

MAMTA HOMES

FUNCTIONAL SERVICING AND STORMWATER MANAGEMENT REPORT

PROPOSED TOWNHOUSE DEVELOPMENT
645 MARTINS STREET
TOWN OF MOUNT FOREST
TOWNSHIP OF WELLINGTON NORTH

FEBRUARY 2023

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TABLE OF CONTENTS

1.	INTRODUCTION	1
1.1	Location	1
1.2	Development Proposal	1
2.	WATER DISTRIBUTION SYSTEM	3
2.1	Design Criteria.....	3
2.2	Water Consumption	3
2.3	Fire Flow.....	3
2.4	Design Flow Rates	4
2.5	Watermain Configuration	4
3.	SANITARY SERVICING.....	5
3.1	Design Criteria.....	5
3.2	Design Flow Rates	5
3.3	Sanitary Sewer Configuration.....	5
4.	STORMWATER MANAGEMENT AND SITE DRAINAGE	6
4.1	Existing drainage Conditions	6
4.2	Proposed drainage Conditions.....	6
4.3	Stormwater Modeling.....	6
4.3.1	Pre-Development COnditions	6
4.3.2	Post Development Conditions.....	7
5.	GRADING, EROSION & SEDIMENT CONTROL	10
5.1	Construction Stage	10
5.2	Site Development.....	Error! Bookmark not defined.
6.	TRAFFIC AND ROADS.....	11
7.	UTILITIES.....	12
7.1	Streetlights.....	12

7.2	Electricity	12
7.3	Natural Gas	12
7.4	Telephone/ Cable TV/ Internet.....	12

LIST OF TABLES

Table 1 - Proposed Water Demands	3
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LIST OF FIGURES

Figure 1 - Regional Location Map	2
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APPENDICES

Appendix A – Drawings

03909-CP – Concept Plan

03909-SSP – Site Servicing Plan

03909-SWM1 – Pre-Development Drainage Areas

03909-SWM2 – Post Development Drainage Areas

Appendix B – Pre Development Model and Output

Appendix C – Post Development Model and Output

1. INTRODUCTION

Cobide Engineering Inc. was retained by Mamta Developments Inc. to prepare a Functional Servicing and Stormwater Management Report to support the Draft Plan Approval for a proposed Condominium townhouse development located in Mount Forest.

1.1 LOCATION

The proposed development is located at 645 Martin Street in Mount Forest, within the Township of Wellington North, County of Wellington (described herein as the “site”). The proposed site will have access from Cork Street. A Regional Location Map is included as Figure 1.

1.2 DEVELOPMENT PROPOSAL

The overall property area is 1.98 hectares (4.89 acres) in size. The portion fronting onto Martin Street is intended to be developed under a separate application. The remainder of the property, 1.62 hectares (4.0 acres), is planned to be developed into a private townhouse site consisting of nine (9) townhouse blocks, including internal roadways, parking, sidewalks, and landscaped green space.

There will be private road throughout the development with provisions for fire truck access. Two entrances will be provided off Cork Street, one at the north end and one at the south end.

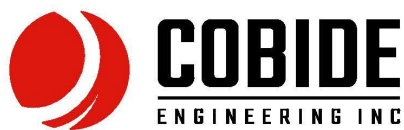
The Site Plan showing the overall configuration of the development has been included in Appendix A and noted as Drawing 03909-C1.

The proposed development is located within the “Residential” designation of both the current County and Local Official Plans. The property is currently zoned R2(H) Medium Density Residential with a Holding and is in the process of being re-zoned to remove the holding to permit construction of homes.

Functional Servicing and Stormwater Management Report
Martin Street Townhouses – Mount Forest
Township of Wellington North



MAP SOURCE - MTO ROAD MAP



517 - 10th STREET, Hanover, Ontario N4N 1R4
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ORIGINAL SHEET - 8 x 11

Client/Project

PROPOSED TOWNHOUSE DEVELOPMENT
 645 MARTIN STREET, MOUNT FOREST
 TOWNSHIP OF WELLINGTON NORTH
 COUNTY OF WELLINGTON
 FUNCTIONAL SERVICING AND STORMWATER
 MANAGEMENT REPORT

Figure No.

1

Title

REGIONAL LOCATION MAP

Figure 1 – Regional Location Map

2. WATER DISTRIBUTION SYSTEM

The water distribution system will be sized based on the existing conditions at the connection to the municipal system and the subdivisions demands which are determined by the Ministry of the Environment, Conservation and Parks (MECP) Design Guidelines for Drinking-Water Systems (2008).

2.1 DESIGN CRITERIA

The water distribution system will be design in accordance MECP guidelines which state the system “should be designed to satisfy the greater of the following demands:

- *Maximum day demand plus fire flow; or,*
- *Peak hour demand*

The maximum day demand and peak hour demand are based on the projected water consumption from the development and the fire flow is based on the type of the development.

Based on MECP guidelines, the minimum pressure at ground level at all points in the distribution system under maximum day demand plus fire flow conditions are to be 140 kPa (20 psi). The normal operation pressure should be between 350 kPa (50 psi) to 480 kPa (70 psi). There shall be no point in the distribution system that has a normal operating pressure of less than 275 kPa (40 psi). The maximum pressure in the pipe cannot exceed 700 kPa (100 psi).

2.2 WATER CONSUMPTION

The system will be designed based on a domestic water demand of 450 L/cap/day. The peaking factors will be derived from Table 3-1 of the MECP Design Guidelines. Based on the projected population of 48 people the peaking factor for the maximum day demand will be 2.75 and the peaking factor for the peak hour demand will be 4.13.

Table 1 below summarizes the projected maximum day and peak hour demands for all the units within the proposed development:

Table 1 - Proposed Water Demands

Demand	Population	Consumption (L/cap/day)	Peaking Factor	Peak Rate (L/day)	Peak Rate (L/s)
Maximum Day	85	450	2.75	105,188	1.22
Peak Hour	85	450	4.13	157,973	1.83

The system should be capable of supplying a minimum of 1.83 L/s of water to meet the peak hour demand of the entire proposed development.

2.3 FIRE FLOW

The AWWA *Manual of Water Supply Practices M31 – Distribution System Requirements for Fire Protection* and the Fire Underwriters Survey document *Water Supply for Public Fire Protection* will determine the required fire flows.

The fire flows are dependent upon many factors including the type of construction materials, building height and density of the development.

The Township's "Mount Forest Sanitary and Water Servicing Technical Update" completed in 2021 identifies the minimum target fire flow capabilities of 50 L/s for low density residential and 75 L/s for medium density residential development.

2.4 WATERMAIN CONFIGURATION

The proposed watermain will be connected to the municipal system at the proposed entrance from Cork Street and will loop around and through the site. The new watermain for the development will be 150mm diameter and will run east into the site complete with a valve at the property line to allow for isolation from the municipal water system. A 150mm diameter watermain is required to provide adequate fire coverage.

Fire hydrants will be placed throughout the site at a spacing no greater than 90m to provide fire department connections. A blow-off will be provided at each end of the turn-around where the watermain terminates.

A 25mm diameter water service will be provided to each unit for domestic water distribution.

A drawing showing the proposed watermain distribution system has been included in Appendix A as Drawing 03909-C2.

3. SANITARY SERVICING

The sanitary sewer system will be sized based on the existing conditions at the connection to the municipal system and the demands of the proposed development which are determined by the MECP Design Guidelines for Sewage Works (2008).

3.1 DESIGN CRITERIA

The sanitary sewer system will be design in accordance MECP guidelines.

The sanitary sewer will be designed to convey the projected peak flow based upon the projected population of the development as well as extraneous flows.

3.2 DESIGN FLOW RATES

The sanitary sewer will be designed based on a peak flow of 450 L/cap/day and a peaking factor of 4.0 for the size and type of the development. Based on a population of 85, the projected residential flow is 1.77 L/s. The development will contribute 1.98 ha of extraneous flows to the sanitary sewer. This will contribute 0.55 L/s to the flow. Therefore, the peak flow plus infiltration from the development is 2.32 L/s.

3.3 SANITARY SEWER CONFIGURATION

There is an existing 200mm diameter PVC sanitary sewer on Cork Street that flows north. The site will connect into the existing sanitary manhole located at the north entrance and a new sanitary manhole will be installed on the sewer for the south entrance. Each unit will be serviced with a 125mm sanitary service.

All sanitary sewers within the development are proposed to be 200mm diameter PVC pipe. Where possible the minimum slope considered will be 1.0% to maintain a minimum velocity at full flow to prevent sediment deposition and blockages however due to the depth of the existing sanitary sewer on Cork Street, there are some sections that are required to be less than 1.0%.

A drawing showing the proposed sanitary collection network has been included in Appendix A as Drawing 03909-SSP.

4. STORMWATER MANAGEMENT AND SITE DRAINAGE

The stormwater management and site drainage for the proposed townhouse development must comply with the current standards of the Municipality, Conservation Authority and MECP Design Guidelines. The following provides a summary of the stormwater management and site drainage plan for the development.

The recommended stormwater strategy for the development requires the post development peak flows from the property to be less than or equal to the pre-development peak flow conditions with the stormwater discharging to the existing storm sewer on Cork Street.

4.1 EXISTING DRAINAGE CONDITIONS

The current site topography provides overland drainage from northeast to southwest draining into an existing catch basin in the ditch on the east side of Cork Street. There is existing runoff from the neighbouring lots that discharges onto this site.

4.2 PROPOSED DRAINAGE CONDITIONS

The intent of the stormwater drainage plan for the townhouse development is proposed to collect the runoff from the site and convey the runoff away from neighbouring residential lots.

Since the development will increase the overall impervious area of the subject property, stormwater runoff will need to be over-controlled in order to reduce the peak flows to pre-development conditions. This will be achieved through the construction of a stormwater management pond and underground storage chambers. The outlet from the underground storage chambers will connect to the existing storm sewer on Cork Street at the proposed entrance to the Sunvale Homes Subdivision.

The existing rear yard swale on the lots at the corner of Cork Street and Martin Street will be connected to the existing storm sewer stub at the intersection. The drainage design has been completed based on Cork Street being urbanized.

4.3 STORMWATER MODELING

The stormwater modeling for the pre and post development scenarios was completed using PCSWMM and the 6 hour SCS rainfall distribution. Rainfall data was obtained from the Mount Forest weather station.

According to the Soils Survey of Wellington County, the local native soils are reported to be a mix of Listowel Silt Loam and Harriston Silt Loam which are classified at Hydrologic Soil Group BC.

The existing storm sewer on Cork Street that outlets to the unnamed watercourse is considered to be Discharge Point #1 for the purposes of this report. The storm sewer at the intersection of Cork Street and Martin Street will be considered Discharge Point #2.

4.3.1 PRE-DEVELOPMENT CONDITIONS

A total catchment area of 2.57 ha was used to determine the pre-development peak flows for the design storm events at the property boundary.

Catchment 101 consists of the development lands as well as some upstream lands that discharge across the site. The total area is 2.07 ha. Catchment 101 is considered to outlet to Discharge Point #1.

Catchment 102 consists of a portion of the lands to the south that outlet to the existing catchbasin in the ditch on Cork Street. The total area is 0.50 ha. Catchment 102 is considered to outlet to Discharge Point #1.

The pre-development catchment areas are included in Appendix A as drawing 03909-SWM1.

4.3.2 POST DEVELOPMENT CONDITIONS

The proposed development of the site will include the construction of townhouses as well as paving the associated interior roadway, driveways, and parking areas. Grading of the site and internal storm sewers will direct the majority of the post-development run-off from the site into proposed underground storage chambers and a stormwater management pond. A small portion of the street townhouses will outlet directly to Cork Street as the grading does not allow for the units to outlet to the pond and chambers. The stormwater pond and storage tanks will work in conjunction to provide storage for all post-development flows and control the discharge rate of the site's stormwater into the existing Municipal infrastructure.

Catchment 201 is the development area that can be conveyed to the stormwater system. The total area of Catchment 201 is 1.74 ha. Catchment 201 is considered to outlet to Discharge Point #1.

Catchment 202 is a portion of the 4 unit street townhouses that cannot be conveyed to the pond. The total area of Catchment 202 is 0.06 ha. Catchment 202 is considered to outlet to Discharge Point #1.

Catchment 203 is a portion of the 3 unit street townhouses that cannot be conveyed to the pond. The total area of Catchment 203 is 0.06 ha. Catchment 203 is considered to outlet to Discharge Point #1.

Catchment 204 is a portion of the deep lots fronting Martin Street. This area will be conveyed through the development site. The total area of Catchment 204 is 0.37 ha. Catchment 204 is considered to outlet to Discharge Point #1.

Catchment 205 is a portion of the lots fronting Martin Street. This area will be conveyed to Cork Street but not through the development site. The total area of Catchment 205 is 0.39 ha. Catchment 205 is considered to outlet to Discharge Point #2.

Based on the post development stormwater flows for this catchment area, there will be a requirement to reduce the peak stormwater flow from the catchment areas in order to meet the pre-development peak flow conditions.

The stormwater management facility and outlet structure have been designed to control peak runoff rates as well as conform to MECP best practices.

In order to provide the above required volumes and discharges, the following SWM Facility geometry is being proposed:

Table 6.1 – SWM Facility Geometry

SWM FACILITY	DETAILED DESIGN
Side Slope	3:1
SWM Facility Bottom	410.66
Top Elevation	411.42
High Water Elevation	411.23

Also included as part of the stormwater management system is the ADS Stormtech SC-740 Chambers. There will be 11 rows of chambers with varying numbers per row. The bottom of the chambers will be at an elevation of 409.45 with 150mm of stone below the chambers.

The outlet configuration for the SWM Facility will be as follows:

- A 300mm diameter storm sewer with an outlet elevation of 409.41m c/w a 200mm orifice;

4.3.3 SWM FACILITY PERFORMANCE

Below is a summary of the hydraulic performance of the stormwater SWM Facility during the various storm events.

Table 6.2 – SWM Facility Performance

RETURN PERIOD	ELEVATION (m)	STORAGE (m ³)	DISCHARGE (l/s)
2 Year	409.81	230	49
5 Year	409.99	308	63
25 Year	410.82	421	104
50 Year	410.96	475	109
100 Year	411.08	539	114
MTO 100 Year	411.23	634	119

4.4 MODELLING RESULTS

Based upon the above outlet structure, the following summarizes the pre-development and post development peak flows to the two (2) discharge points.

Table 6.3 - Peak Flow Summary

RETURN PERIOD	DISCHARGE POINT #1 (l/s)		DISCHARGE POINT #2 (l/s)	
	PRE	POST	PRE	POST
	2 Year	51	59	0
5 Year	81	75	0	55
25 Year	140	114	0	81
50 Year	169	131	0	93
100 Year	199	140	0	105
MTO 100 Year	248	149	0	123

As seen in the table above, all storm events with the exception of the 2 year design storm event at Discharge Point #1 have a lower peak flow than the pre development design storm event. The exceedance in the 2 year design storm is not expected to cause any issues downstream as the storm sewers were designed for the 5 year storm event and post development modelling for the neighbouring subdivisions show the channel can convey the design storm events modelled with causing flooding concerns.

While the peak flows for Discharge Point #2 exceed the pre development flows in all instances, this is not expected to be a concern as the storm sewer stub on Cork Street that this area will outlet to was originally designed for a much larger catchment that can no longer be conveyed through this storm sewer as the SVCA is requiring it continue to outlet to the existing ditch as it is providing inflow to the watercourse. Therefore the downstream storm sewer that was proposed to convey that runoff now has significant extra capacity for the Catchment 205 runoff.

The post-development catchment area is included as Appendix A as drawing 03909-SWM2.

5. GRADING, EROSION & SEDIMENT CONTROL

Erosion and sediment controls shall meet the requirements of the most recent version of the MECP *Stormwater Management Planning and Design Manual* at the time of construction.

5.1 CONSTRUCTION STAGE

Prior to the start of construction, appropriate sediment control facilities are to be in place. Following are details regarding erosion and sediment control that are to be implemented:

- Light duty siltation fencing will be installed at any development grading limits where runoff may discharge from the site.
- It is proposed that the pond be constructed first to act as a sedimentation basin.
- Placement of temporary straw check dams within swales and any other locations where a concentrated flow of runoff may occur. All proposed drainage swales are to be seeded during construction;
- Installation of filter cloth under all new and existing catchbasin grates until paving of the subdivision streets is completed;
- Mud mats will be placed at construction accesses to keep public roadways free from debris during the construction period.
- Re-vegetate all disturbed areas after underground and surface works have been constructed.

