

This memo has been prepared to support the Zoning By-Law Approval application for the Site. This memo should be read in conjunction with the Functional Servicing Report prepared by Husson Engineering Management (Husson, 2025).

2.0 Soils Evaluation

A geotechnical investigation has been conducted on the Site and a preliminary report has been prepared by Soil Engineers Ltd. dated September 2025. The field investigation included drilling and sampling of 10 boreholes to a maximum depth of 6.6 meters below ground surface (mbgs) in July 2025. In general, the following soil stratigraphy was encountered during drilling:

- Topsoil up to 36 cm thick
- Sandy Silt to Silty Sand Till below the topsoil layer to 6.6 mbgs (except Borehole 9).
- Silt below the topsoil and sandy silt, ranging from loose to compact; loose silt was encountered at a depth of 0.6 mbgs and gradually becomes compacted with depth. Wet silt is encountered below 4.6 mbgs (Boreholes 1, 6 – 10).
- Sand is found below the topsoil at Boreholes 2 – 4 or below the sandy silt till or silt in all other borehole locations. The deeper sands are found to be wet below 4.6 mbgs.

Localized hydraulic conductivity testing and/or infiltration testing will be complete at the requisite time, however the anticipated T-time of the soils can be estimated using the grain size distribution curves presented in the Geotechnical Investigation Report and comparing the curves to the Supplementary Standard SB-6 of the Ontario Building Code (OBC). Based on the curves presented, the soil type is interpreted to be ML per the Unified Soil Classification System (USCS). According to the Supplementary Standard SB-6, ML soils are described as silty or clayey fine sands or clayey silts with slight plasticity, with a medium to low permeability and a percolation rate of 20 to 50 min/cm. A T-time estimate of 35 min/cm has been assigned for preliminary design purposes.

3.0 Sewage System Design Flow

Following review of the Site Plans and discussions with Fieldgate, Industrial Buildings 1 to 3 will each contain two (2) industrial warehousing/offices and will consist of 15,390 m² total warehouse space and 810 m² total office space. Industrial Condo Buildings 4 and 5 will each contain eleven (11) industrial warehousing/office units and will consist of 9,384 m² total warehouse space and 1,043 m² total office space. Industrial Condo Building 6 will contain eleven (11) industrial warehousing/office units and will consist of 8,833 m² total warehouse space and 981 m² total office space. In addition, it is estimated that Industrial Buildings 1 to 3 will contain up to eighteen (18) water closets, and Industrial Condo Buildings 4 to 6 will contain up to eleven (11) water closets each (one (1) water closet per unit), respectively. For Concept Plan B, Industrial Building 7 will contain two (2) industrial warehousing/office units and Industrial Building 8 will include warehousing/offices. Each of Buildings 7 and 8 will consist of 18,535 m² total warehouse space and 975 m² total office space. In addition, it is estimated that Industrial Buildings 7 and 8 will each contain up to twenty-two (22) water closets.

As the Site Plans are in the conceptual stage, a conservative approach was used to estimate the office area configuration and the number of employees working in each building per day. It is assumed that per the office areas described above, 50% of those stated office areas will be designated as officed space, kitchen and meeting rooms, with the remaining 50% of the office area being allocated for storage and other uses.

Maximum daily design sewage flows for non-residential occupancies are calculated using Table 8.2.1.3.B of the OBC. Table 1 summarizes the total daily design sewage flow for Concept Plan A. Table 2 summarizes the total daily design sewage flow for Concept Plan B. Refer to Attachment A for detailed sewage design flow calculations.

