volume 166.218 Pervious 8.033
Number of stages 187.491 Pervious 187.491
Minimum water level 76.839 Pervious 76.839
Maximum water level 473.33 Pervious 473.33
Starting water level 2366.65 Pervious 2366.65
Level discharge 9.519 Pervious 9.519
Volume 67.320 Pervious 67.320
Keep design data: I = True; O = False
HYDROGRAPH Add Runoff 0.444 Pervious 0.444
0.530 Pervious 0.530
0.201 Pervious 0.201
4 HYDROGRAPH Add Runoff 0.372 Pervious 0.372
0.000 Pervious 0.000
0.000 Pervious 0.000

33 CATCHMENT 101
1 Triangular SCS
1 Equal length
1 SCS method
35.000 Southerly Off-Site Lands
0.240 % Impervious
10.000 Total Area
Flow length

5.000 Overland Slope
0.156 Pervious Area
10.000 Pervious length
5.000 Pervious slope
0.084 Impervious Area
10.000 Impervious length
5.000 Impervious slope
0.250 Pervious Manning "n"
78.000 Pervious SCS Curve No.
0.437 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
7.164 Pervious Initial abstraction
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.827 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction

Catchment 101 Pervious 0.000 c.m./sec
Surface Area 0.356 Pervious Total Area hectare
Time of concentration 5.828 Pervious 0.984
Target outflow 190.564 Pervious 5.275
Hydrograph volume 163.892 Pervious 178.121
Number of stages 64.54 Pervious 76.839
Minimum water level 119.87 Pervious 64.54
Maximum water level 43.234 Pervious 184.41
Starting water level 33.606 Pervious 32.768
Level discharge 52.42 Pervious 44.071
Volume 0.437 Pervious 105.77
Keep design data: I = True; O = False
HYDROGRAPH Add Runoff 0.020 Pervious 0.427
0.028 Pervious 0.028
4 Add Runoff 0.404 Pervious 0.000
0.000 Pervious 0.000

4.040 Current peak flow c.m./sec
0.001 Target outflow c.m./sec
1360.5 Hydrograph volume c.m.
37 Number of stages metre
394.000 Minimum water level metre
395.790 Maximum water level metre
394.000 Starting water level metre
0 Level discharge Volume
394.000 0.000
394.050 0.01320 0.3000
394.100 0.01400 1.900
394.150 0.01480 5.100
394.200 0.01550 9.900
394.250 0.01620 16.200
394.300 0.01680 23.500
394.350 0.01750 31.700
394.400 0.01810 40.700
394.450 0.01870 50.600
394.500 0.01920 61.400
394.550 0.01980 73.100
394.600 0.02030 85.700
394.650 0.02090 99.400
394.700 0.02140 114.100
394.750 0.02190 129.900
394.800 0.02240 146.800
394.850 0.02290 164.800
394.900 0.02330 183.900
394.950 0.02360 204.200
395.000 0.02380 225.800
395.050 0.02410 248.800
395.100 0.02430 273.100
395.150 0.02450 298.900
395.200 0.02470 326.100
395.250 0.02490 354.900
395.300 0.02510 385.200
395.350 0.02530 416.900
395.400 0.02540 450.200
395.450 0.02540 485.100
395.500 0.02540 521.700