Prior to removal of sediment control facilities, ensure that sediment that may have accumulated has been removed.

Once the area has been stabilized, the silt fencing can be removed.

6. TRAFFIC AND ROADS

Vehicular access to the development will be from Cork Street.

- 6.1m road width;
- 6.0m wide Fire Route with 12.0m radius;
- Pavement design:
 - 450mm Granular B
 - 150mm Granular A
 - 50mm HL4
 - 40mm HL3

7. UTILITIES

7.1 STREETLIGHTS

The configuration of the streetlights will be designed in accordance with municipal standards. Photometric design and calculations will be provided.

7.2 ELECTRICITY

Wellington North Hydro will be responsible for reviewing the design of the electrical distribution system. Underground distribution lines will be utilized for this development and each unit will be metered separately.

7.3 NATURAL GAS

Enbridge Gas will be responsible for completing the design of the natural gas distribution system. Underground distribution lines will be utilized for this development and each unit will be metered separately.

7.4 TELEPHONE/ CABLE TV/ INTERNET

Wightman, Eastlink and other utilities will be given the opportunity to provide telephone, cable TV and internet services to the development. They will complete their own design, based upon the proposed design configuration along with trench locations dictated by the Township's standard cross section.

If you have any questions regarding the above, please contact either of the undersigned at 519-506-5959 ext. 101.

Sincerely,

Cobide Engineering Inc.



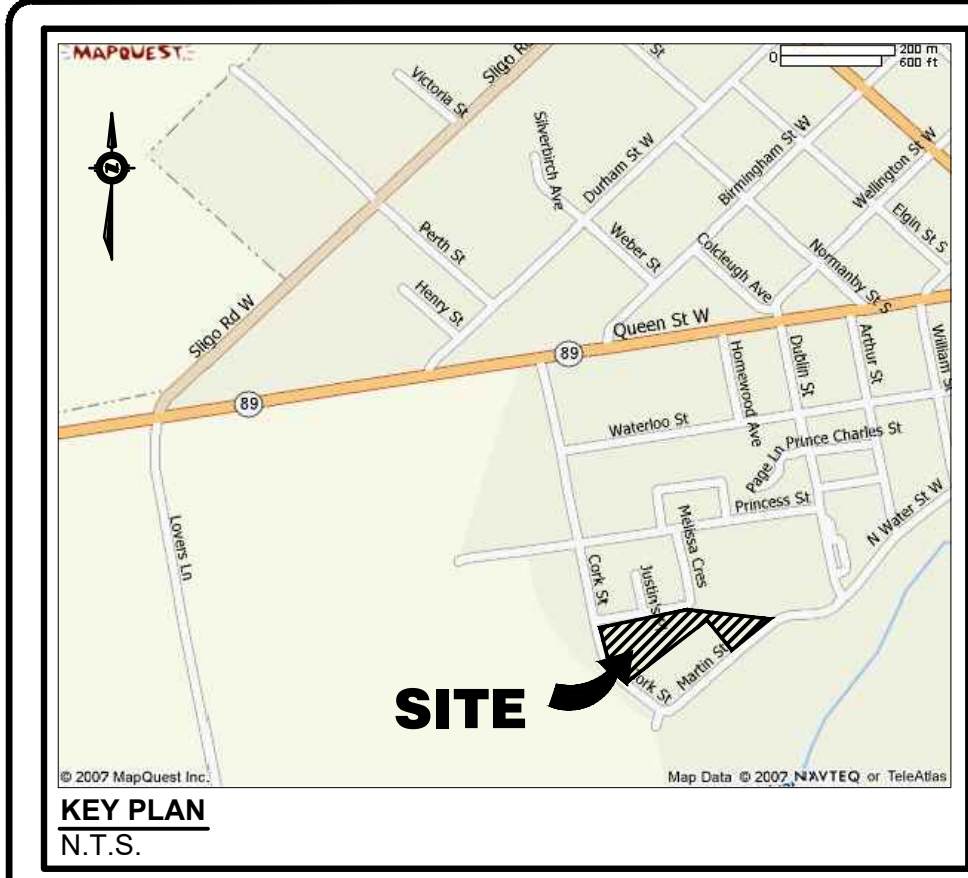
Travis Burnside, P. Eng.



Appendix A

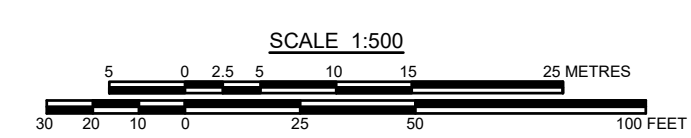
DRAWINGS

645 MARTINS STREET DEVELOPMENT



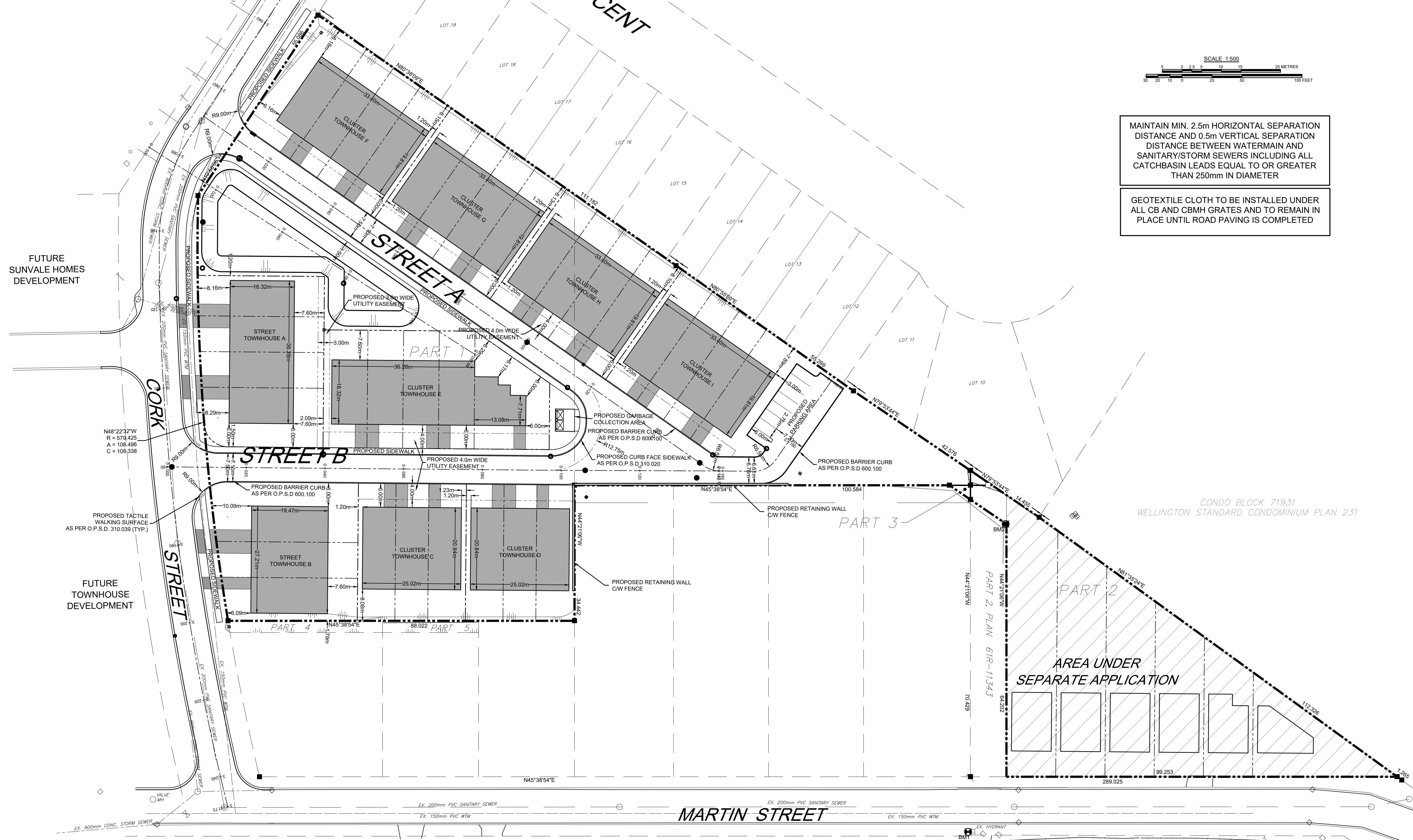
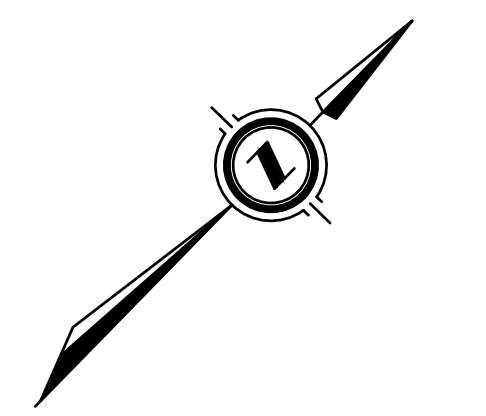
LEGEND			
	PROPERTY BOUNDARY		PROPOSED SANITARY MANHOLE
	PROPOSED RIGHT OF WAY		EXISTING SANITARY MANHOLE
	EDGE OF PROPOSED PAVEMENT		PROPOSED STORM MANHOLE
	EDGE OF EXISTING PAVEMENT		EXISTING STORM MANHOLE
	EDGE OF EXISTING GRAVEL		PROPOSED CATCHBASIN MANHOLE
	PROPOSED SANITARY SEWER		EXISTING CATCHBASIN MANHOLE
	EXISTING SANITARY SEWER		PROPOSED TWIN INLET CATCHBASIN MANHOLE
	PROPOSED STORM SEWER		EXISTING TWIN INLET CATCHBASIN MANHOLE
	EXISTING STORM SEWER		PROPOSED CATCH BASIN
	PROPOSED SUBDRAIN		EXISTING CATCH BASIN
	EXISTING SUBDRAIN		PROPOSED DITCH INLET CATCHBASIN
	EXISTING WATERMAIN		PROPOSED SANITARY SERVICE CLEANOUT
	PROPOSED SANITARY SERVICE		EXISTING SANITARY SERVICE CLEANOUT
	PROPOSED WATER SERVICE		PROPOSED CURB STOP VALVE
	EXISTING WATER SERVICE		EXISTING CURB STOP VALVE
	EXISTING UNDERGROUND TV CABLE		PROPOSED STORM SERVICE
	EXISTING UNDERGROUND GAS LINE		PROPOSED HYDRANT SET
	EXISTING UNDERGROUND HYDRO CABLE		EXISTING FIRE HYDRANT
	EXISTING UNDERGROUND TELEPHONE CABLE		PROPOSED CAP C/W THRUST BLOCK
	PROPOSED 1.80m HIGH SOLID BOARD FENCE		PROPOSED BLOWOFF
	PROPOSED 1.22m HIGH CHAINLINK FENCE		PROPOSED PARKING LOT LIGHT STANDARD
			PROPOSED BUILDING FACE LIGHT

- CAUTION:**
THE POSITION OF POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE DRAWINGS, AND, WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, THE CONTRACTOR SHALL INFORM THEMSELVES OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES, AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.
- Notes**
1. TOPOGRAPHIC INFORMATION DERIVED FROM TOPOGRAPHICAL SURVEY BY COBIDE ENGINEERING INC. DATED OCTOBER 27, 2022.
 2. PROPERTY BOUNDARY DERIVED FROM LEGAL PLAN OF SURVEY BY WILSON-FORD DATED APRIL 29, 2022.
 3. SEE SHEET 03909-C1 FOR TYPICAL CROSS SECTIONS AND PAVEMENT DESIGN.
 4. ALL ORGANIC MATERIAL WITHIN 1.2m OF FINISHED PROFILE GRADE TO BE REMOVED FROM ALL AREAS UNDER THE TRAVELLED PORTION OF THE ROAD.
 5. COVER OVER WATERMAIN & SERVICES 2.0m MINIMUM AT ALL POINTS.
 6. SANITARY SEWER TO BE PVC SDR35.
 7. ALL WATERMANS TO BE PVC DR18 WITH MUNICIPEX WATER SERVICES.
 8. MAINTAIN 2.5m CLEARANCE BETWEEN STORM SEWER AND WATERMAIN.
 9. ALL HYDRANT SETS REQUIRE TEST POINT AND HYDRANT MARKER.
 10. ALL JOINTS OF SANITARY MANHOLES TO BE CAULKED WITH MIN. 15mm BEAD, INSTALLED ON THE TOP OF JOINT OF EACH SECTION PRIOR TO SECTION ABOVE BEING INSTALLED. CAULKING TO BE SIKAFLEX 1A OR APPROVED EQUIVALENT. ALL JOINTS TO BE WRAPPED WITH 150mm DENS0 TAPE.
 11. ALL STORM CATCHBASINS TO HAVE A MINIMUM SUMP OF 600mm AND ALL STORM MANHOLES AND CATCHBASIN MANHOLES TO HAVE A SUMP OF 300mm.
 12. CONTRACTOR TO OBTAIN A COPY OF THE MOST CURRENT VERSION OF THE MUNICIPAL SERVICING STANDARDS PRIOR TO CONSTRUCTION.
 13. 48 HOURS NOTICE MUST BE GIVEN TO THE TOWNSHIP PUBLIC WORKS DEPARTMENT BEFORE ANY CONSTRUCTION WITHIN THE TOWNSHIP ROAD ALLOWANCE.
 14. THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION PURPOSES UNTIL STAMPED 'ISSUED FOR CONSTRUCTION'.
 15. ALL CONSTRUCTION TO BE COMPLETED TO MUNICIPAL SERVICING STANDARDS FOR THE TOWNSHIP OF WELLINGTON NORTH.
 16. WATERMAIN TO BE RESTRAINED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. RESTRAINTS TO CONFORM TO TOWNSHIP OF WELLINGTON NORTH STANDARDS.
 17. SANITARY SERVICES SHALL BE PVC DR 28, 125mmØ IN SIZE AND CONNECTED USING A FACTORY TEE.
 18. STORM SERVICES SHALL BE PVC DR 28, 100mmØ IN SIZE AND CONNECTED USING A FACTORY TEE.
 19. WATER SERVICES (19mmØ MUNICIPEX) ARE TO BE COMPLETELY EMBEDDED USING SAND. SAND SHALL BE FINE AGGREGATE PASSING SPECS AS PER OPS: 1001.