Table 1: Total Maximum Daily Sanitary Sewage Design Flow – Concept Plan A

Building	Area (m ²)	Occupancy	Unit	Unit Flow (L)	Total Flow (L/day)	Total Flow (L/day)
Industrial Building 1	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Building 2	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Building 3	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Condo Building 4	9,384	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	522	Office	Per each 9.3 m ² of floor space	75	56	4,200
Industrial Condo Building 5	9,384	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	522	Office	Per each 9.3 m ² of floor space	75	56	4,200
Industrial Condo Building 6	8,833	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	491	Office	Per each 9.3 m ² of floor space	75	52	3,900
Total Daily Sewage Design Flow:						130,275
Sewage System Design Flow:						130,500

Table 2: Total Maximum Daily Sanitary Sewage Design Flow – Concept Plan B

Building	Area (m ²)	Occupancy	Unit	Unit Flow (L)	Total Flow (L/day)	Total Flow (L/day)
Industrial Building 1	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Building 2	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Building 3	15,390	Warehouse	Per water closet, and	950	18	17,100
			Per loading bay	150	46	6,900
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225
Industrial Condo Building 4	9,384	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	522	Office	Per each 9.3 m ² of floor space	75	56	4,200
Industrial Condo Building 5	9,384	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	522	Office	Per each 9.3 m ² of floor space	75	56	4,200
Industrial Condo Building 6	8,833	Warehouse	Per water closet, and	950	11	10,450
			Per loading bay	150	11	1,650
	491	Office	Per each 9.3 m ² of floor space	75	52	3,900
Industrial Building 7	18,535	Warehouse	Per water closet, and	950	22	20,900
			Per loading bay	150	33	4,950
	488	Office	Per each 9.3 m ² of floor space	75	52	3,935
Industrial Building 8	18,535	Warehouse	Per water closet, and	950	22	20,900
			Per loading bay	150	33	4,950

Building	Area (m ²)	Occupancy	Unit	Unit Flow (L)	Total Flow (L/day)	Total Flow (L/day)
	488	Office	Per each 9.3 m ² of floor space	75	52	3935
Total Daily Sewage Design Flow:						189,720
Sewage System Design Flow:						190,000

As shown in Tables 1 and 2, the sewage system design flow ranges from 130,500 L/day (Concept Plan A) to 190,000 L/day (Concept Plan B). Properties with a total daily design sanitary sewage flow exceeding 10,000 L/day are subject to Section 53 of the Ontario Water Resources Act and require an Environmental Compliance Approval (ECA) issued by the Ministry of Environment, Conservation and Parks (MECP). Given that the maximum daily design sewage flow is over 10,000 L/day in both concepts, an ECA will be required for the onsite sewage system.

An ECA application form, design brief and supporting studies will be prepared in upcoming stages of the project and submitted to the MECP for review at the appropriate time.

4.0 Proposed Onsite Sewage System

Potential onsite sewage system options for the Site include an onsite sewage system with subsurface disposal via leaching bed, or an onsite sewage system with discharge of treated effluent to a surface water receiver. According to the Preliminary Site Grading Plan prepared by Husson, a stormwater outlet is proposed to discharge to a dry ditch located in the northeast corner of the property along Ellis Road towards Puslinch Golf Club. Based on a review of available online mapping tools, there are two nearby water bodies that could be the receiver depending on the ultimate drainage path of the dry ditch; Irish Creek to the north and Puslinch Lake to the southeast. However, due to the current uncertainty regarding the ultimate receiver for the surface runoff outlet, an onsite sewage system with subsurface disposal via leaching bed is the preferred solution for the Site.

Centralized onsite sewage system(s) are proposed, which will consist of an advanced treatment system with discharge of treated effluent to a leaching bed. The centralized sewage system(s) will receive sanitary sewage from each building through a combination of gravity sewers, sewage pumping stations, and forcemains, where required. Refer to the Site Servicing Plans within the Functional Servicing Report, prepared by Husson for an overview of the collection system.

For Concept Plan A, one (1) centralized sewage treatment block is proposed. For Concept Plan B, two (2) sewage treatment blocks are proposed (1 block for Buildings 1 through 6 and a 1 block for Buildings 7 and 8). The treatment blocks are shown on the attached Private Servicing Strategy figures.

A hydrogeological study, which is currently ongoing, and impact assessment in accordance with Chapter 22 of the MECP Design Guidelines for Sewage Works (2008) will be required to establish the effluent criteria that the treatment system(s) will need to achieve prior to discharge. The following effluent limits are anticipated (Table 3).