317033 - Post 100 yr - Jan19

317033 - Post 100 yr - Jan19
 0.086 0.000 0.086 0.086

395.550 0.06510 559.800
 395.600 0.06600 599.600
 395.650 0.1240 641.000
 395.700 0.2294 684.100
 395.750 0.3639 728.900
 395.790 0.4927 766.000
 Peak outflow 0.122 c.m/sec
 Maximum level 395.649 metre
 Maximum storage 639.921 c.m
 Centroidal lag 4.866 hours
 0.042 0.404 0.122 0.000 c.m/sec
 HYDROGRAPH " Combine 1"
 1 Node #"
 Easterly Outlet" 0.122 c.m/sec
 Maximum flow 1360.548 c.m
 Hydrograph volume 0.122 0.122
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary" 0.000 0.122 0.122
 CATCHMENT "200"
 1 Triangular SCS"
 1 Equal length"
 1 SCS method"
 200 Northwesterly Portion of Subject Property"
 % Impervious" 10.000
 Total Area" 0.980 hectare
 Flow length" 120.000 metres
 Overland Slope" 0.882
 Pervious Area" 120.000
 Pervious length" 120.000
 Pervious slope" 10.000
 Impervious Area" 0.098
 Impervious length" 120.000
 Impervious slope" 10.000
 Pervious Manning "n" 0.250
 Pervious SCS Curve No. " 78.000
 Pervious Runoff coefficient" 0.445
 Pervious Ia/S coefficient" 0.100
 Pervious Initial abstraction" 0.164
 Impervious Manning "n" 0.1015
 Impervious SCS Curve No. " 98.000
 Impervious Runoff coefficient" 0.913
 Impervious Ia/S coefficient" 0.100
 Impervious Initial abstraction" 0.518
 0.086 0.000 0.122 c.m/sec
 Catchment 200 Pervious Total Area " hectare
 Surface Area 0.882 0.980
 Time of concentration 2.767 2.767
 21.023 17.633
 211.435 203.352
 211.435 167.909
 76.839 76.839
 76.839 76.839
 677.72 75.30
 753.02 753.02
 42.657 6.676
 34.183 39.059
 301.49 70.163
 301.49 37.781
 0.445 68.76
 0.913 370.25
 0.076 0.492
 0.030 0.086
 HYDROGRAPH Add Runoff " 0.122 c.m/sec
 4 Add Runoff " 0.122
 0.086 0.086 0.122 0.122
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.086 0.086 0.122
 HYDROGRAPH " Combine 2"
 6 Combine " Combine 2"
 2 Node #"
 Northwesterly Outlet"
 Maximum flow 0.086 c.m/sec
 Hydrograph volume 370.249 c.m
 0.086 0.086
 HYDROGRAPH Start - New Tributary"
 2 Start - New Tributary" 0.086 0.086

CATCHMENT 300"
 1 Triangular SCS"
 1 Equal length"
 1 SCS method"
 300 Northwesterly Portion of Subject Property"
 % Impervious" 10.000
 Total Area" 0.220 hectare
 Flow length" 50.000 metres
 Overland Slope" 0.198
 Pervious Area" 10.000
 Pervious length" 50.000
 Pervious slope" 10.000
 Impervious Area" 0.022
 Impervious length" 50.000
 Impervious slope" 10.000
 Pervious Manning "n" 0.250
 Pervious SCS Curve No. " 78.000
 Pervious Runoff coefficient" 0.443
 Pervious Ia/S coefficient" 0.100
 Pervious Initial abstraction" 0.164
 Impervious Manning "n" 0.1015
 Impervious SCS Curve No. " 98.000
 Impervious Runoff coefficient" 0.883
 Impervious Ia/S coefficient" 0.100
 Impervious Initial abstraction" 0.518
 0.025 0.000 0.086 0.086
 Catchment 300 Pervious Total Area " hectare
 Surface Area 0.198 0.220
 Time of concentration 12.433 10.476
 199.720 166.237
 76.839 76.839
 152.14 16.90
 42.771 8.955
 37.468 39.390
 67.468 67.874
 34.183 37.449
 0.443 14.93
 0.443 0.883
 0.021 0.007
 HYDROGRAPH Add Runoff " 0.086 c.m/sec
 4 Add Runoff " 0.086
 0.025 0.025 0.086 0.086
 HYDROGRAPH Copy to Outflow"
 8 Copy to Outflow" 0.025 0.025 0.086
 HYDROGRAPH " Combine 3"
 6 Combine " Combine 3"
 3 Node #"
 Northwesterly Outlet"
 Maximum flow 0.025 c.m/sec
 Hydrograph volume 82.388 c.m
 0.025 0.025
 START/RE-START TOTALS 300"
 3 Runoff Totals on EXIT"
 Total Catchment area 4.520 hectare
 Total Impervious area 0.820 hectare
 Total % impervious 18.142

MIDUSS Output ----- Version 2.25 Rev. 473
MIDUSS version Sunday, February 07, 2010
MIDUSS created Te METRIC
Units Used: \\os-2k8\users_private\awilkinson\Documents\MIDUSS\317033\gmbp
Job Folder: 317033 - Post REG - Jan19.out
Output filename: Hewlett-Packard Company
Licensee name: 1/22/2019 at 9:55:58 AM

TIME PARAMETERS

31 60.000 Max. Storm length" 2.028
3600.000 Max. Hydrograph" 2.028
5 STORM HISTORIC 2.028
2880.000 Duration" 2.028
48.000 Rainfall intensity values" 2.028
2.028 2.028
2.028 2.028
2.028 2.028
2.028 2.028
2.028 2.028
2.028 2.028
2.028 2.028
17.000 6.000 4.000 6.000 13.000
53.000 13.000 13.000
Maximum intensity 53.000 mm/hr
Total depth 284.998 mm