MAINTAIN MIN. 2.5m HORIZONTAL SEPARATION DISTANCE AND 0.5m VERTICAL SEPARATION DISTANCE BETWEEN WATERMAIN AND SANITARY/STORM SEWERS INCLUDING ALL CATCHBASIN LEADS EQUAL TO OR GREATER THAN 250mm IN DIAMETER

GEOTEXTILE CLOTH TO BE INSTALLED UNDER ALL CB AND CBMH GRATES AND TO REMAIN IN PLACE UNTIL ROAD PAVING IS COMPLETED



Benchmark Information

BM1	TOP NUT OF EXISTING HYDRANT AT 650 MARTIN STREET (HYDRANT NO. 148).	ELEVATION	415.65m
BM2	TOP OF EXISTING IB LOCATED AT THE NORTHEAST PROPERTY BOUNDARY, APPROX. 73m NORTH OF THE SANITARY MANHOLE LOCATED BETWEEN 645 AND 655 MARTIN STREET.	ELEVATION	415.01m

No.	DATE	DESCRIPTION	BY	APPD
0	FEB 13/23	PRELIMINARY SUBMISSION	ARMH	TLB

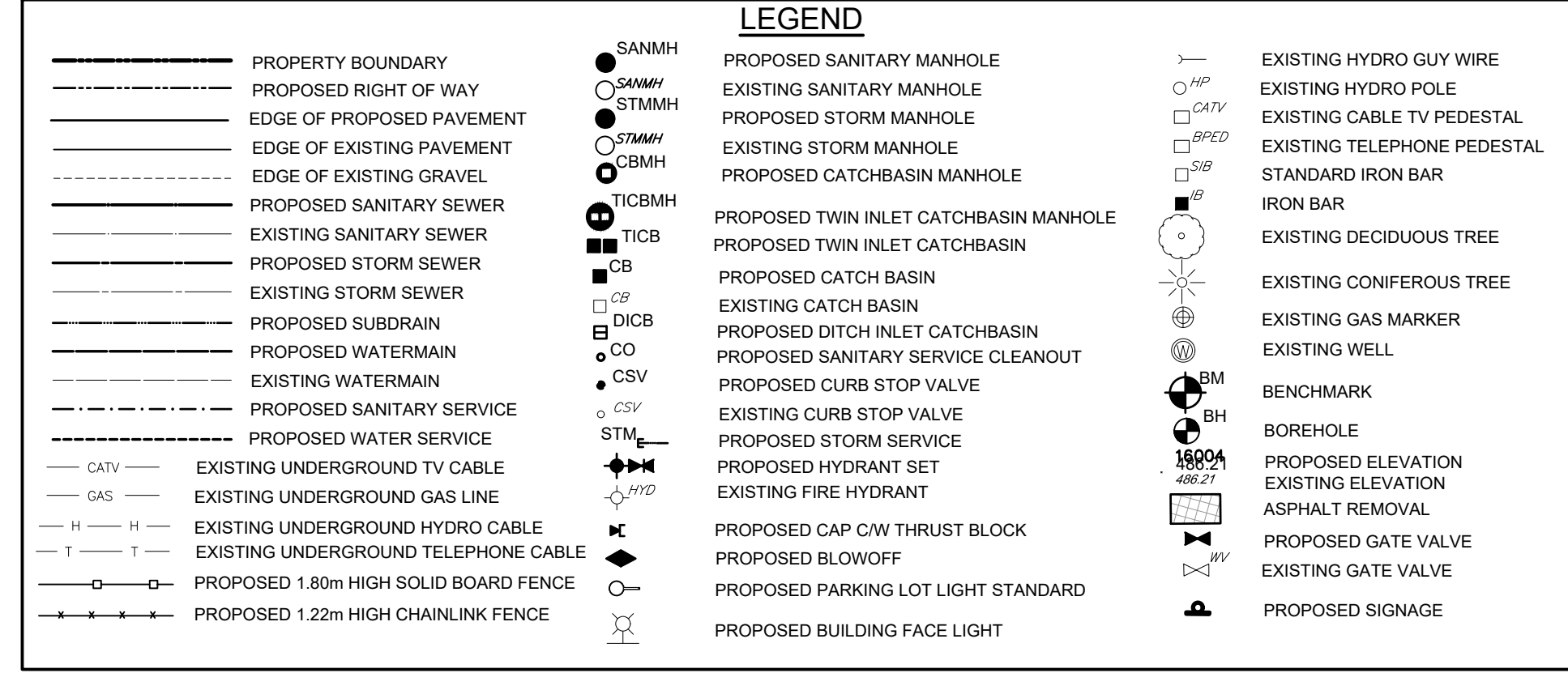
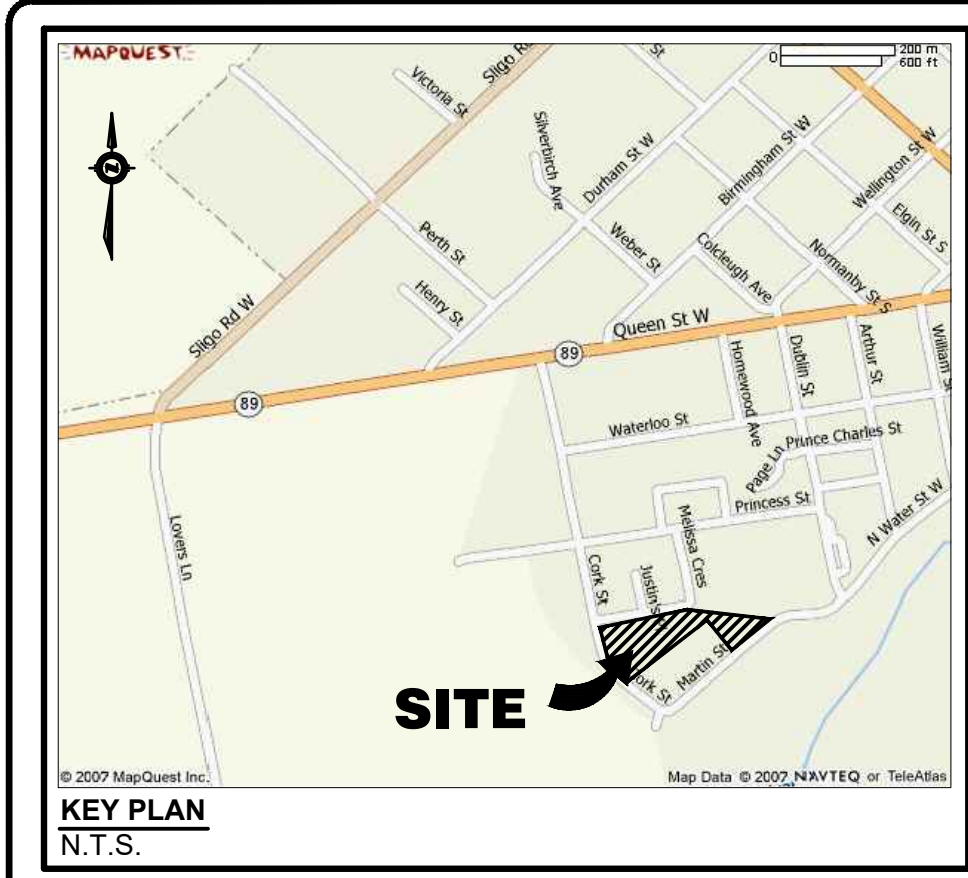
Seal not valid unless signed and dated

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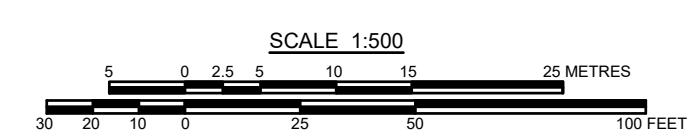
**PROPOSED TOWNHOUSE DEVELOPMENT
645 MARTIN STREET, MOUNT FOREST
TOWNSHIP OF WELLINGTON NORTH
SITE PLAN**

Client:	MAMTA HOMES
Design:	TLB
Drawn:	ARMH
Checked:	TLB
Date:	NOVEMBER 2022
DRAWING No.	03909-C1

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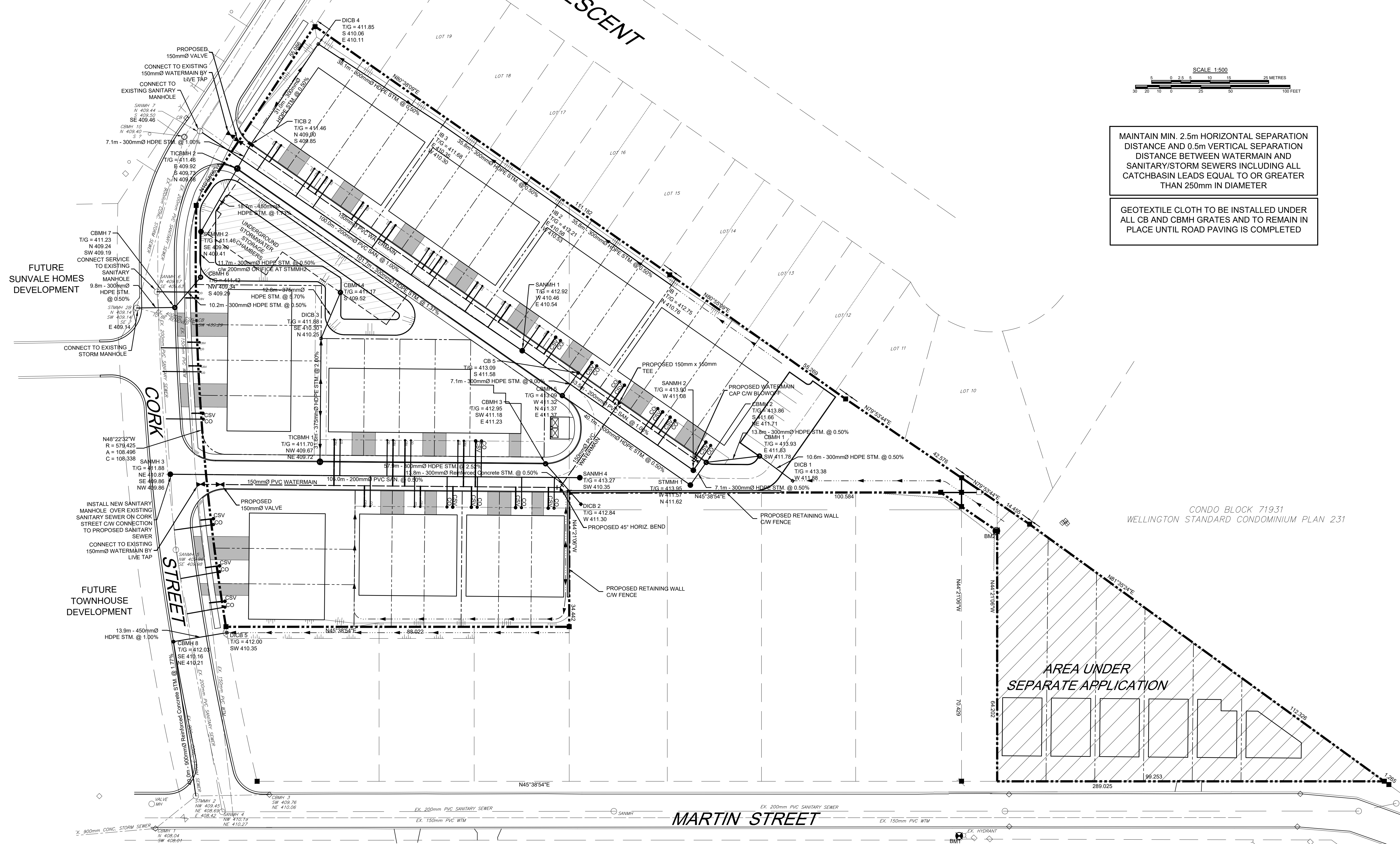
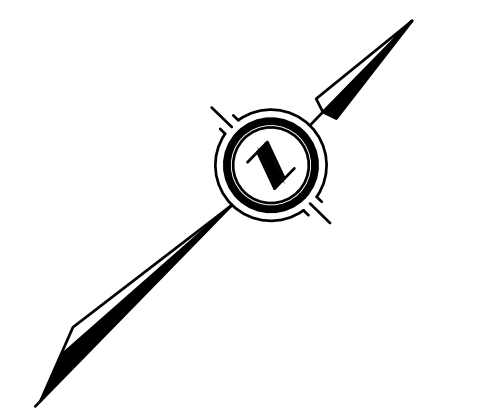


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GEOTEXTILE CLOTH TO BE INSTALLED UNDER ALL CB AND CBMH GRATES AND TO REMAIN IN PLACE UNTIL ROAD PAVING IS COMPLETED



Benchmark Information

BM1	TOP NUT OF EXISTING HYDRANT AT 650 MARTIN STREET (HYDRANT NO. 148).	ELEVATION	415.65m
BM2	TOP OF EXISTING IB LOCATED AT THE NORTHEAST PROPERTY BOUNDARY, APPROX. 73m NORTH OF THE SANITARY MANHOLE LOCATED BETWEEN 645 AND 655 MARTIN STREET.	ELEVATION	415.01m

No.	DATE	DESCRIPTION	BY	APPD
0	FEB 13/23	PRELIMINARY SUBMISSION	ARMH	TLB
REVISION / ISSUE				

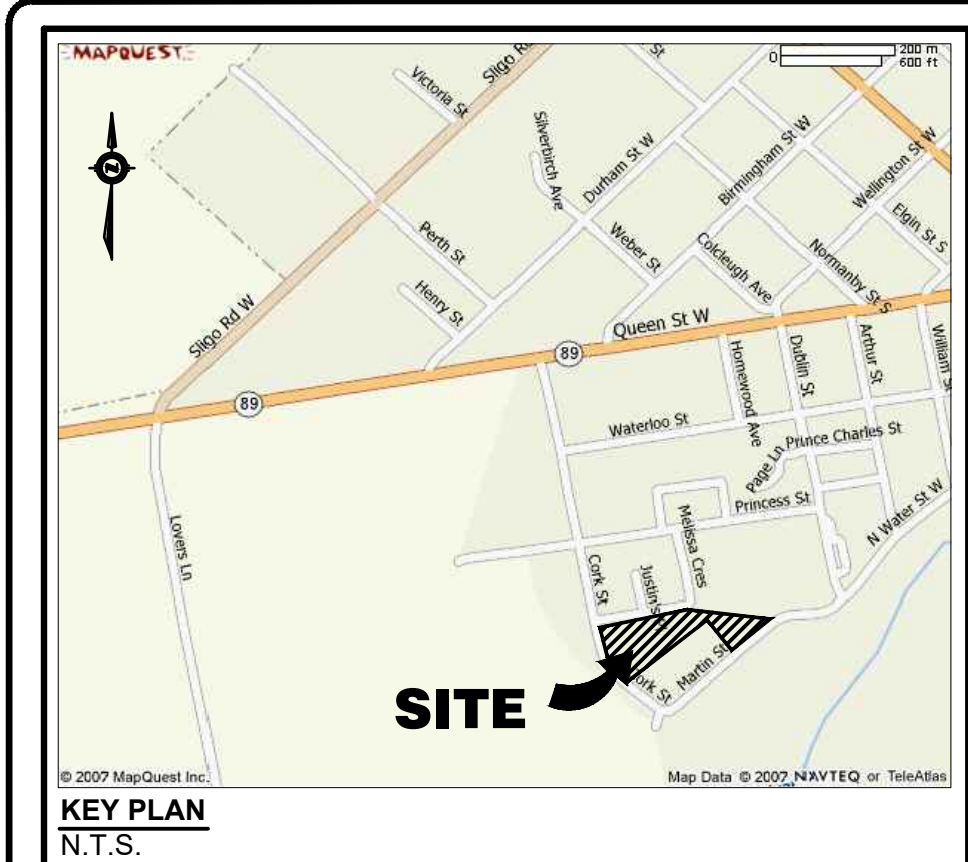


**PROPOSED TOWNHOUSE DEVELOPMENT
645 MARTIN STREET, MOUNT FOREST
TOWNSHIP OF WELLINGTON NORTH
SITE SERVICING PLAN**

Client: **MAMTA HOMES**

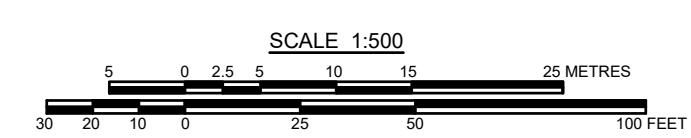
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Drawn:	ARMH	Approved:	
Checked:	TLB	Date:	NOVEMBER 2022
DRAWING No.	03909-C2	Design Engineer	

H:\Keng\03909-645 Martine Street Mount Forest\Drawings\Submissions\2022-11-11 Preliminary Submission\03909-Base_2023-01-03.dwg