Table 3: Anticipated Effluent Criteria for Advanced Treatment System

Parameter	Effluent Limits (Monthly Average)
Carbonaceous Biological Oxygen Demand	10 mg/L
Total Suspended Solids	10 mg/L
Nitrate-Nitrogen	5 mg/L

The effluent criteria will be refined through the completion of the hydrogeological study and impact assessment and are subject to the approval of the MECP.

4.1 Proposed Treatment Unit

An advanced treatment unit is proposed for the onsite sewage treatment system. There are different technologies available on the market that can achieve the anticipated effluent criteria noted in Table 3, including a biological trickling filter with enhanced denitrification as designed and supplied by Waterloo Biofilter or approved alternative.

Other technologies that are also able to achieve this effluent quality include a Moving Bed Biofilter Reactor (MBBR) system as designed and supplied by BNA Inc. Selection of the treatment supplier and detailed design of the treatment system will be completed in the upcoming stages of the project, after the impact assessment has been completed and effluent criteria have been approved by MECP. The proposed treatment system blocks are shown on the Private Servicing Strategy figures.

4.2 Proposed Leaching Bed

The treated effluent from the advanced treatment system will be discharged to a leaching bed. For Concept Plan A, two (2) equally sized Type A dispersal beds are proposed. For Concept Plan B, a third Type A dispersal bed is proposed.

The Type A dispersal beds will consist of a stone and sand layer as sized in accordance with Section 8.7.7 of the OBC. The stone shall be washed septic stone meeting the gradation requirements of Table 8.7.3.3 and installed to accommodate the effluent distribution piping. The sand shall be poorly graded material with less than 5% silt content per 8.7.7.1(4)(a). Tables 4 and 5 summarize the Type A dispersal bed sizing for each concept plan. Detailed calculations are included in Attachment A.

Table 4: Type A Dispersal Bed Sizing – Concept Plan A

	Type A Dispersal Beds 1 & 2 (each)	
	Minimum Area (m ²)	Provided Area (m ²)
Stone Layer (m ²)	1,305	1,350
Sand Layer (m ²)	5,710	5,890

Table 5: Type A Dispersal Bed Sizing – Concept Plan B

	Type A Dispersal Beds 1 & 2 (each)		Type A Dispersal Bed 3	
	Minimum Area (m ²)	Provided Area (m ²)	Minimum Area (m ²)	Provided Area (m ²)
Stone Layer (m ²)	1,305	1,350	1,190	1,260
Sand Layer (m ²)	5,710	5,890	5,206	5,250

The proposed leaching bed blocks are shown on the Private Servicing Strategy figures.

4.3 Preliminary Cost Estimate

A preliminary cost estimate for the onsite sewage system for each concept is provided below in Table 6. Note that these costs should be considered “order of magnitude” cost estimates.

Table 6: Preliminary Cost Estimate

Description	Preliminary Capital Cost Estimate ¹ (+ HST)	
	Concept Plan A	Concept Plan B
Advanced Treatment System	\$ 1,000,000	\$ 1,500,000
Leaching Beds	\$ 1,000,000	\$ 1,500,000
Total	\$ 2,000,000	\$ 3,000,000
Contingency	+/- 20%	

¹. Does not include professional engineering fees.

4.4 Regulatory Requirements

An ECA will be required for the sewage treatment system, encompassing the treatment works and effluent discharge to the subsurface leaching bed. Crozier recommends engaging in discussions with the MECP to determine what additional studies will be required to support the ECA and their scope. For a subsurface discharge system, Crozier anticipates that at a minimum an impact assessment will be required to confirm the effluent criteria for the treatment system.

If a detailed assessment confirms that surface discharge is a feasible alternative based on the evaluation of nearby potential surface water receivers, the MECP should also be consulted to identify the additional studies required to support the ECA and their scope with this type of effluent discharge option. For a system with surface discharge, Crozier anticipates that at a minimum an assimilative capacity assessment of the identified surface water receiver will be necessary to establish the effluent criteria. The GRCA should also be consulted as the nearby surface water bodies, including Irish Creek to the north of the Site or Puslinch Lake to the southeast of the Site are both potential receivers for the final effluent and are regulated by the GRCA.