CATCHMENT 101

1 Equal length" 0.029
1 SCS method" 0.156
1 Easterly Off-site Lands" 2473.122
35.000 % Impervious" 2303.773
10.000 Total Area" 284.998
10.000 Flow length" 444.60
5.000 Overland Slope" 87.470
10.000 Pervious Area" 41.306
5.000 Pervious length" 197.528
0.084 Impervious Area" 213.686
10.000 Impervious length" 512.85
5.000 Impervious slope" 0.750
0.250 Pervious Manning "n" 0.011
78.000 Pervious SCS Curve No. " 0.029
0.693 Pervious Runoff coefficient" 0.361
0.100 Pervious Ia/S coefficient" 0.000
7.164 Pervious Initial abstraction" 0.000
0.015 Impervious Manning "n" 0.018
98.000 Impervious SCS Curve No. " 0.156
0.855 Impervious Runoff coefficient" 0.029
0.100 Impervious Ia/S coefficient" 0.361
0.518 Impervious Initial abstraction" 0.000

Catchment 101 Pervious Total Area " 0.000 c.m/sec"
Surface Area 0.156 hectare "
Time of concentration 2473.122 minutes "
Rainfall depth 284.998 mm "
Rainfall volume 444.60 c.m "
Rainfall losses 87.470 mm "
Runoff depth 41.306 mm "
Runoff volume 197.528 c.m "
Runoff coefficient 0.693 c.m/sec "
Maximum flow 308.14 c.m/sec "
HYDROGRAPH Add Runoff " 0.018 0.029 0.000

4 Add Runoff " 0.029 0.390 0.000 0.000"
54 POND DESIGN"
0.390 Current peak flow c.m/sec "
0.287 Target outflow c.m/sec "
7276.7 Hydrograph volume c.m "
37. Number of stages metre "
394.000 Minimum water level metre "
395.790 Maximum water level metre "
394.000 Starting water level metre "
0 Keep Design Data: I = True; O = False

Level Discharge Volume "
394.000 0.000 0.000 "
394.050 0.01320 0.3000 "
394.100 0.01400 1.900 "
394.150 0.01480 5.100 "
394.200 0.01550 9.900 "
394.250 0.01620 16.200 "
394.300 0.01680 23.500 "
394.350 0.01750 31.700 "
394.400 0.01810 40.700 "
394.450 0.01870 50.600 "
394.500 0.01920 61.400 "
394.550 0.01980 73.100 "
394.600 0.02030 85.700 "
394.650 0.02090 99.400 "
394.700 0.02140 114.100 "
394.750 0.02190 129.900 "
394.800 0.02240 146.800 "
394.850 0.02290 164.800 "
394.900 0.02330 183.900 "
394.950 0.02380 204.200 "
395.000 0.05510 225.800 "
395.050 0.05610 248.800 "
395.100 0.05700 273.100 "
395.150 0.05800 298.900 "
395.200 0.05890 326.100 "

1 Equal length" 0.029
1 SCS method" 0.156
1 Easterly Off-site Lands" 2473.122
35.000 % Impervious" 2303.773
10.000 Total Area" 284.998
10.000 Flow length" 444.60
5.000 Overland Slope" 87.470
10.000 Pervious Area" 41.306
5.000 Pervious length" 197.528
0.084 Impervious Area" 213.686
10.000 Impervious length" 512.85
5.000 Impervious slope" 0.750
0.250 Pervious Manning "n" 0.011
78.000 Pervious SCS Curve No. " 0.029
0.693 Pervious Runoff coefficient" 0.361
0.100 Pervious Ia/S coefficient" 0.000
7.164 Pervious Initial abstraction" 0.000
0.015 Impervious Manning "n" 0.018
98.000 Impervious SCS Curve No. " 0.156
0.855 Impervious Runoff coefficient" 0.029
0.100 Impervious Ia/S coefficient" 0.361
0.518 Impervious Initial abstraction" 0.000

Catchment 101 Pervious Total Area " 0.000 c.m/sec"
Surface Area 0.156 hectare "
Time of concentration 2473.122 minutes "
Rainfall depth 284.998 mm "
Rainfall volume 444.60 c.m "
Rainfall losses 87.470 mm "
Runoff depth 41.306 mm "
Runoff volume 197.528 c.m "
Runoff coefficient 0.693 c.m/sec "
Maximum flow 308.14 c.m/sec "
HYDROGRAPH Add Runoff " 0.018 0.029 0.000