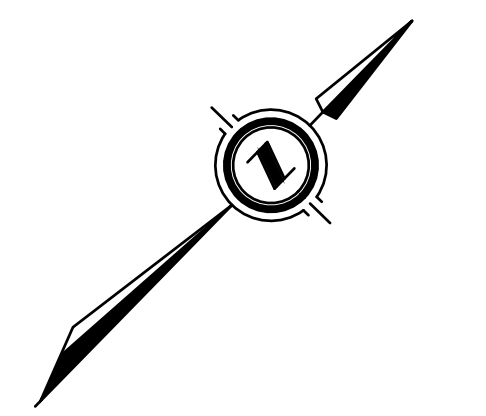
LEGEND	
	PROPERTY BOUNDARY
	PROPOSED RIGHT OF WAY
	EDGE OF PROPOSED PAVEMENT
	EDGE OF EXISTING PAVEMENT
	EDGE OF EXISTING GRAVEL
	PROPOSED SANITARY SEWER
	EXISTING SANITARY SEWER
	PROPOSED STORM SEWER
	EXISTING STORM SEWER
	PROPOSED SUBDRAIN
	PROPOSED WATERMAIN
	EXISTING WATERMAIN
	PROPOSED SANITARY SERVICE
	PROPOSED WATER SERVICE
	EXISTING UNDERGROUND TV CABLE
	EXISTING UNDERGROUND GAS LINE
	EXISTING UNDERGROUND HYDRO CABLE
	EXISTING UNDERGROUND TELEPHONE CABLE
	PROPOSED 1.80m HIGH SOLID BOARD FENCE
	PROPOSED 1.22m HIGH CHAINLINK FENCE
	PROPOSED SANITARY MANHOLE
	EXISTING SANITARY MANHOLE
	PROPOSED STORM MANHOLE
	EXISTING STORM MANHOLE
	PROPOSED CATCHBASIN MANHOLE
	PROPOSED TWIN INLET CATCHBASIN MANHOLE
	PROPOSED TWIN INLET CATCHBASIN
	PROPOSED CATCH BASIN
	EXISTING CATCH BASIN
	PROPOSED DITCH INLET CATCHBASIN
	PROPOSED SANITARY SERVICE CLEANOUT
	PROPOSED CURB STOP VALVE
	EXISTING CURB STOP VALVE
	PROPOSED STORM SERVICE
	PROPOSED HYDRANT SET
	EXISTING FIRE HYDRANT
	PROPOSED CAP C/W THRUST BLOCK
	PROPOSED BLOWOFF
	PROPOSED PARKING LOT LIGHT STANDARD
	PROPOSED BUILDING FACE LIGHT
	EXISTING HYDRO GUY WIRE
	EXISTING HYDRO POLE
	EXISTING CABLE TV PEDESTAL
	EXISTING TELEPHONE PEDESTAL
	STANDARD IRON BAR
	IRON BAR
	EXISTING DECIDUOUS TREE
	EXISTING CONIFEROUS TREE
	EXISTING GAS MARKER
	EXISTING WELL
	BENCHMARK
	BOREHOLE
	PROPOSED ELEVATION
	EXISTING ELEVATION
	ASPHALT REMOVAL
	PROPOSED GATE VALVE
	EXISTING GATE VALVE
	PROPOSED SIGNAGE

- CAUTION:**
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 4. ALL ORGANIC MATERIAL WITHIN 1.2m OF FINISHED PROFILE GRADE TO BE REMOVED FROM ALL AREAS UNDER THE TRAVELLED PORTION OF THE ROAD.
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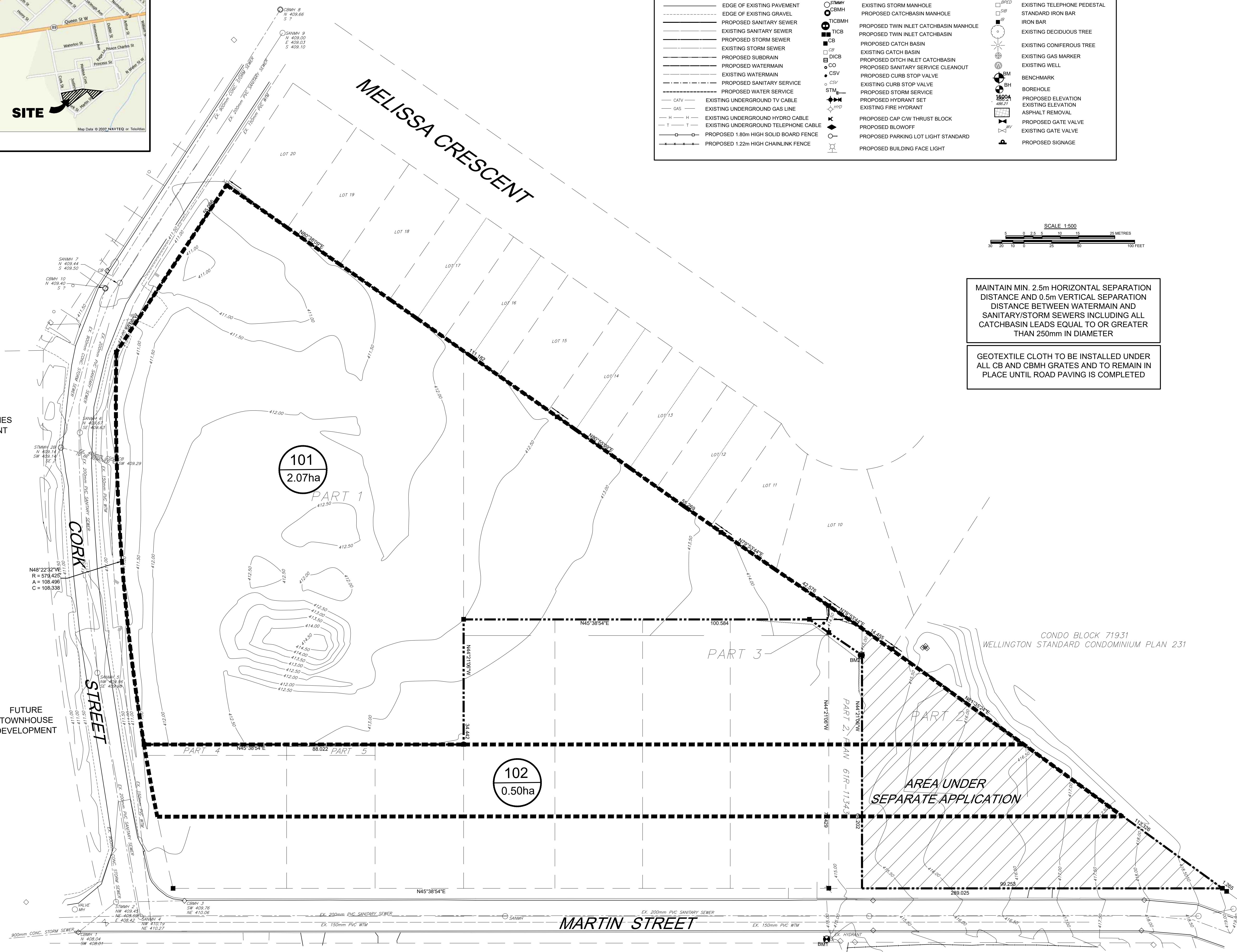
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FUTURE SUNVALE HOMES DEVELOPMENT

FUTURE TOWNHOUSE DEVELOPMENT



Benchmark Information

BM1	TOP NUT OF EXISTING HYDRANT AT 650 MARTIN STREET (HYDRANT NO. 148).	ELEVATION	415.65m
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No.	DATE	DESCRIPTION	BY	APPD
0	FEB 13/23	PRELIMINARY SUBMISSION	ARMH	TLB
REVISION / ISSUE				

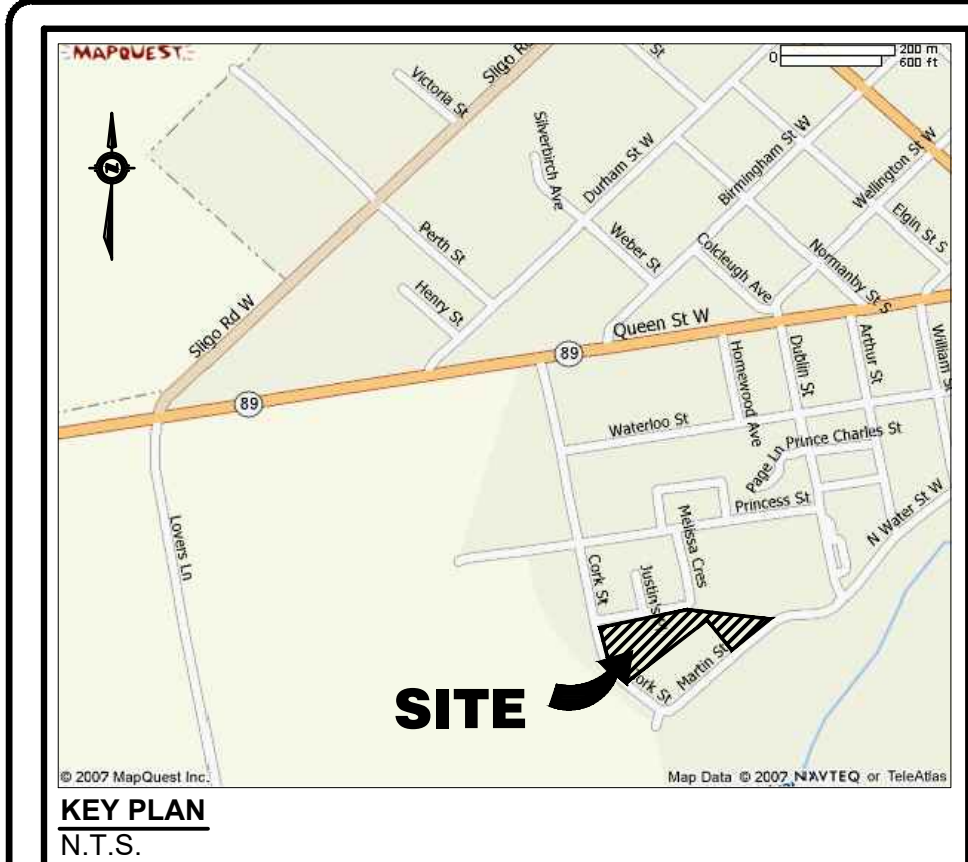


517 10th Street, Hanover, Ontario N4N 1R4
Telephone: (519) 506-5959
www.cobideeng.com

**PROPOSED TOWNHOUSE DEVELOPMENT
645 MARTIN STREET, MOUNT FOREST
TOWNSHIP OF WELLINGTON NORTH
PRE-DEVELOPMENT DRAINAGE AREAS**

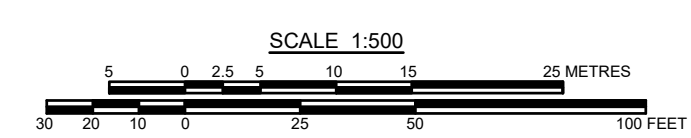
Client: MAMTA HOMES

Design:	TLB	Scale:	1:500
Drawn:	ARMH	Approved:	
Checked:	TLB		
Date:	NOVEMBER 2022		
DRAWING No.	03909-SWM1	Design Engineer	



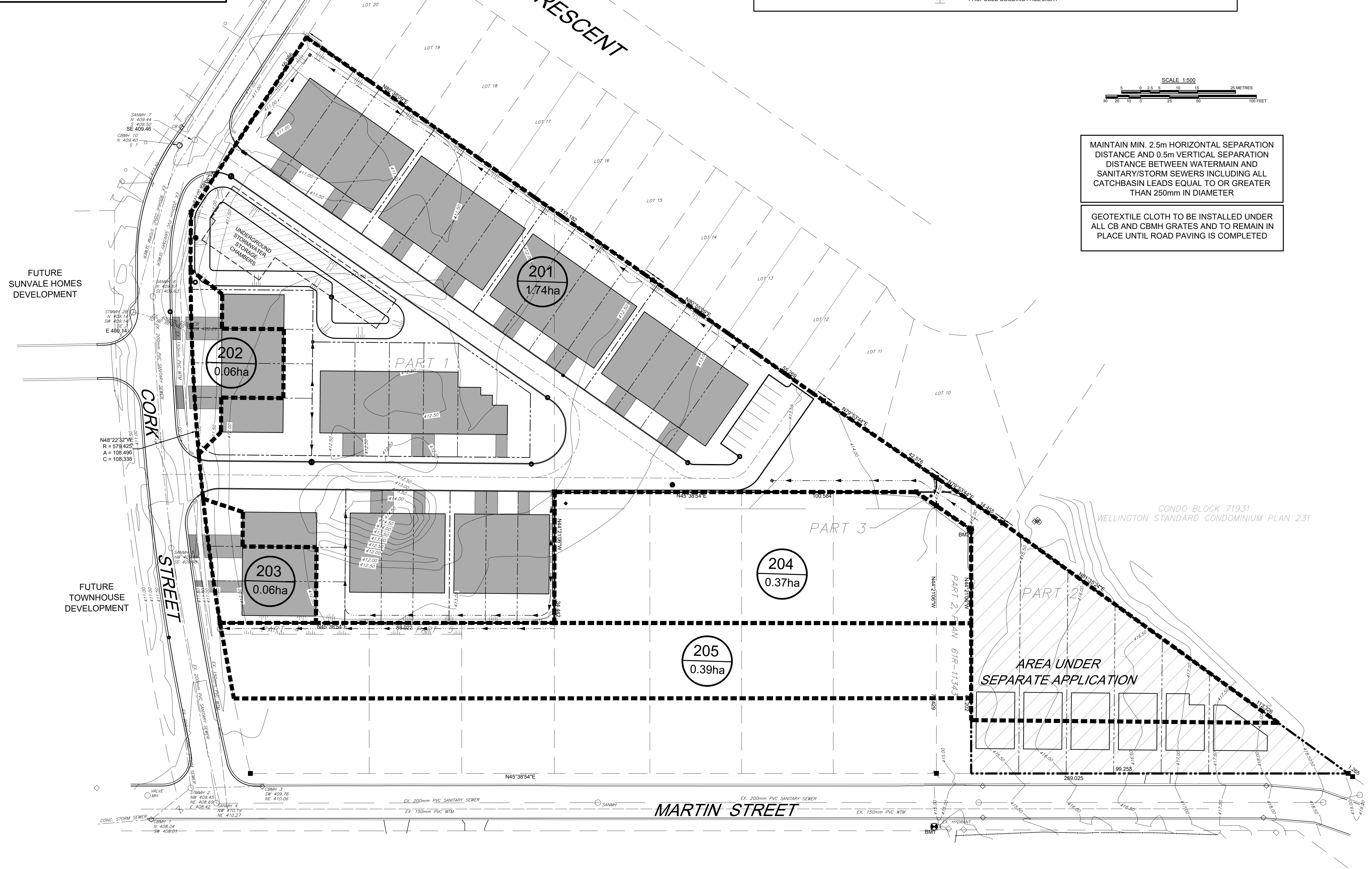
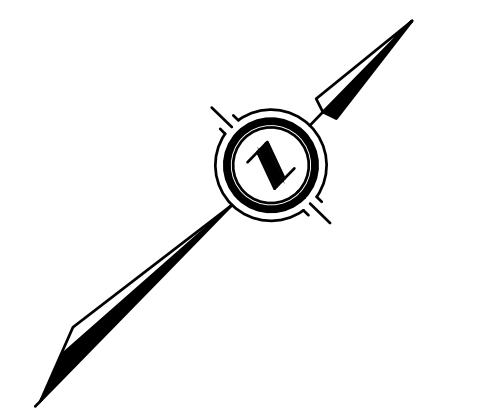
LEGEND			
	PROPERTY BOUNDARY		PROPOSED SANITARY MANHOLE
	PROPOSED RIGHT OF WAY		EXISTING SANITARY MANHOLE
	EDGE OF PROPOSED PAVEMENT		PROPOSED STORM MANHOLE
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	EXISTING SANITARY SEWER		PROPOSED DITCH INLET CATCHBASIN
	PROPOSED STORM SEWER		PROPOSED SANITARY SERVICE CLEANOUT
	EXISTING STORM SEWER		PROPOSED CURB STOP VALVE
	PROPOSED SUBDRAIN		EXISTING CURB STOP VALVE
	PROPOSED WATERMAIN		PROPOSED STORM SERVICE
	EXISTING WATERMAIN		PROPOSED HYDRANT SET
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No.	DATE	DESCRIPTION	BY	APPD
0	FEB 13/23	PRELIMINARY SUBMISSION	ARMH	TLB
REVISION / ISSUE				



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**PROPOSED TOWNHOUSE DEVELOPMENT
645 MARTIN STREET, MOUNT FOREST
TOWNSHIP OF WELLINGTON NORTH
POST DEVELOPMENT DRAINAGE AREAS**

Client: **MAMTA HOMES**

Design:	TLB	Scale:	1:500
Drawn:	ARMH	Approved:	
Checked:	TLB		
Date:	NOVEMBER 2022		
DRAWING No.	03909-SWM1		