4.5 Next Steps and Supporting Studies

The following list outlines the next steps and supporting studies to implement an onsite sewage system with subsurface discharge to a leaching bed.

- Engagement and Pre-Consultation with the MECP
- Impact Assessment (once Hydrogeological Study is complete)
- Detailed Design of the Onsite Sewage System
- Application for an ECA with the MECP

5.0 Private Water Supply Investigation

The following sections describe the feasibility of servicing the proposed development on the northern portion of the Site via private water supply, supporting estimated water demands, and water supply recommendations.

5.1 MECP Water Supply Wells

Within 1 km of the Site, there are 136 water well records available through the MECP Well Record database. The well records describe wells installed from 1952 to 2022 surrounding the Site and can be described in detail below.

- Eighty-one (81) records are identified to be wells for domestic purposes. The remaining records identify monitoring wells, specialty wells (dewatering, injection) or abandonment records within the area.
- Approximately 50% of water supply wells draw from bedrock sources and 50% draw from overburden sources. The wells north of Highway 401 are primarily installed in "rock" or "limestone" as described in the well records. South of Highway 401, the majority of private wells service the community surrounding Puslinch Lake and draw from both bedrock sources and gravel or sand.
- Pumping rates of water supply wells that were installed after 1990 range from 15.1 liters per minute (LPM) to 95.6 LPM. Any pumping rates reported before 1990 pre-date the guidelines for testing set out in Ontario Well Regulation (O.Reg. 903) and may not be a true representation of water quantity.

- Where water quality was reported, all well records note a “clear” sample and no additional water quality notes are mentioned.

The nearest municipal wells are part of the Hespeler Wellfield—H3, H3A, H4A, H5A and H5. H4A is located at 189 Hungerford Road, approximately 1 km west of the Northern Site boundary. H4A is installed 125 mbgs within limestone and on average produces 13.5 L/s.

5.2 Water Demand

As described in Section 3.0 above, the total onsite daily sewage flow is calculated to be 130,500 L/day for the Site. Since all wastewater from the buildings will be directed to the proposed onsite sewage system, it can be reasonably assumed that maximum water demand for Concept A is equal to the maximum daily onsite sewage flow of 130,500 L/day or 90.6 LPM and Concept B is equal to the maximum daily onsite sewage flow of 190,000 L/day or 131.9 LPM.

Using a peaking factor of 2.5, the average daily water demand for Concept A is to 36.2 LPM and for Concept B is 52.76 LPM.

5.3 Hydrostratigraphy

In general, the hydrostratigraphy of the Site and surrounding area is described in detail within the Grand River Source Protection Plan Assessment Report. The water bearing and water restrictive units from youngest to oldest (shallowest to deepest) are described in Table 7 below.

Table 7: Hydrostratigraphy of the Cambridge Area (GRCA, 2010)

Name	Estimated Elevation (masl)	Description	Hydrogeologic Properties
Overburden	~ 320 -290	Sandy silt, till, sand, gravel	Aquifer/aquitard
Guelph Formation	~ 290 -280	Medium to thick bedded dolostone, fossiliferous	Aquifer
Eramosa Formation	absent	Thinly bedded fine dolostone	Aquitard
Goat Island Formation	~ 280 – 260	Chert-rich, fine dolostone and grainstone	Aquifer/Aquitard
Gasport Formation	~ 260 – 190	Cross bedded Grainstone and dolostone	Aquifer

The Guelph and Gasport Formations are significant regional aquifers across the Cambridge area. The Guelph Formation is a medium bedded fossiliferous dolostone that supplies significant water to domestic water wells and municipal wells in the south areas of Cambridge (Galt).

In the Hespeler area near to the Site, the Guelph Formation is interpreted to be relatively thin based on cross section mapping conducted for regional studies. However, the Gasport Formation contains numerous fractures and is highly transmissible, leading to high water supplies from this unit in the Hespeler area.

The Gasport Formation is believed to have a top elevation of roughly 260 masl in the area of the Site. This unit is characterized by light grey, grainstone and dolostone, often field identified in well records as “limestone” or “grey rock”. The Gasport Formation is a significant regional aquifer and supplies numerous municipal wells across the City of Cambridge.