4 Add Runoff " 0.029 0.390 0.000 0.000"
54 POND DESIGN"
0.390 Current peak flow c.m/sec "
0.287 Target outflow c.m/sec "
7276.7 Hydrograph volume c.m "
37. Number of stages metre "
394.000 Minimum water level metre "
395.790 Maximum water level metre "
394.000 Starting water level metre "
0 Keep Design Data: I = True; O = False

Level Discharge Volume "
394.000 0.000 0.000 "
394.050 0.01320 0.3000 "
394.100 0.01400 1.900 "
394.150 0.01480 5.100 "
394.200 0.01550 9.900 "
394.250 0.01620 16.200 "
394.300 0.01680 23.500 "
394.350 0.01750 31.700 "
394.400 0.01810 40.700 "
394.450 0.01870 50.600 "
394.500 0.01920 61.400 "
394.550 0.01980 73.100 "
394.600 0.02030 85.700 "
394.650 0.02090 99.400 "
394.700 0.02140 114.100 "
394.750 0.02190 129.900 "
394.800 0.02240 146.800 "
394.850 0.02290 164.800 "
394.900 0.02330 183.900 "
394.950 0.02380 204.200 "
395.000 0.05510 225.800 "
395.050 0.05610 248.800 "
395.100 0.05700 273.100 "
395.150 0.05800 298.900 "
395.200 0.05890 326.100 "

"	395.250	0.05990	354.900"	0.106	0.106	Northwesterly Outlet"	
"	385.200	0.06080	385.200"	0.106	0.106	Maximum Flow	
"	395.350	0.06170	416.900"	0.106	0.106	Hydrograph volume	
"	395.400	0.06260	450.200"	0.106	0.106	HYDROGRAPH Start - New Tributary"	
"	395.450	0.06350	485.100"	0.106	0.106	2 Start - New Tributary"	
"	395.500	0.06430	521.700"	0.106	0.106	CATCHMENT 300"	
"	395.550	0.06510	559.800"	0.106	0.106	1 Triangular SCS"	
"	395.600	0.06600	599.600"	0.106	0.106	1 Equal length"	
"	395.650	0.1240	641.000"	0.106	0.106	1 SCS method"	
"	395.700	0.2294	684.100"	0.106	0.106	300 Northeastly Portion of Subject Property"	
"	395.750	0.3656	728.900"	0.106	0.106	10.000 % Impervious	
"	395.790	0.4927	766.000"	0.106	0.106	0.220 Total Area"	
"						50.000 Flow length"	
"						10.000 Overland Slope"	
"						0.198 Pervious Area"	
"						50.000 Pervious length"	
"						10.000 Pervious slope"	
"						0.022 Impervious Area"	
"						50.000 Impervious length"	
"						10.000 Impervious slope"	
"						0.250 Pervious Manning "n"	
"						78.000 Pervious SCS Curve No."	
"						0.758 Pervious Runoff coefficient"	
"						0.100 Pervious Ia/S coefficient"	
"						7.164 Pervious Initial abstraction"	
"						0.015 Pervious Manning "n"	
"						98.000 Impervious SCS Curve No."	
"						0.846 Impervious Runoff coefficient"	
"						0.100 Impervious Ia/S coefficient"	
"						0.518 Impervious Initial abstraction"	
"						0.025 0.000 0.106	
"						Catchment 300	
"						Surface Area	
"						Time of concentration	
"						Time to Centroid	
"						Rainfall depth	
"						Rainfall volume	
"						Rainfall losses	
"						Runoff depth	
"						Runoff volume	
"						Runoff coefficient	
"						Maximum Flow	
"						HYDROGRAPH Add Runoff "	
"						4 Add Runoff "	
"						0.025 0.025 0.106	
"						HYDROGRAPH Copy to Outflow"	
"						8 Copy to Outflow"	
"						0.025 0.025 0.025	
"						HYDROGRAPH Combine 3"	
"						6 Combine " Combine	
"						3 Node #"	
"						Northwesterly Outlet"	
"						Maximum Flow	
"						Hydrograph volume	
"						0.025 0.025 0.025	
"						START/REG-START TOTALS 300"	
"						3 Runoff Totals on EXIT"	
"						Total Catchment area	
"						Total Impervious area	
"						Total % Impervious	
"						EXIT"	
"						4.520 hectare"	
"						0.820 hectare"	
"						18.142	