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Appendix B

PRE DEVELOPMENT MODEL AND OUTPUT

645 MARTINS STREET DEVELOPMENT

Table A.1 Parameter Summary Table

Existing Conditions									
Outlet Location	Model Catchment ID	Description	Area (ha)	Drainage Channel (m)	Flow Length (m)	Gradient (%)	Total Imperv. Connected (%)	Manning's 'n' (Perv.)	CN (Perv.)
Cork Street North	101	Entire Site boundary	2.07	150	138	2.0	0.0	0.30	72.0
Cork Street South	102	Rear half of lots fronting Martin Street	0.50	198	25	2.3	27.9	0.25	77.0

Table A.2 Site Soils: (as per Ontario Soil Survey Report No. 35 for Wellington County)

Soil Type
Listowel Silt Loam

Hydrologic Soil Group
BC

Land Use	TABLE OF CURVE NUMBERS (CN's)								Manning's 'n'	
	Hydrologic Soil Type									
	A	AB	B	BC	C	CD	D			
Meadow	50	54	58	64.5	71	74.5	78	0.4	continuous grass	
Woodlot	50	55.3	60.5	67	73.5	76.8	80	0.4	forests	
Long Grass	55	60	65	72	79	81.5	84	0.3	natural, not maintained	
Lawns	60	65.5	71	77	83	86	89	0.25	maintained	
Pasture/Range	58	61.5	65	70.5	76	78.5	81	0.17	farm pasture	
Crop	66	70	74	78	82	84	86	0.13	farm land	
Fallow (bare)	77	82	86	89	91	93	94	0.05	idle farm land (bare)	
Built-up	60	65.5	71	77	83	89	89	0.25	Lawns Existing	
Streets, paved	98	98	98	98	98	98	98	0.01		

Catchment	HYDROLOGIC SOIL TYPE (%) - Existing Conditions							TOTAL
	Hydrologic Soil Type							
	A	AB	B	BC	C	CD	D	
101	0	0	0	100	0	0	0	100
102	0	0	0	100	0	0	0	100
103	0	0	0	100	0	0	0	100
104	0	0	0	100	0	0	0	100
105	0	0	0	100	0	0	0	100
106	0	0	0	100	0	0	0	100
107	0	0	0	100	0	0	0	100
108	0	0	0	100	0	0	0	100
109	0	0	0	100	0	0	0	100
110	0	0	0	100	0	0	0	100
111	0	0	0	100	0	0	0	100

Catchment	LAND USE (%) - Existing Conditions									Total
	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Imperv. Not Connected (Rooftops)	Imperv. Connected	
101	0	0	100	0	0.0	0.0	0	0.0	0.0	100
102	0	0	0	72	0	0	0	27.5	0.4	100
103	0	0	0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
104	0	0	0	0	0.0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
105	0	0	0	0	0.0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
106	0	0	0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
107	0	0	0	0	0	0.0	0	#DIV/0!	#DIV/0!	#DIV/0!
108	0	0	0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
109	0	0	0.0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
110	0	0	0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!
111	0	0	0.0	0	0	0	0	#DIV/0!	#DIV/0!	#DIV/0!

Catchment	CURVE NUMBER (CN) - Existing Conditions										Manning's 'n'
	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Built-up	Imperv. Not Connected (Rooftops)	Weighted CN - Pervious	
101	65	67	72	77	70.5	78	89	77	90	72.0	0.30
102	65	67.0	72	77	71	78	89	77	90	77.0	0.25
103	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
104	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
105	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
106	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
107	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
108	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
109	65	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
110	64.5	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!
111	64.5	67	72	77	70.5	78	89	77	90	#DIV/0!	#DIV/0!

Table A.3: Impervious Area Determination for Subcatchment 101-102

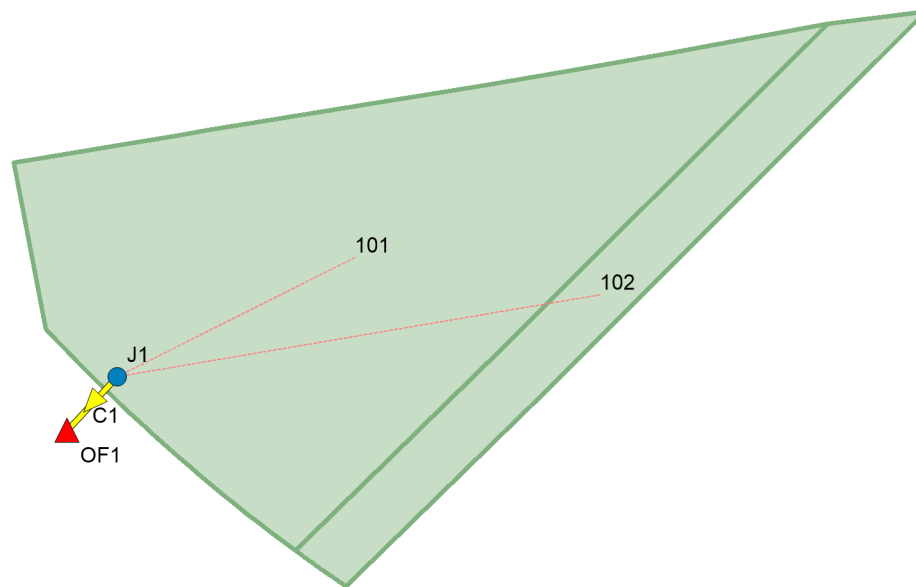
Existing Conditions

Area of Concern	Total Area (ha)	Impervious Area Connected		Impervious Area Not Connected (Rooftops)		Total (%)
		(ha)	(%)	(ha)	(%)	
101	2.07	0.00	0.0	0.00	0.0	0.0
102	0.50	0.00	0.4	0.14	27.5	27.9

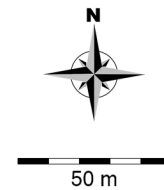
Table A.3 - Impervious Area Determination for Existing Catchments 101-102

Catchment					Imperv. Area	Imperv %
101	1	m of	1	m wide ROW @ 45% imperv.	0.00 ha	0.0 %
	1	Impervious Area	1	m ² @ 100% imperv. Concrete	0.00 ha	0.0 %
	1	Roof Area	1	m ² @ 100% imperv.	0.00 ha	0.0 %
					0.00 ha	
102	0	m of	10	m wide ROW @ 50% imperv.	0.00 ha	0.0 %
	1	Driveway	18.74	m ² @ 100% imperv.	0.00 ha	0.4 %
	1	Roof Area	1375.04	m ² @ 100% imperv.	0.14 ha	27.5 %
					0.14 ha	

645 MARTINS STREET DEVELOPMENT - PRE DEVELOPMENT MODEL SCHEMATIC



- ### Legend
- Junctions
 - ▲ Outfalls
 - Conduits
 - Subcatchments



645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MODEL DETAILS

[TITLE]

;;Project Title/Notes

[OPTIONS]

```
;;Option Value
FLOW_UNITS LPS
INFILTRATION HORTON
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 11/30/2022
START_TIME 00:00:00
REPORT_START_DATE 11/30/2022
REPORT_START_TIME 00:00:00
END_DATE 12/1/2022
END_TIME 00:00:00
SWEEP_START 1/1
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:05:00
DRY_STEP 00:05:00
ROUTING_STEP 5
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 8
```

[EVAPORATION]

```
;;Data Source Parameters
;;-----
CONSTANT 0.0
DRY_ONLY NO
```

[RAINGAGES]

```
;;Name Format Interval SCF Source
;;-----
SCS_Type_II_38.8mm_2yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_38.8mm_2yr
SCS_Type_II_49.4mm_5yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_49.4mm_5yr
SCS_Type_II_65.3mm_25yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_65.3mm_25yr
SCS_Type_II_71.9mm_50yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_71.9mm_50yr
SCS_Type_II_78.4mm_100yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_78.4mm_100yr
SCS_Type_II_88mm_MTO100Yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_88mm_MTO100Yr
```

[SUBCATCHMENTS]

```
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen SnowPack
;;-----
101 SCS_Type_II_88mm_MTO100Yr J1 2.07 0 150 2 0
102 SCS_Type_II_88mm_MTO100Yr J1 0.5 27.9 198 2.3 0
```

[SUBAREAS]

```
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
;;-----
101 0.01 0.3 0.05 0.05 25 OUTLET
102 0.01 0.25 0.05 0.05 25 OUTLET
```

[INFILTRATION]

```
;;Subcatchment Param1 Param2 Param3 Param4 Param5
;;-----
101 72 0.5 7 0 0 CURVE_NUMBER
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MODEL DETAILS

102 77 0.5 7 0 0 CURVE_NUMBER

[JUNCTIONS]

```
;;Name            Elevation    MaxDepth    InitDepth    SurDepth    Aponded
-----
J1                409.2        1            0            0            0
```

[OUTFALLS]

```
;;Name            Elevation    Type            Stage Data            Gated        Route To
-----
OF1                409.13        FREE                                            NO
```

[CONDUITS]

```
;;Name            From Node            To Node            Length        Roughness    InOffset    OutOffset    InitFlow
MaxFlow
-----
C1                J1                    OF1                15.4          0.013        409.2        409.13        0            0
```

[XSECTIONS]

```
;;Link            Shape            Geom1            Geom2        Geom3        Geom4        Barrels        Culvert
-----
C1                CIRCULAR        0.375            0            0            0            1
```

[LOSSES]

```
;;Link            Kentry        Kexit        Kavg        Flap Gate    Seepage
-----
```

[TIMESERIES]

```
;;Name            Date            Time            Value
-----
;SCS_Type_II_38.8mm design storm, total rainfall = 38.8 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_38.8mm_2yr

;SCS_Type_II_49.4mm design storm, total rainfall = 49.4 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_49.4mm_5yr

;SCS_Type_II_65.3mm design storm, total rainfall = 65.3 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_65.3mm_25yr

;SCS_Type_II_71.9mm design storm, total rainfall = 71.9 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_71.9mm_50yr

;SCS_Type_II_78.4mm design storm, total rainfall = 78.4 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_78.4mm_100yr

;SCS_Type_II_88mm design storm, total rainfall = 88 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_88mm_MTO100Yr
```

[REPORT]

```
;;Reporting Options
INPUT            YES
CONTROLS        NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DIMENSIONS        520556.537886047 4868693.67327316 520882.338393007 4868899.03726366
UNITS             Meters
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_38.8mm_2yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_38.8mm_2yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.100      38.800
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.071      27.516
Surface Runoff ..... 0.028      10.981
Final Storage ..... 0.001      0.301
Continuity Error (%) ..... 0.006
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.028      0.283
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.028      0.283
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
None
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      :      4.50 sec
Average Time Step      :      5.00 sec
Maximum Time Step     :      5.00 sec
Percent in Steady State :      0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 100.00 %
 3.155 - 1.991 sec : 0.00 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
101		38.80	0.00	0.00	29.69	0.00	8.74	8.74	0.18
15.52	0.225								
102		38.80	0.00	0.00	18.51	10.82	9.42	20.25	0.10
41.46	0.522								

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.03	0.17	409.37	0 03:06	0.17
OF1	OUTFALL	0.03	0.16	409.29	0 03:06	0.16

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	51.23	51.23	0 03:06	0.283	0.283	0.004
OF1	OUTFALL	0.00	51.21	0 03:06	0	0.283	0.000

 Node Surcharge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Outfall Loading Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

```

-----
                Flow      Avg      Max      Total
Outfall Node   Freq      Flow      Flow      Volume
                Pcnt      LPS      LPS      10^6 ltr
-----
OF1             99.86     3.27     51.21     0.283
-----
System         99.86     3.27     51.21     0.283
    
```

 Link Flow Summary

```

-----
                Maximum   Time of Max   Maximum   Max/   Max/
Link           Type      |Flow|       Occurrence   |Veloc|   Full   Full
                LPS      days hr:min   m/sec     Flow   Depth
-----
C1             CONDUIT     51.21       0 03:06     1.08     0.43   0.45
    
```

 Flow Classification Summary

```

-----
                Adjusted   ----- Fraction of Time in Flow Class -----
Conduit        /Actual   Up   Down Sub Sup Up   Down Norm Inlet
                Length  Dry Dry Dry Crit Crit Crit Crit Crit Ltd Ctrl
-----
C1             1.00   0.00 0.00 0.00 0.64 0.36 0.00 0.00 0.00 0.00
    
```

 Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Mon Feb 13 14:51:14 2023
 Analysis ended on: Mon Feb 13 14:51:14 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_49.4mm_5yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_49.4mm_5yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.127      49.400
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.084      32.602
Surface Runoff ..... 0.042      16.494
Final Storage ..... 0.001      0.301
Continuity Error (%) ..... 0.006
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.042      0.424
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.042      0.424
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
None
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      :      4.51 sec
Average Time Step      :      5.00 sec
Maximum Time Step      :      5.00 sec
Percent in Steady State :      0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 100.00 %
 3.155 - 1.991 sec : 0.00 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff	Subcatchment	Runoff Coeff	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imperv Runoff mm	Perv Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr
101	28.78	0.279	49.40	0.00	0.00	35.27	0.00	13.76	13.76	0.28
102	58.94	0.563	49.40	0.00	0.00	21.57	13.78	14.01	27.79	0.14

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.04	0.22	409.42	0 03:06	0.22
OF1	OUTFALL	0.03	0.21	409.34	0 03:06	0.21

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	81.49	81.49	0 03:06	0.424	0.424	0.002
OF1	OUTFALL	0.00	81.39	0 03:06	0	0.424	0.000

 Node Surcharge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Outfall Loading Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

```

-----
                Flow      Avg      Max      Total
Outfall Node    Freq      Flow      Flow      Volume
                Pcnt      LPS      LPS      10^6 ltr
-----
OF1              99.88     4.96     81.39     0.424
-----
System          99.88     4.96     81.39     0.424
    
```

Link Flow Summary

```

-----
                Maximum  Time of Max  Maximum  Max/  Max/
Link           Type      |Flow|      Occurrence  |Veloc|  Full  Full
                LPS      days hr:min  m/sec   Flow  Depth
-----
C1              CONDUIT    81.39      0 03:06     1.25    0.69  0.57
    
```

Flow Classification Summary

```

-----
                Adjusted  ----- Fraction of Time in Flow Class -----
Conduit         /Actual  Up   Down  Sub  Sup  Up   Down  Norm  Inlet
                Length  Dry Dry  Dry  Crit Crit Crit  Crit  Ltd  Ctrl
-----
C1              1.00  0.00 0.00 0.00 0.64 0.36 0.00 0.00 0.00 0.00
    
```

Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Mon Feb 13 14:51:57 2023
Analysis ended on: Mon Feb 13 14:51:57 2023
Total elapsed time: < 1 sec

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_65.3mm_25yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_65.3mm_25yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.168      65.300
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.100      38.834
Surface Runoff ..... 0.067      26.155
Final Storage ..... 0.001      0.309
Continuity Error (%) ..... 0.003
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.067      0.673
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.067      0.673
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
Link C1 (3.70%)
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      :      3.57 sec
Average Time Step      :      4.98 sec
Maximum Time Step      :      5.00 sec
Percent in Steady State :      0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 100.00 %
 3.155 - 1.991 sec : 0.00 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
101		65.30	0.00	0.00	42.09	0.00	22.83	22.83	0.47
58.68	0.350								
102		65.30	0.00	0.00	25.35	18.22	21.70	39.93	0.20
89.25	0.611								

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.04	0.32	409.52	0 03:06	0.31
OF1	OUTFALL	0.04	0.28	409.41	0 03:06	0.28

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	140.04	140.04	0 03:06	0.673	0.673	0.001
OF1	OUTFALL	0.00	139.90	0 03:06	0	0.673	0.000

 Node Surcharge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Outfall Loading Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

```

-----
                Flow      Avg      Max      Total
Outfall Node   Freq      Flow      Flow      Volume
                Pcnt      LPS       LPS       10^6 ltr
-----
OF1             99.90     8.26     139.90    0.673
-----
System         99.90     8.26     139.90    0.673
    
```

 Link Flow Summary

```

-----
                Maximum  Time of Max  Maximum  Max/  Max/
Link           Type      |Flow|      Occurrence  |Veloc|  Full  Full
                LPS      days hr:min  m/sec    Flow  Depth
-----
C1             CONDUIT    139.90     0 03:06     1.50    1.18  0.79
    
```

 Flow Classification Summary

```

-----
                Adjusted  ----- Fraction of Time in Flow Class -----
Conduit        /Actual  Up      Down  Sub  Sup  Up  Down  Norm  Inlet
                Length  Dry    Dry  Dry  Crit Crit Crit  Crit  Ltd  Ctrl
-----
C1             1.00    0.00  0.00  0.00  0.66  0.34  0.00  0.00  0.00  0.00
    
```

 Conduit Surcharge Summary

```

-----
                ----- Hours Full ----- Hours  Hours
Conduit        Both Ends  Upstream  Dnstream  Above Full  Capacity
                ----- Normal Flow ----- Limited
-----
C1             0.01     0.01     0.01     0.15     0.01
    
```

Analysis begun on: Mon Feb 13 14:52:32 2023
 Analysis ended on: Mon Feb 13 14:52:32 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_71.9mm_50yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_71.9mm_50yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.185      71.900
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.106      41.133
Surface Runoff ..... 0.078      30.462
Final Storage ..... 0.001      0.304
Continuity Error (%) ..... 0.000
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.078      0.784
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.078      0.784
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
Link C1 (5.24%)
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      :      2.08 sec
Average Time Step      :      4.96 sec
Maximum Time Step      :      5.00 sec
Percent in Steady State :      0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 99.79 %
 3.155 - 1.991 sec : 0.21 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
101		71.90	0.00	0.00	44.62	0.00	26.91	26.91	0.56
75.21	0.374								
102		71.90	0.00	0.00	26.72	20.07	25.11	45.17	0.23
103.01	0.628								

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.05	0.39	409.59	0 03:06	0.39
OF1	OUTFALL	0.04	0.30	409.43	0 03:06	0.30

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	168.70	168.70	0 03:06	0.784	0.784	0.001
OF1	OUTFALL	0.00	168.70	0 03:06	0	0.784	0.000

 Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
J1	JUNCTION	0.02	0.013	0.612

 Node Flooding Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

No nodes were flooded.