5.4 Proposed Water Supply Recommendations

Based on nearby well records and the estimated water demand for the Site, servicing the proposed buildings on the northern portion of the property is functionally feasible. The majority of water supply wells in the area that draw from bedrock—either the shallower Guelph Formation or the deeper Gasport Formation reportedly produce in excess of 37.8 LPM, greater than the average estimated water demand for the Site of 36.2 LPM and 52.76 LPM. Some wells reportedly produce greater than the maximum daily water demand of 90.6 LPM, installed greater than 50 mbgs.

Preliminary water supply recommendations for the proposed development based on this desktop analysis are as follows:

- During detailed design, a water supply study should be conducted including the construction and testing of test water supply wells. This study will determine the actual capacity of the aquifer to support the proposed development and inform the ultimate private servicing strategy (e.g. one well, multiple wells etc.).
- Please note that an Environmental Activity Sector Registration will be required to facilitate the testing of the proposed water supply well or wells. A Permit to Take Water (PTTW) will be required to continue the operation of the well since the pumping rate will exceed 50,000 L/day.
- During the water supply study, groundwater quality samples from each well, before and after pump testing shall be submitted to a licensed laboratory for analysis. Results shall be compared to the Ontario Drinking Water Quality Standards to determine raw groundwater potability or if treatment or filtration is required.
- An impact assessment for the zone of influence of the proposed onsite water supply well(s) should be completed during detailed design.
- The onsite water supply well(s) should be constructed within the Gasport Formation to a minimum depth of approximately 50 mbgs. The well shall be constructed by a licensed well contractor in accordance with Ontario Regulation 903.
- The onsite water supply well(s) shall be constructed upgradient from the proposed onsite sewage treatment system to prevent contamination. A proposed location has been shown on the Private Servicing Strategy Figure.

- The wellhead shall remain accessible for maintenance and repair in perpetuity.
- No salt or snow storage shall be conducted with the immediate vicinity of the wellhead. Best practices shall be upheld at the Site in terms of Winter Maintenance practices. The Owner should consult with the Source Protection Official to determine a winter maintenance strategy for the Site.

6.0 Conclusions & Recommendations

This assessment was prepared in support of the Zoning By-Law Amendment for the property located at 2809 Townline Road in the Township of Puslinch. Two (2) concept plans are being explored, both of which can be serviced via an onsite sewage system and water supply wells. Our conclusions include:

Proposed Sanitary Services

- The total daily design sewage flow for the proposed development ranges from 130,500 L/day (Concept Plan A) to 190,000 L/day (Concept Plan B). Sanitary servicing will be provided via an onsite sewage system, consisting of an advanced treatment system with discharge of treated effluent to Type A dispersal beds.

Proposed Water Services

- Nearby private services and hydrogeological information suggest a peak daily water demand of 90.6 LPM and 131.9 LPM is theoretically feasible using one to multiple wells respectively. Localized testing shall be completed by the applicant at detailed design and will include quantity and quality testing of the proposed well(s). An impact assessment of the proposed well(s) will be conducted at that time.

Respectively submitted,

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Caitlyn MacPhee, P.Geo., EIT
Hydrogeology

KW:CM/cj

Encl.
Attachment A – Preliminary Design Flows (Concept A and B)
Figures – Private Servicing Strategy (Concept A and B)