"	395.250	0.05990	354.900"	0.364	0.364	Peak outflow	
"	385.200	0.06080	385.200"	395.756	0.364	Maximum level	
"	395.350	0.06170	416.900"	734.121	0.364	Maximum storage	
"	395.400	0.06260	450.200"	41.791	0.364	Centroidal lag	
"	395.450	0.06350	485.100"	0.364	0.000	C.m/sec"	
"	395.500	0.06430	521.700"	0.364	0.364	HYDROGRAPH Combine 1"	
"	395.550	0.06510	559.800"	0.364	0.364	6 Combine " Combine	
"	395.600	0.06600	599.600"	0.364	0.364	1 Node #"	
"	395.650	0.1240	641.000"	0.364	0.364	Easterly Outlet"	
"	395.700	0.2294	684.100"	7244.684	0.364	Maximum Flow	
"	395.750	0.3656	728.900"	0.364	0.364	Hydrograph volume	
"	395.790	0.4927	766.000"	0.364	0.364	0.029 0.390 0.364	
"						HYDROGRAPH Start - New Tributary"	
"						2 Start - New Tributary"	
"						0.029 0.000 0.364	
"						CATCHMENT 200"	
"						1 Triangular SCS"	
"						1 Equal length"	
"						1 SCS method"	
"						200 Northwestly Portion of Subject Property"	
"						10.000 % Impervious	
"						0.980 Total Area"	
"						120.000 Flow length"	
"						10.000 Overland Slope"	
"						0.882 Pervious Area"	
"						120.000 Pervious length"	
"						10.000 Pervious slope"	
"						0.098 Impervious Area"	
"						120.000 Impervious length"	
"						10.000 Impervious slope"	
"						78.250 Pervious Manning "n"	
"						0.762 Pervious SCS Curve No."	
"						0.100 Pervious Runoff coefficient"	
"						7.164 Pervious Ia/S coefficient"	
"						0.015 Pervious Initial abstraction"	
"						98.000 Impervious SCS Curve No."	
"						0.858 Impervious Runoff coefficient"	
"						0.100 Impervious Ia/S coefficient"	
"						0.518 Impervious Initial abstraction"	
"						0.106 0.000 0.364	
"						Catchment 200	
"						Surface Area	
"						Time of concentration	
"						Time to Centroid	
"						Rainfall depth	
"						Rainfall volume	
"						Rainfall losses	
"						Runoff depth	
"						Runoff volume	
"						Runoff coefficient	
"						Maximum Flow	
"						HYDROGRAPH Add Runoff "	
"						4 Add Runoff "	
"						0.106 0.106 0.364	
"						HYDROGRAPH Copy to Outflow"	
"						8 Copy to Outflow"	
"						0.106 0.106 0.106	
"						HYDROGRAPH Combine 2"	
"						6 Combine " Combine	
"						2 Node #"	

GLEN ALLAN SUBDIVISION
Municipality of Mapleton
Our File: 317033

January 2019

WATER BUDGET CALCULATIONS

EASTERLY OUTLET										
SOIL GROUP BC - HURON LOAM										
	Pre-Development Conditions				Pre-Dev. Total	Post-Development Conditions				
	CATCH. 10, 11 & Off-Site Lands		Catch. 100, 101			Off-Site Lands		Off-Site Lands		Post-Dev Total
	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious
Annual Precipitation (mm)	940	940	940	940						
Annual Evapotranspiration (mm)	215	541	215	531						
Available for Recharge & Runoff	725	399	725	410						
Total Area (ha)	8.80		3.32		6.24		6.24		9.56	
Area (ha)	0.06	8.74	0.70	2.62	0.00	6.24	0.00	6.24	9.56	9.56
Area (m ²)	600	87400	7000	26200	0	62400	0	62400	95600	95600
Annual Infiltration:										
Infiltration Component Value (mm)	0	88	0	82	0	88	0	88	0	2148
Infiltration Volume (m ³)	0	7691	0	2148	0	7691	0	7691	0	2148
Annual Runoff:										
Runoff Component Value (mm)	725	311	725	328	725	311	725	311	725	311
Runoff Volume prior to SWM Control Consideration (m ³)	435	27181	435	8581	5075	19406	0	19406	0	19406
Runoff Volume not subject to VFS Reduction (m ³)	435	27181	2248	0	2248	0	0	19406	0	21654
Runoff Volume subject to VFS Reduction (m ³)	0	0	2828	8581	2828	0	0	0	0	11408
Runoff Volume Reduction via VFS (~37.5%)	0	0	1060	3218	1060	0	0	0	0	4278
Runoff Volume Draining from Site (m ³)	435	27181	4015	5363	4015	5363	0	19406	0	28784
Summary:										
Total Runoff (m ³ /year)	435	27181	4015	5363	4015	5363	0	19406	0	28784
Total Recharge (m ³ /year)	0	7691	0	2148	0	2148	0	5491	0	7640
										Percent Difference
										4%
										-1%