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.91	10.01	168.70	0.784
System	99.91	10.01	168.70	0.784

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	168.70	0 03:06	1.61	1.43	0.90

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								
		Up Dry		Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.65	0.35	0.00	0.00	0.00	0.00

 Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	Flow	Limited
C1	0.01	0.02	0.01	0.22		0.01

Analysis begun on: Mon Feb 13 14:53:10 2023
 Analysis ended on: Mon Feb 13 14:53:10 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_78.4mm_100yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_78.4mm_100yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.201      78.400
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.111      43.153
Surface Runoff ..... 0.090      34.942
Final Storage ..... 0.001      0.305
Continuity Error (%) ..... 0.000
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.090      0.899
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.090      0.899
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
Link C1 (6.68%)
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      : 0.02 sec
Average Time Step      : 4.94 sec
Maximum Time Step      : 5.00 sec
Percent in Steady State : 0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 98.80 %
 3.155 - 1.991 sec : 1.20 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment LPS	mm	mm	mm	mm	mm	mm	mm	10^6 ltr
101	78.40	0.00	0.00	46.84	0.00	31.19	31.19	0.65
93.56 0.398								
102	78.40	0.00	0.00	27.90	21.88	28.62	50.50	0.25
117.13 0.644								

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.05	0.46	409.66	0 03:06	0.46
OF1	OUTFALL	0.04	0.32	409.45	0 03:06	0.32

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	199.27	199.27	0 03:06	0.899	0.899	0.001
OF1	OUTFALL	0.00	199.37	0 03:06	0	0.899	0.000

 Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
J1	JUNCTION	0.16	0.083	0.542

 Node Flooding Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

No nodes were flooded.

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.91	12.05	199.37	0.899
System	99.91	12.05	199.37	0.899

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	199.37	0 03:06	1.86	1.69	0.93

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								
		Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl	
C1	1.00	0.00	0.00	0.00	0.66	0.34	0.00	0.00	0.00	0.00

 Conduit Surcharge Summary

Conduit	Both Ends	Hours Full			Hours Above Normal Flow	Hours Capacity Limited
		Upstream	Dnstream	Full		
C1	0.01	0.16	0.01	0.36	0.01	

Analysis begun on: Mon Feb 13 14:53:48 2023
 Analysis ended on: Mon Feb 13 14:53:48 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 2
 Number of nodes 2
 Number of links 1
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
101	2.07	150.00	0.00	2.0000	SCS_Type_II_88mm_MTO100Yr	J1
102	0.50	198.00	27.90	2.3000	SCS_Type_II_88mm_MTO100Yr	J1

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

```

Analysis Options
*****
Flow Units ..... LPS
Process Models:
  Rainfall/Runoff ..... YES
  RDII ..... NO
  Snowmelt ..... NO
  Groundwater ..... NO
  Flow Routing ..... YES
  Ponding Allowed ..... NO
  Water Quality ..... NO
Infiltration Method ..... HORTON
Flow Routing Method ..... DYNWAVE
Surcharge Method ..... EXTRAN
Starting Date ..... 11/30/2022 00:00:00
Ending Date ..... 12/01/2022 00:00:00
Antecedent Dry Days ..... 0.0
Report Time Step ..... 00:01:00
Wet Time Step ..... 00:05:00
Dry Time Step ..... 00:05:00
Routing Time Step ..... 5.00 sec
Variable Time Step ..... YES
Maximum Trials ..... 8
Number of Threads ..... 1
Head Tolerance ..... 0.001524 m
    
```

```

*****
Runoff Quantity Continuity
*****
Total Precipitation ..... 0.226      88.000
Evaporation Loss ..... 0.000      0.000
Infiltration Loss ..... 0.118      45.847
Surface Runoff ..... 0.108      41.847
Final Storage ..... 0.001      0.310
Continuity Error (%) ..... -0.004
    
```

```

*****
Flow Routing Continuity
*****
Dry Weather Inflow ..... 0.000      0.000
Wet Weather Inflow ..... 0.108      1.077
Groundwater Inflow ..... 0.000      0.000
RDII Inflow ..... 0.000      0.000
External Inflow ..... 0.000      0.000
External Outflow ..... 0.108      1.077
Flooding Loss ..... 0.000      0.000
Evaporation Loss ..... 0.000      0.000
Exfiltration Loss ..... 0.000      0.000
Initial Stored Volume .... 0.000      0.000
Final Stored Volume ..... 0.000      0.000
Continuity Error (%) ..... 0.000
    
```

```

*****
Time-Step Critical Elements
*****
Link C1 (8.69%)
    
```

```

*****
Highest Flow Instability Indexes
*****
All links are stable.
    
```

```

*****
Routing Time Step Summary
*****
Minimum Time Step      :      2.29 sec
Average Time Step      :      4.90 sec
Maximum Time Step     :      5.00 sec
Percent in Steady State :     -0.00
    
```

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

Average Iterations per Step : 2.00
 Percent Not Converging : 0.00
 Time Step Frequencies :
 5.000 - 3.155 sec : 97.82 %
 3.155 - 1.991 sec : 2.18 %
 1.991 - 1.256 sec : 0.00 %
 1.256 - 0.792 sec : 0.00 %
 0.792 - 0.500 sec : 0.00 %

 Subcatchment Runoff Summary

Peak Runoff		Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment	Coeff	mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
101		88.00	0.00	0.00	49.79	0.00	37.83	37.83	0.78
124.32	0.430								
102		88.00	0.00	0.00	29.53	24.56	33.93	58.49	0.29
138.86	0.665								

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.06	0.58	409.78	0 03:06	0.58
OF1	OUTFALL	0.05	0.35	409.48	0 03:06	0.35

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	248.43	248.43	0 03:06	1.08	1.08	-0.000
OF1	OUTFALL	0.00	248.24	0 03:06	0	1.08	0.000

 Node Surcharge Summary

Surcharging occurs when water rises above the top of the highest conduit.

Node	Type	Hours Surcharged	Max. Height Above Crown Meters	Min. Depth Below Rim Meters
J1	JUNCTION	0.27	0.208	0.417

 Node Flooding Summary

645 MARTINS STREET – PRE DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

No nodes were flooded.

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.92	15.61	248.24	1.077
System	99.92	15.61	248.24	1.077

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	248.24	0 03:06	2.28	2.10	0.96

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl	
C1	1.00	0.00	0.00	0.00	0.67	0.33	0.00	0.00	0.00	0.00

 Conduit Surcharge Summary

Conduit	Hours Full			Hours Above Full	
	Both Ends	Upstream	Dnstream	Normal Flow	Capacity Limited
C1	0.01	0.27	0.01	0.61	0.01

Analysis begun on: Mon Feb 13 14:54:37 2023
 Analysis ended on: Mon Feb 13 14:54:37 2023
 Total elapsed time: < 1 sec

Appendix C

POST DEVELOPMENT MODEL AND OUTPUT

645 MARTINS STREET DEVELOPMENT

Table B.1 Parameter Summary Table

Proposed Conditions									
Outlet Location	Model Catchment ID	Description	Area (ha)	Drainage Channel (m)	Flow Length (m)	Gradient (%)	Total Imperv. (%)	Manning's 'n' (Perv.)	CN (Perv.)
Pond/Storage Gallery	201	Townhomes - Main Site	1.74	170	102	2.0	50.4	0.25	77.0
Cork Street	202	Frontage onto Cork Street	0.06	45	13	4.0	64.4	0.25	77.0
Cork Street	203	Frontage onto Cork Street	0.06	33	18	3.0	71.5	0.25	77.0
Cork Street	204	Rear of lots off Martin Street	0.37	110	34	1.3	0.0	0.25	77.0
Cork Street	205	Rear of lots off Martin Street	0.39	198	20	2.3	35.7	0.25	77.0

Table B.2 Site Soils: (as per Ontario Soil Survey Report No. 35 for Wellington County)

Soil Type
Listowel Silt Loam

Hydrologic Soil Group
BC

TABLE OF CURVE NUMBERS (CN's)								
Land Use	Hydrologic Soil Type							Manning's 'n'
	A	AB	B	BC	C	CD	D	
Meadow	50	54	58	64.5	71	74.5	78	0.4
Woodlot	50	55.3	60.5	67	73.5	76.8	80	0.4
Long Grass	55	60	65	72	79	81.5	84	0.3
Lawns	60	65.5	71	77	83	86	89	0.25
Pasture/Range	58	61.5	65	70.5	76	78.5	81	0.17
Crop	66	70	74	78	82	84	86	0.13
Fallow (bare)	77	82	86	89	91	93	94	0.05
Built-up	60	65.5	71	77	83	89	89	0.25
Streets, paved	98	98	98	98	98	98	98	0.01

continuous grass
forests
natural, not maintained
maintained
farm pasture
farm land
idle farm land (bare)
Lawns Proposed

HYDROLOGIC SOIL TYPE (%) - Proposed Conditions								
Catchment	Hydrologic Soil Type							TOTAL
	A	AB	B	BC	C	CD	D	
201	0	0	0	100	0	0	0	100
202	0	0	0	100	0	0	0	100
203	0	0	0	100	0	0	0	100
204	0	0	0	100	0	0	0	100
205	0	0	0	100	0	0	0	100

LAND USE (%) - Proposed Conditions										
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Imperv. Not Connected (Rooftops)	Imperv. Connected	Total
201	0	0	0	50	0	0	0	30.6	19.8	100
202	0	0	0.0	36	0	0	0	49.4	15.0	100
203	0	0	0	29	0	0	0	58.9	12.6	100
204	0	0	0	100	0	0	0	0.0	0.0	100
205	0	0	0	64	0	0	0	35.3	0.5	100

CURVE NUMBER (CN) - Proposed Conditions											
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Built-up	Imperv. Not Connected (Rooftops)	Weighted CN - Pervious	Manning's 'n'
201	64.5	67	72	77	70.5	78	89	77	90	77.0	0.25
202	64.5	67.0	72	77	70.5	78	89	77	90	77.0	0.25
203	64.5	67	72	77	70.5	78	89	77	90	77.0	0.25
204	64.5	67	72	77	70.5	78	89	77	90	77.0	0.25
205	64.5	67	72	77	70.5	78	89	77	90	77.0	0.25

Table B.3: Impervious Area Determination for Subcatchments 201 - 205

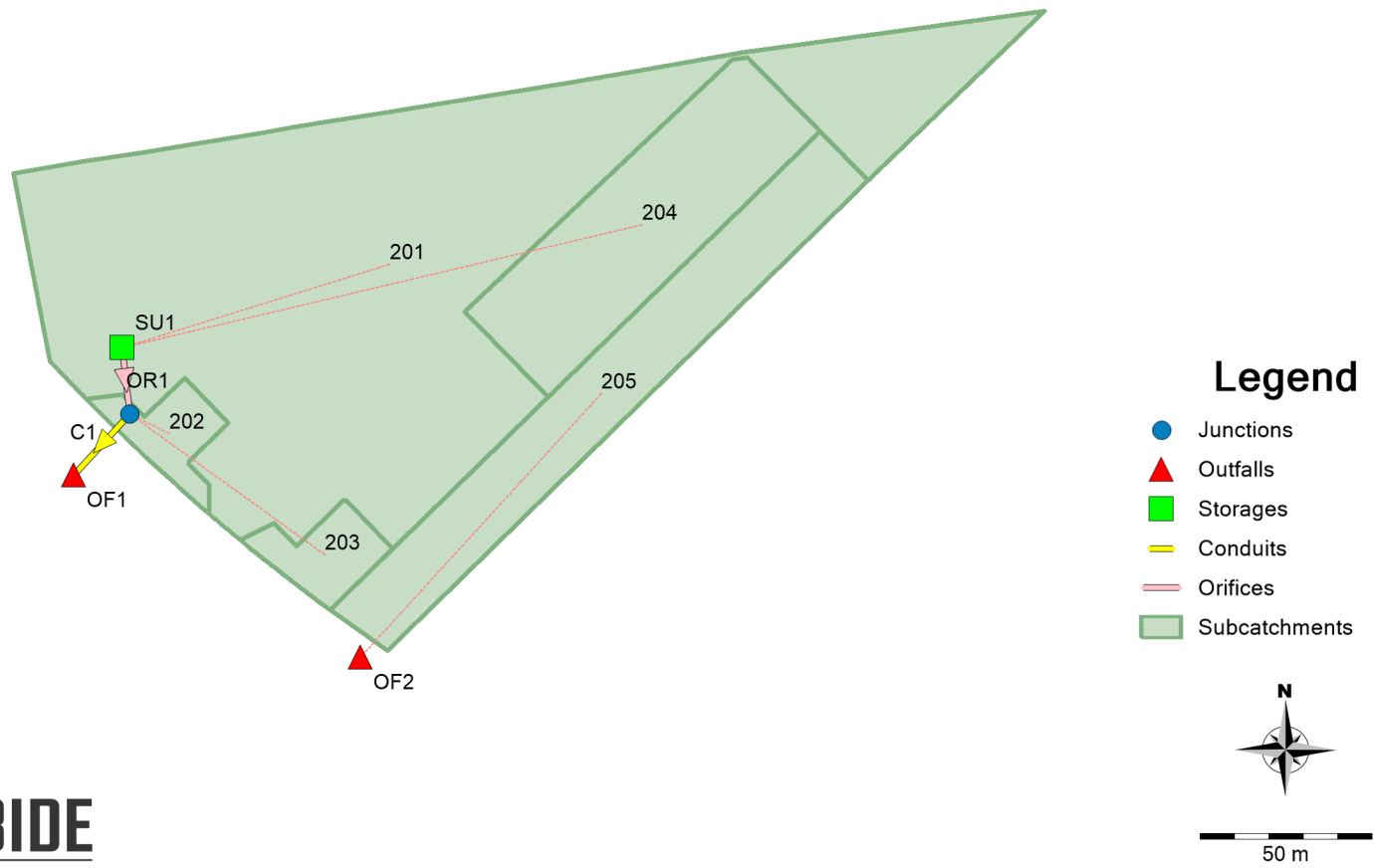
Proposed Conditions

Area of Concern	Total Area (ha)	Impervious Area Connected		Impervious Area Not Connected (Rooftops)		Total (%)
		(ha)	(%)	(ha)	(%)	
201	1.74	0.34	19.8	0.53	30.6	50.4
202	0.06	0.01	15.0	0.03	49.4	64.4
203	0.06	0.01	12.6	0.04	58.9	71.5
204	0.37	0.00	0.0	0.00	0.0	0.0
205	0.39	0.00	0.5	0.14	35.3	35.7