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ONSITE SEWAGE SYSTEM NON-RESIDENTIAL CALCULATION SHEET - CONCEPT A

Project Name: 2809 Townline Road
Project Number: 2996-7616

Date: 2025.09.22
Designed By: KW
Checked By: JD

PRELIMINARY FLOW ESTIMATES

Description	Area (m ²)	Occupancy	Unit	Unit Flow	Number of Units	Total Flow (L/day)	References/Notes
Industrial Building 1	15,390	Warehouse	Per water closet, and	950	18	17,100	Assume half of the office floor area is usable work space
			Per loading bay	150	46	6,900	
Industrial Building 2	15,390	Warehouse	Per each 9.3 m ² of floor space	75	43	3,225	
			Per water closet, and	950	18	17,100	
Industrial Building 3	15,390	Warehouse	Per each 9.3 m ² of floor space	75	43	3,225	
			Per water closet, and	950	18	17,100	
Industrial Condo Building 4	9,384	Warehouse	Per each 9.3 m ² of floor space	75	43	3,225	
			Per water closet, and	950	11	10,450	
Industrial Condo Building 5	9,384	Warehouse	Per each 9.3 m ² of floor space	75	56	4,200	
			Per water closet, and	950	11	10,450	
Industrial Condo Building 6	8,833	Warehouse	Per each 9.3 m ² of floor space	75	56	4,200	
			Per water closet, and	950	11	10,450	
	491	Office	Per each 9.3 m ² of floor space	75	52	3,900	
			Per loading bay	150	11	1,650	
Total Daily Sewage Design Flow						130,275	
Sewage System Design Flow:						130,500	
Pre-Treatment Options							
Required septic tank size =	391500	L minimum					
Propose Level IV Treatment (Y/N):	Y						
Native Percolation time, T =	35	min/cm					
Imported Percolation time =	10	min/cm					
Type A Dispersal Bed							
Stone area required =	2610	m ²					
Sand area required =	11419	m ²					