Table B.3 - Impervious Area Determination for Proposed Catchments 201 - 205

Catchment					Imperv. Area	Imperv %
201	1	Road, Sidewalk, Parking	2950.85	m ² @ 100% imperv.	0.30 ha	17.0 %
	13.5	Driveways	36	m ² @ 100% imperv.	0.05 ha	2.8 %
	1	Roof Area	5333	m ² @ 100% imperv.	0.53 ha	30.6 %
					0.88 ha	
202	1	Driveways	42.17	m ² @ 100% imperv.	0.00 ha	7.0 %
	1	Driveways	48.00	m ² @ 100% imperv.	0.00 ha	8.0 %
	1	Roof Area	296.12	m ² @ 100% imperv.	0.03 ha	49.4 %
					0.04 ha	
203	1	Driveways	53.41	m ² @ 100% imperv.	0.01 ha	8.9 %
	1	Driveways	22.04	m ² @ 100% imperv.	0.00 ha	3.7 %
	1	Roof Area	353.35	m ² @ 100% imperv.	0.04 ha	58.9 %
					0.04 ha	
204	0	m of	1	m wide ROW @ 45% imperv.	0.00 ha	0.0 %
	0	Impervious Area	1	m ² @ 100% imperv.	0.00 ha	0.0 %
	0	Roof Area	1	m ² @ 100% imperv.	0.00 ha	0.0 %
					0.00 ha	
205	1	Driveway	18.74	m ² @ 100% imperv.	0.00 ha	0.5 %
	1	Roof Area	1375.04	m ² @ 100% imperv.	0.14 ha	35.3 %
					0.14 ha	

645 MARTINS STREET DEVELOPMENT - POST DEVELOPMENT MODEL SCHEMATIC



645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

[TITLE]

;;Project Title/Notes

[OPTIONS]

```
;;Option Value
FLOW_UNITS LPS
INFILTRATION CURVE_NUMBER
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO

START_DATE 11/30/2022
START_TIME 00:00:00
REPORT_START_DATE 11/30/2022
REPORT_START_TIME 00:00:00
END_DATE 12/1/2022
END_TIME 00:00:00
SWEEP_START 1/1
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:05:00
DRY_STEP 00:05:00
ROUTING_STEP 5
RULE_STEP 00:00:00

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 8
```

[EVAPORATION]

```
;;Data Source Parameters
;;-----
CONSTANT 0.0
DRY_ONLY NO
```

[RAINGAGES]

```
;;Name Format Interval SCF Source
;;-----
SCS_Type_II_38.8mm_2yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_38.8mm_2yr
SCS_Type_II_49.4mm_5yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_49.4mm_5yr
SCS_Type_II_65.3mm_25yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_65.3mm_25yr
SCS_Type_II_71.9mm_50yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_71.9mm_50yr
SCS_Type_II_78.4mm_100yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_78.4mm_100yr
SCS_Type_II_88mm_MTO100Yr INTENSITY 0:06 1.0 TIMESERIES SCS_Type_II_88mm_MTO100Yr
```

[SUBCATCHMENTS]

```
;;Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen SnowPack
;;-----
201 SCS_Type_II_88mm_MTO100Yr SU1 1.74 50.4 170 2 0
202 SCS_Type_II_88mm_MTO100Yr J1 0.0613 64.4 45 4 0
203 SCS_Type_II_88mm_MTO100Yr J1 0.0643 71.5 33 3 0
204 SCS_Type_II_88mm_MTO100Yr SU1 0.37 0 110 1.3 0
205 SCS_Type_II_88mm_MTO100Yr OF2 0.3887 35.7 198 2.3 0
```

[SUBAREAS]

```
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
;;-----
201 0.01 0.25 0.05 0.05 25 OUTLET
202 0.01 0.25 0.05 0.05 25 OUTLET
203 0.01 0.25 0.05 0.05 25 OUTLET
204 0.01 0.25 0.05 0.05 25 OUTLET
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

205 0.01 0.25 0.05 0.05 25 OUTLET

[INFILTRATION]

;;Subcatchment	Param1	Param2	Param3	Param4	Param5
201	77	0.5	7	0	0
202	77	0.5	7	0	0
203	77	0.5	7	0	0
204	77	0.5	7	0	0
205	77	0.5	7	0	0

[JUNCTIONS]

;;Name	Elevation	MaxDepth	InitDepth	SurDepth	Aponded
J1	409.2	1	0	0	0

[OUTFALLS]

;;Name	Elevation	Type	Stage Data	Gated	Route To
OF1	409.13	FREE		NO	
OF2	0	FREE		NO	

[STORAGE]

;;Name	Elev.	MaxDepth	InitDepth	Shape	Curve Name/Params	N/A	Fevap	Psi
SU1	409.26	2.24	0	TABULAR	Pond	0	0	

[CONDUITS]

;;Name	From Node	To Node	Length	Roughness	InOffset	OutOffset	InitFlow
C1	J1	OF1	15.4	0.013	409.2	409.13	0

[ORIFICES]

;;Name	From Node	To Node	Type	Offset	Qcoeff	Gated	CloseTime
OR1	SU1	J1	SIDE	409.41	0.65	NO	0

[XSECTIONS]

;;Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels	Culvert
C1	CIRCULAR	0.375	0	0	0	1	
OR1	CIRCULAR	0.2	0	0	0		

[LOSSES]

;;Link	Kentry	Kexit	Kavg	Flap Gate	Seepage
--------	--------	-------	------	-----------	---------

[CURVES]

;;Name	Type	X-Value	Y-Value
Pond	Storage	0	245.2268939
Pond		0.0254	245.2268939
Pond		0.0508	245.2268939
Pond		0.0762	245.2268939
Pond		0.1016	245.2268939
Pond		0.127	245.2268939
Pond		0.1524	508.041218
Pond		0.1778	506.97322
Pond		0.2032	504.437586
Pond		0.2286	501.6745719
Pond		0.254	499.0482614
Pond		0.2794	495.8474095
Pond		0.3048	492.2813159
Pond		0.3302	488.7128521
Pond		0.3556	484.5372142
Pond		0.381	480.3792707
Pond		0.4064	475.4982856
Pond		0.4318	470.6268367
Pond		0.4572	466.0915875
Pond		0.4826	459.8779537

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

Pond	0.508	453.9385068
Pond	0.5334	447.578751
Pond	0.5588	440.7642347
Pond	0.5842	433.625874
Pond	0.6096	426.767811
Pond	0.635	418.3586015
Pond	0.6604	406.5607377
Pond	0.6858	395.9220997
Pond	0.7112	385.7814932
Pond	0.7366	373.163407
Pond	0.762	358.4181444
Pond	0.7874	340.6824874
Pond	0.8128	317.1375828
Pond	0.8382	278.7962763
Pond	0.8636	264.6248243
Pond	0.889	251.7748544
Pond	0.9144	245.2268939
Pond	0.9398	245.2268939
Pond	0.9652	245.2268939
Pond	0.9906	245.2268939
Pond	1.016	245.2268939
Pond	1.0414	245.2268939
Pond	1.0668	245.2268939
Pond	1.07	5
Pond	1.4	5
Pond	1.49	115
Pond	1.59	315
Pond	1.69	460
Pond	1.79	565
Pond	1.89	650
Pond	1.99	725
Pond	2.09	810
Pond	2.14	850

[TIMESERIES]

```

;;Name          Date          Time          Value
;-----
;SCS_Type_II_38.8mm design storm, total rainfall = 38.8 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_38.8mm_2yr

;SCS_Type_II_49.4mm design storm, total rainfall = 49.4 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_49.4mm_5yr

;SCS_Type_II_65.3mm design storm, total rainfall = 65.3 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_65.3mm_25yr

;SCS_Type_II_71.9mm design storm, total rainfall = 71.9 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_71.9mm_50yr

;SCS_Type_II_78.4mm design storm, total rainfall = 78.4 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_78.4mm_100yr

;SCS_Type_II_88mm design storm, total rainfall = 88 mm, rain interval = 6 minutes, rain units = mm/hr.
SCS_Type_II_88mm_MTO100Yr
    
```

[REPORT]

```

;;Reporting Options
INPUT          YES
CONTROLS       NO
SUBCATCHMENTS ALL
NODES         ALL
LINKS         ALL
    
```

[TAGS]

[MAP]

```

DIMENSIONS      520556.46295      4868691.77995      520883.91205      4868897.01905
UNITS           Meters
    
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
Number of subcatchments ... 5
Number of nodes 4
Number of links 2
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_38.8mm_2yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_38.8mm_2yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_38.8mm_2yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_38.8mm_2yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_38.8mm_2yr	OF2

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 11/30/2022 00:00:00
Ending Date 12/01/2022 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:05:00
Dry Time Step 00:05:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.102	38.800
Evaporation Loss	0.000	0.000
Infiltration Loss	0.040	15.078
Surface Runoff	0.062	23.699
Final Storage	0.000	0.079
Continuity Error (%)	-0.144	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.062	0.622
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.058	0.581
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	0.000	

Time-Step Critical Elements

None

Highest Flow Instability Indexes

All links are stable.

Routing Time Step Summary

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

```

*****
Minimum Time Step      :    4.50 sec
Average Time Step     :    5.00 sec
Maximum Time Step     :    5.00 sec
Percent in Steady State :    0.00
Average Iterations per Step :    2.00
Percent Not Converging :    0.00
Time Step Frequencies :
    5.000 - 3.155 sec : 100.00 %
    3.155 - 1.991 sec :    0.00 %
    1.991 - 1.256 sec :    0.00 %
    1.256 - 0.792 sec :    0.00 %
    0.792 - 0.500 sec :    0.00 %
    
```

```

*****
Subcatchment Runoff Summary
*****
    
```

Peak Runoff		Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Runoff	Coeff	mm	mm	mm	mm	mm	mm	mm	10^6 ltr
Subcatchment									
LPS									
201		38.80	0.00	0.00	12.95	19.62	6.23	25.85	0.45
194.08	0.666								
202		38.80	0.00	0.00	9.14	24.99	4.66	29.65	0.02
9.70	0.764								
203		38.80	0.00	0.00	7.32	27.76	3.73	31.49	0.02
10.74	0.812								
204		38.80	0.00	0.00	25.92	0.00	12.77	12.77	0.05
10.52	0.329								
205		38.80	0.00	0.00	16.51	13.85	8.41	22.26	0.09
39.51	0.574								

```

*****
Node Depth Summary
*****
    
```

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.04	0.18	409.38	0 03:06	0.18
OF1	OUTFALL	0.03	0.18	409.31	0 03:06	0.18
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.19	0.55	409.81	0 03:19	0.55

```

*****
Node Inflow Summary
*****
    
```

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	20.45	59.45	0 03:06	0.0385	0.495	0.002
OF1	OUTFALL	0.00	59.45	0 03:06	0	0.495	0.000
OF2	OUTFALL	39.51	39.51	0 03:00	0.0866	0.0866	0.000
SU1	STORAGE	200.75	200.75	0 03:00	0.497	0.497	0.001

```

*****
Node Surcharge Summary
    
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 2 YEAR DESIGN STORM

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcmt Full	Evap Pcmt Loss	Exfil Pcmt Loss	Maximum Volume 1000 m3	Max Pcmt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.061	7	0	0	0.230	27	0 03:19	49.19

Outfall Loading Summary

Outfall Node	Flow Freq Pcmt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.81	5.74	59.45	0.495
OF2	36.73	2.72	39.51	0.087
System	68.27	8.46	93.22	0.581

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	59.45	0 03:06	1.13	0.50	0.48
OR1	ORIFICE	49.19	0 03:19			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.62	0.38	0.00	0.00	0.00

Conduit Surge Summary

No conduits were surcharged.

Analysis begun on: Mon Feb 13 15:29:59 2023
 Analysis ended on: Mon Feb 13 15:29:59 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
Number of subcatchments ... 5
Number of nodes 4
Number of links 2
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_49.4mm_5yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_49.4mm_5yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_49.4mm_5yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_49.4mm_5yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_49.4mm_5yr	OF2

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 11/30/2022 00:00:00
Ending Date 12/01/2022 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:05:00
Dry Time Step 00:05:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001524 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.130	49.400
Evaporation Loss	0.000	0.000
Infiltration Loss	0.046	17.596
Surface Runoff	0.083	31.795
Final Storage	0.000	0.080
Continuity Error (%)	-0.143	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.084	0.835
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.079	0.794
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	0.000	

Time-Step Critical Elements

Link C1 (3.88%)

Highest Flow Instability Indexes

All links are stable.

Routing Time Step Summary

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

```

*****
Minimum Time Step      :      4.63 sec
Average Time Step     :      4.99 sec
Maximum Time Step     :      5.00 sec
Percent in Steady State :      0.00
Average Iterations per Step :      2.00
Percent Not Converging :      0.00
Time Step Frequencies :
  5.000 - 3.155 sec   :    100.00 %
  3.155 - 1.991 sec   :      0.00 %
  1.991 - 1.256 sec   :      0.00 %
  1.256 - 0.792 sec   :      0.00 %
  0.792 - 0.500 sec   :      0.00 %
    
```

```

*****
Subcatchment Runoff Summary
*****
    
```

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
201		49.40	0.00	0.00	15.10	24.98	9.33	34.31	0.60
256.49	0.695								
202		49.40	0.00	0.00	10.65	31.82	6.93	38.75	0.02
12.80	0.784								
203		49.40	0.00	0.00	8.53	35.34	5.55	40.89	0.03
14.06	0.828								
204		49.40	0.00	0.00	30.32	0.00	18.97	18.97	0.07
18.80	0.384								
205		49.40	0.00	0.00	19.24	17.64	12.50	30.14	0.12
55.04	0.610								

```

*****
Node Depth Summary
*****
    
```

Node	Type	Average Depth	Maximum Depth	Maximum HGL	Time of Max Occurrence	Reported Max Depth
		Meters	Meters	Meters	days hr:min	Meters
J1	JUNCTION	0.04	0.21	409.41	0 03:06	0.21
OF1	OUTFALL	0.04	0.20	409.33	0 03:06	0.20
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.21	0.73	409.99	0 03:23	0.73

```

*****
Node Inflow Summary
*****
    
```

Node	Type	Maximum Lateral Inflow	Maximum Total Inflow	Time of Max Occurrence	Lateral Inflow Volume	Total Inflow Volume	Flow Balance Error
		LPS	LPS	days hr:min	10^6 ltr	10^6 ltr	Percent
J1	JUNCTION	26.87	75.40	0 03:06	0.0501	0.677	0.001
OF1	OUTFALL	0.00	75.40	0 03:06	0	0.677	0.000
OF2	OUTFALL	55.04	55.04	0 03:00	0.117	0.117	0.000
SU1	STORAGE	269.05	269.05	0 03:00	0.668	0.668	0.001

```

*****
Node Surcharge Summary
    
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 5 YEAR DESIGN STORM

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcmt Full	Evap Pcmt Loss	Exfil Pcmt Loss	Maximum Volume 1000 m3	Max Pcmt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.070	8	0	0	0.308	36	0 03:23	62.89

Outfall Loading Summary

Outfall Node	Flow Freq Pcmt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.85	7.91	75.40	0.677
OF2	37.39	3.70	55.04	0.117
System	68.62	11.61	124.63	0.794

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	75.40	0 03:06	1.22	0.64	0.55
OR1	ORIFICE	62.89	0 03:23			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.63	0.37	0.00	0.00	0.00

Conduit Surge Summary

No conduits were surcharged.

Analysis begun on: Mon Feb 13 15:30:54 2023
 Analysis ended on: Mon Feb 13 15:30:54 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
Number of subcatchments ... 5
Number of nodes 4
Number of links 2
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_65.3mm_25yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_65.3mm_25yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_65.3mm_25yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_65.3mm_25yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_65.3mm_25yr	OF2

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

based on results found at every computational time step,
not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 11/30/2022 00:00:00
Ending Date 12/01/2022 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:05:00
Dry Time Step 00:05:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001524 m

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.171	65.300
Evaporation Loss	0.000	0.000
Infiltration Loss	0.054	20.697
Surface Runoff	0.117	44.616
Final Storage	0.000	0.080
Continuity Error (%)	-0.142	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.117	1.172
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.113	1.131
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	0.003	

Time-Step Critical Elements

Link C1 (8.15%)

Highest Flow Instability Indexes

All links are stable.

Routing Time Step Summary

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

```

*****
Minimum Time Step      :      1.19 sec
Average Time Step      :      4.96 sec
Maximum Time Step      :      5.00 sec
Percent in Steady State :     -0.00
Average Iterations per Step :      2.00
Percent Not Converging :      0.00
Time Step Frequencies :
  5.000 - 3.155 sec    :     99.99 %
  3.155 - 1.991 sec    :      0.00 %
  1.991 - 1.256 sec    :      0.00 %
  1.256 - 0.792 sec    :      0.01 %
  0.792 - 0.500 sec    :      0.00 %
    
```

```

*****
Subcatchment Runoff Summary
*****
    
```

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
LPS									
201		65.30	0.00	0.00	17.78	33.01	14.54	47.56	0.83
357.51	0.728								
202		65.30	0.00	0.00	12.49	42.06	10.76	52.82	0.03
17.65	0.809								
203		65.30	0.00	0.00	10.00	46.71	8.61	55.32	0.04
19.21	0.847								
204		65.30	0.00	0.00	35.65	0.00	29.55	29.55	0.11
34.82	0.453								
205		65.30	0.00	0.00	22.57	23.32	19.41	42.73	0.17
81.15	0.654								

```

*****
Node Depth Summary
*****
    
```

Node	Type	Average Depth	Maximum Depth	Maximum HGL	Time of Max Occurrence	Reported Max Depth
		Meters	Meters	Meters	days hr:min	Meters
J1	JUNCTION	0.05	0.28	409.48	0 03:09	0.27
OF1	OUTFALL	0.05	0.25	409.38	0 03:09	0.25
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.26	1.56	410.82	0 03:20	1.56

```

*****
Node Inflow Summary
*****
    
```

Node	Type	Maximum Lateral Inflow	Maximum Total Inflow	Time of Max Occurrence	Lateral Inflow Volume	Total Inflow Volume	Flow Balance Error
		LPS	LPS	days hr:min	10^6 ltr	10^6 ltr	Percent
J1	JUNCTION	36.87	111.61	0 03:09	0.068	0.964	0.000
OF1	OUTFALL	0.00	113.68	0 03:09	0	0.964	0.000
OF2	OUTFALL	81.15	81.15	0 03:00	0.166	0.166	0.000
SU1	STORAGE	382.39	382.39	0 03:00	0.937	0.937	0.004

```

*****
Node Surcharge Summary
    
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 25 YEAR DESIGN STORM

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcmt Full	Evap Pcmt Loss	Exfil Pcmt Loss	Maximum Volume 1000 m3	Max Pcmt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.086	10	0	0	0.421	49	0 03:20	103.58

Outfall Loading Summary

Outfall Node	Flow Freq Pcmt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.87	11.90	113.68	0.964
OF2	38.39	5.46	81.15	0.166
System	69.13	17.36	173.12	1.131

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	113.68	0 03:09	1.40	0.96	0.70
OR1	ORIFICE	103.58	0 03:20			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.65	0.35	0.00	0.00	0.00

Conduit Surge Summary

No conduits were surcharged.

Analysis begun on: Mon Feb 13 15:31:43 2023
 Analysis ended on: Mon Feb 13 15:31:43 2023
 Total elapsed time: < 1 sec

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 5
 Number of nodes 4
 Number of links 2
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_71.9mm_50yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_71.9mm_50yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_71.9mm_50yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_71.9mm_50yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_71.9mm_50yr	OF2

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS

Process Models:

Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO

Infiltration Method CURVE_NUMBER

Flow Routing Method DYNWAVE

Surcharge Method EXTRAN

Starting Date 11/30/2022 00:00:00

Ending Date 12/01/2022 00:00:00

Antecedent Dry Days 0.0

Report Time Step 00:01:00

Wet Time Step 00:05:00

Dry Time Step 00:05:00

Routing Time Step 5.00 sec

Variable Time Step YES

Maximum Trials 8

Number of Threads 1

Head Tolerance 0.001524 m

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.189	71.900
Evaporation Loss	0.000	0.000
Infiltration Loss	0.057	21.772
Surface Runoff	0.132	50.150
Final Storage	0.000	0.080
Continuity Error (%)	-0.142	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.132	1.317
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.127	1.275
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	0.100	

Time-Step Critical Elements

Link C1 (9.55%)

Highest Flow Instability Indexes

All links are stable.

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

Routing Time Step Summary

```

*****
Minimum Time Step      :      3.71 sec
Average Time Step     :      4.94 sec
Maximum Time Step     :      5.00 sec
Percent in Steady State :      0.00
Average Iterations per Step :      2.00
Percent Not Converging :      0.02
Time Step Frequencies :
  5.000 - 3.155 sec   :    100.00 %
  3.155 - 1.991 sec   :      0.00 %
  1.991 - 1.256 sec   :      0.00 %
  1.256 - 0.792 sec   :      0.00 %
  0.792 - 0.500 sec   :      0.00 %
    
```

Subcatchment Runoff Summary

```

*****

```

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
201		71.90	0.00	0.00	18.69	36.35	16.91	53.26	0.93
401.86	0.741								
202		71.90	0.00	0.00	13.14	46.32	12.47	58.78	0.04
19.72	0.818								
203		71.90	0.00	0.00	10.52	51.43	9.98	61.41	0.04
21.39	0.854								
204		71.90	0.00	0.00	37.54	0.00	34.26	34.26	0.13
42.48	0.477								
205		71.90	0.00	0.00	23.78	25.67	22.45	48.12	0.19
92.77	0.669								

Node Depth Summary

```

*****

```

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.06	0.30	409.50	0 03:05	0.30
OF1	OUTFALL	0.05	0.27	409.40	0 03:05	0.27
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.29	1.70	410.96	0 03:22	1.70

Node Inflow Summary

```

*****

```

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	41.11	130.54	0 03:06	0.0756	1.09	-0.000
OF1	OUTFALL	0.00	131.17	0 03:05	0	1.09	0.000
OF2	OUTFALL	92.77	92.77	0 03:00	0.187	0.187	0.000
SU1	STORAGE	432.98	432.98	0 03:00	1.05	1.05	0.125

```

*****

```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.093	11	0	0	0.475	55	0 03:22	109.10

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.88	13.77	131.17	1.087
OF2	38.67	6.29	92.77	0.187
System	69.27	20.06	209.60	1.275

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	131.17	0 03:05	1.47	1.11	0.76
OR1	ORIFICE	109.10	0 03:22			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Time in Flow Class					
		Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.65	0.35	0.00	0.00	0.00

Conduit Surcharge Summary

Conduit	Hours Full Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Normal Flow	Hours Full Capacity Limited

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 50 YEAR DESIGN STORM

C1 0.01 0.01 0.01 0.08 0.01

Analysis begun on: Mon Feb 13 15:32:46 2023
Analysis ended on: Mon Feb 13 15:32:46 2023
Total elapsed time: < 1 sec

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 5
 Number of nodes 4
 Number of links 2
 Number of pollutants 0
 Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_78.4mm_100yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_78.4mm_100yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_78.4mm_100yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_78.4mm_100yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_78.4mm_100yr	OF2

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Surcharge Method EXTRAN
Starting Date 11/30/2022 00:00:00
Ending Date 12/01/2022 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:05:00
Dry Time Step 00:05:00
Routing Time Step 5.00 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 1
Head Tolerance 0.001524 m

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.206	78.400
Evaporation Loss	0.000	0.000
Infiltration Loss	0.060	22.748
Surface Runoff	0.146	55.683
Final Storage	0.000	0.080
Continuity Error (%)	-0.142	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.146	1.462
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.142	1.423
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	-0.114	

Time-Step Critical Elements

Link C1 (11.04%)

Highest Flow Instability Indexes

All links are stable.

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

Routing Time Step Summary

```

*****
Minimum Time Step      :      3.57 sec
Average Time Step      :      4.91 sec
Maximum Time Step      :      5.00 sec
Percent in Steady State :      0.00
Average Iterations per Step :      2.00
Percent Not Converging :      0.02
Time Step Frequencies :
  5.000 - 3.155 sec    :    100.00 %
  3.155 - 1.991 sec    :      0.00 %
  1.991 - 1.256 sec    :      0.00 %
  1.256 - 0.792 sec    :      0.00 %
  0.792 - 0.500 sec    :      0.00 %
    
```

Subcatchment Runoff Summary

```

*****
    
```

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
201		78.40	0.00	0.00	19.54	39.63	19.29	58.92	1.03
446.82	0.752								
202		78.40	0.00	0.00	13.73	50.50	14.20	64.70	0.04
21.77	0.825								
203		78.40	0.00	0.00	10.99	56.08	11.37	67.45	0.04
23.56	0.860								
204		78.40	0.00	0.00	39.18	0.00	39.13	39.13	0.14
50.50	0.499								
205		78.40	0.00	0.00	24.84	28.00	25.58	53.58	0.21
104.58	0.683								

Node Depth Summary

```

*****
    
```

Node	Type	Average Depth	Maximum Depth	Maximum HGL	Time of Max Occurrence	Reported Max Depth
		Meters	Meters	Meters	days hr:min	Meters
J1	JUNCTION	0.06	0.32	409.52	0 03:03	0.31
OF1	OUTFALL	0.05	0.28	409.41	0 03:06	0.28
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.32	1.82	411.08	0 03:25	1.82

Node Inflow Summary

```

*****
    
```

Node	Type	Maximum Lateral Inflow	Maximum Total Inflow	Time of Max Occurrence	Lateral Inflow Volume	Total Inflow Volume	Flow Balance Error
		LPS	LPS	days hr:min	10^6 ltr	10^6 ltr	Percent
J1	JUNCTION	45.34	139.95	0 03:06	0.0831	1.21	-0.000
OF1	OUTFALL	0.00	139.92	0 03:06	0	1.21	0.000
OF2	OUTFALL	104.58	104.58	0 03:00	0.209	0.209	0.000
SU1	STORAGE	484.65	484.65	0 03:00	1.17	1.17	-0.142

```

*****
    
```

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.102	12	0	0	0.539	63	0 03:25	113.52

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.89	15.71	139.92	1.214
OF2	39.09	7.11	104.58	0.209
System	69.49	22.83	231.28	1.423

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	139.92	0 03:06	1.50	1.18	0.79
OR1	ORIFICE	113.52	0 03:25			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Time in Flow Class					
		Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.66	0.34	0.00	0.00	0.00

Conduit Surcharge Summary

Conduit	Hours Full Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Normal Flow	Hours Full Capacity Limited

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – 100 YEAR DESIGN STORM

C1 0.01 0.01 0.01 0.40 0.01

Analysis begun on: Mon Feb 13 15:33:51 2023

Analysis ended on: Mon Feb 13 15:33:51 2023

Total elapsed time: < 1 sec

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.015)

Element Count

Number of rain gages 6
 Number of subcatchments ... 5
 Number of nodes 4
 Number of links 2
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
SCS_Type_II_38.8mm_2yr	SCS_Type_II_38.8mm_2yr	INTENSITY	6 min.
SCS_Type_II_49.4mm_5yr	SCS_Type_II_49.4mm_5yr	INTENSITY	6 min.
SCS_Type_II_65.3mm_25yr	SCS_Type_II_65.3mm_25yr	INTENSITY	6 min.
SCS_Type_II_71.9mm_50yr	SCS_Type_II_71.9mm_50yr	INTENSITY	6 min.
SCS_Type_II_78.4mm_100yr	SCS_Type_II_78.4mm_100yr	INTENSITY	6 min.
SCS_Type_II_88mm_MTO100Yr	SCS_Type_II_88mm_MTO100Yr	INTENSITY	6 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
201	1.74	170.00	50.40	2.0000	SCS_Type_II_88mm_MTO100Yr	SU1
202	0.06	45.00	64.40	4.0000	SCS_Type_II_88mm_MTO100Yr	J1
203	0.06	33.00	71.50	3.0000	SCS_Type_II_88mm_MTO100Yr	J1
204	0.37	110.00	0.00	1.3000	SCS_Type_II_88mm_MTO100Yr	SU1
205	0.39	198.00	35.70	2.3000	SCS_Type_II_88mm_MTO100Yr	OF2

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
J1	JUNCTION	409.20	1.00	0.0	
OF1	OUTFALL	409.13	0.38	0.0	
OF2	OUTFALL	0.00	0.00	0.0	
SU1	STORAGE	409.26	2.24	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
C1	J1	OF1	CONDUIT	15.4	0.4546	0.0130
OR1	SU1	J1	ORIFICE			

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.38	0.11	0.09	0.38	1	118.22

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Surcharge Method EXTRAN
 Starting Date 11/30/2022 00:00:00
 Ending Date 12/01/2022 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:05:00
 Dry Time Step 00:05:00
 Routing Time Step 5.00 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 1
 Head Tolerance 0.001524 m

*****	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
*****	-----	-----
Total Precipitation	0.231	88.000
Evaporation Loss	0.000	0.000
Infiltration Loss	0.063	24.047
Surface Runoff	0.168	63.998
Final Storage	0.000	0.080
Continuity Error (%)	-0.143	

*****	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.168	1.681
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.164	1.638
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.004	0.041
Continuity Error (%)	0.091	

Time-Step Critical Elements

Link C1 (13.24%)

Highest Flow Instability Indexes

All links are stable.

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

Routing Time Step Summary

```

*****
Minimum Time Step      :    1.19 sec
Average Time Step      :    4.88 sec
Maximum Time Step      :    5.00 sec
Percent in Steady State :    0.00
Average Iterations per Step :    2.00
Percent Not Converging :    0.01
Time Step Frequencies :
  5.000 - 3.155 sec    :   99.99 %
  3.155 - 1.991 sec    :    0.00 %
  1.991 - 1.256 sec    :    0.00 %
  1.256 - 0.792 sec    :    0.01 %
  0.792 - 0.500 sec    :    0.00 %
    
```

Subcatchment Runoff Summary

Peak Runoff	Runoff Coeff	Total Precip	Total Runon	Total Evap	Total Infil	Imperv Runoff	Perv Runoff	Total Runoff	Total Runoff
Subcatchment		mm	mm	mm	mm	mm	mm	mm	10^6 ltr
201		88.00	0.00	0.00	20.65	44.48	22.94	67.42	1.17
515.35	0.766								
202		88.00	0.00	0.00	14.50	56.69	16.85	73.54	0.05
24.84	0.836								
203		88.00	0.00	0.00	11.61	62.95	13.49	76.44	0.05
26.80	0.869								
204		88.00	0.00	0.00	41.44	0.00	46.48	46.48	0.17
63.04	0.528								
205		88.00	0.00	0.00	26.24	31.43	30.36	61.79	0.24
122.56	0.702								

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
J1	JUNCTION	0.07	0.34	409.54	0 03:01	0.33
OF1	OUTFALL	0.06	0.28	409.41	0 03:02	0.28
OF2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
SU1	STORAGE	0.36	1.97	411.23	0 03:28	1.97

Node Inflow Summary

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
J1	JUNCTION	51.64	148.91	0 03:02	0.0943	1.4	-0.000
OF1	OUTFALL	0.00	148.89	0 03:02	0	1.4	0.000
OF2	OUTFALL	122.56	122.56	0 03:00	0.241	0.241	0.000
SU1	STORAGE	564.05	564.05	0 03:00	1.35	1.35	0.114

645 MARTINS STREET – POST DEVELOPMENT SWM MODELLING – MTO 100 YEAR DESIGN STORM

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
SU1	0.116	14	0	0	0.634	74	0 03:28	118.63

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	99.90	18.53	148.89	1.397
OF2	39.65	8.36	122.56	0.241
System	69.77	26.89	264.69	1.638

Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
C1	CONDUIT	148.89	0 03:02	1.53	1.26	0.82
OR1	ORIFICE	118.63	0 03:28			1.00

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Dry	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl
C1	1.00	0.00	0.00	0.00	0.67	0.33	0.00	0.00	0.00

Conduit Surcharge Summary

Conduit	Hours Full Both Ends	Hours Full Upstream	Hours Full Dnstream	Hours Above Normal Flow	Hours Full Capacity Limited