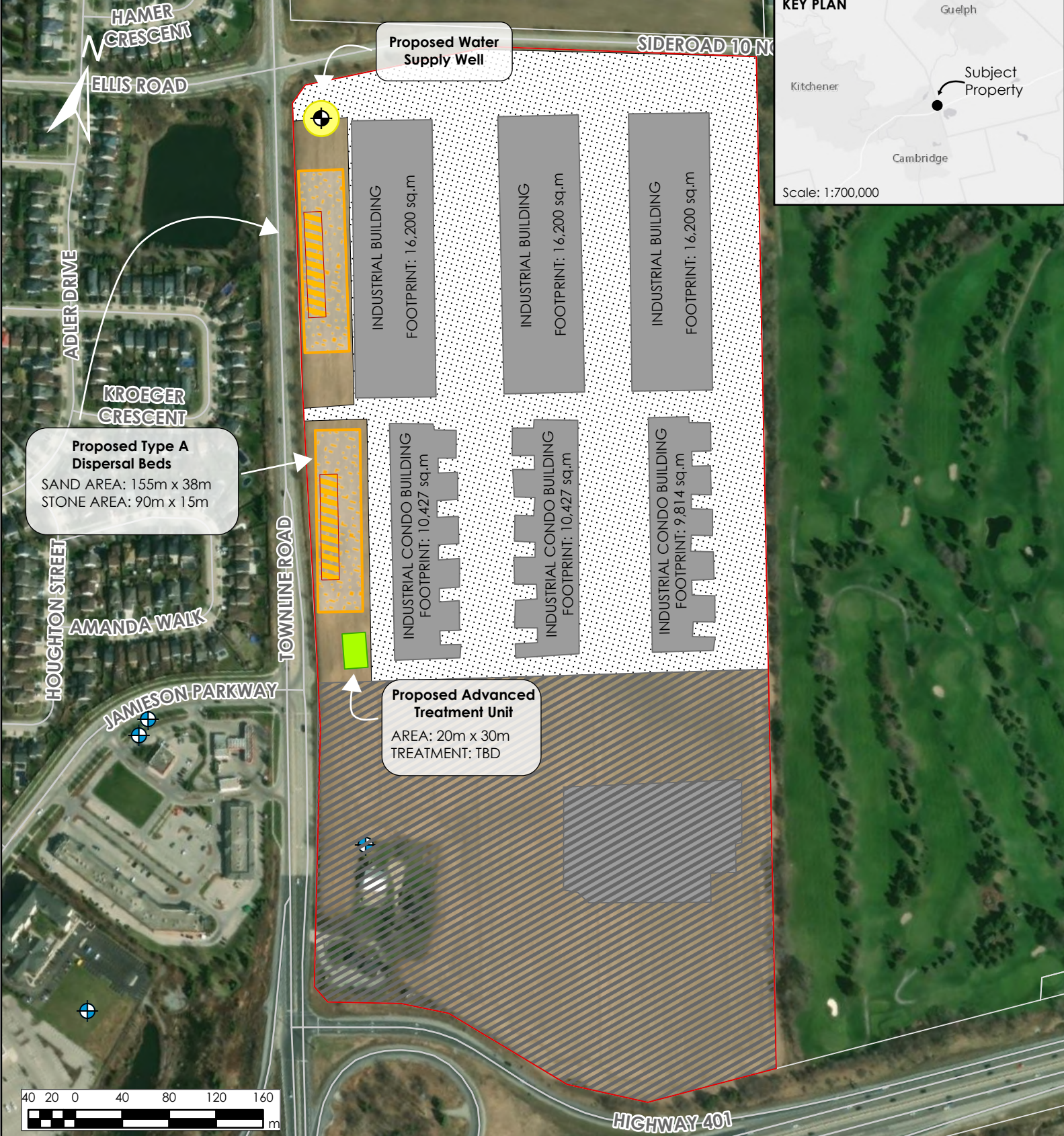


ONSITE SEWAGE SYSTEM NON-RESIDENTIAL CALCULATION SHEET - CONCEPT B

Project Name: 2809 Townline Road
Project Number: 2996-7616

Date: 2025.09.22
Designed By: KW
Checked By: JD

PRELIMINARY FLOW ESTIMATES							References/Notes	
Description	Area (m ²)	Occupancy	Unit	Unit Flow	Number of Units	Total Flow (L/day)		
Industrial Building 1	15,390	Warehouse	Per water closet, and	950	18	17,100	Assume half of the office floor area is usable work space	
			Per loading bay	150	46	6,900		
	405	Office	Per each 9.3 m ² of floor space	75	41	3,099		
Industrial Building 2	15,390	Warehouse	Per water closet, and	950	18	17,100		Assume half of the office floor area is usable work space
			Per loading bay	150	46	6,900		
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225		
Industrial Building 3	15,390	Warehouse	Per water closet, and	950	18	17,100	Assume half of the office floor area is usable work space	
			Per loading bay	150	46	6,900		
	405	Office	Per each 9.3 m ² of floor space	75	43	3,225		
Industrial Condo Building 4	9,384	Warehouse	Per water closet, and	950	11	10,450		Assume half of the office floor area is usable work space
			Per loading bay	150	11	1,650		
	522	Office	Per each 9.3 m ² of floor space	75	56	4,200		
Industrial Condo Building 5	9,384	Warehouse	Per water closet, and	950	11	10,450	Assume half of the office floor area is usable work space	
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Industrial Condo Building 6	8,833	Warehouse	Per water closet, and	950	11	10,450		Assume half of the office floor area is usable work space
			Per loading bay	150	11	1,650		
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Industrial Condo Building 7	18,535	Warehouse	Per water closet, and	950	22	20,900	Assume half of the office floor area is usable work space	
			Per loading bay	150	33	4,950		
	488	Office	Per each 9.3 m ² of floor space	75	52	3,935		
Industrial Condo Building 8	18,535	Warehouse	Per water closet, and	950	22	20,900		Assume half of the office floor area is usable work space
			Per loading bay	150	33	4,950		
	488	Office	Per each 9.3 m ² of floor space	75	52	3,935		
Total Daily Sewage Design Flow						189,720		
Sewage System Design Flow:						190,000		
Pre-Treatment Options								
Required septic tank size =	570000	L minimum						
Propose Level IV Treatment (Y/N):	Y							
Native Percolation time, T =	35	min/cm						
Imported Percolation time =	10	min/cm						
Type A Dispersal Bed								
Stone area required =	3800	m ²						
Sand area required =	16625	m ²						



Proposed Type A Dispersal Beds
 SAND AREA: 155m x 38m
 STONE AREA: 90m x 15m

Proposed Water Supply Well

Proposed Advanced Treatment Unit
 AREA: 20m x 30m
 TREATMENT: TBD

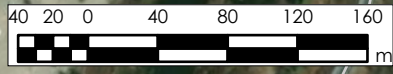
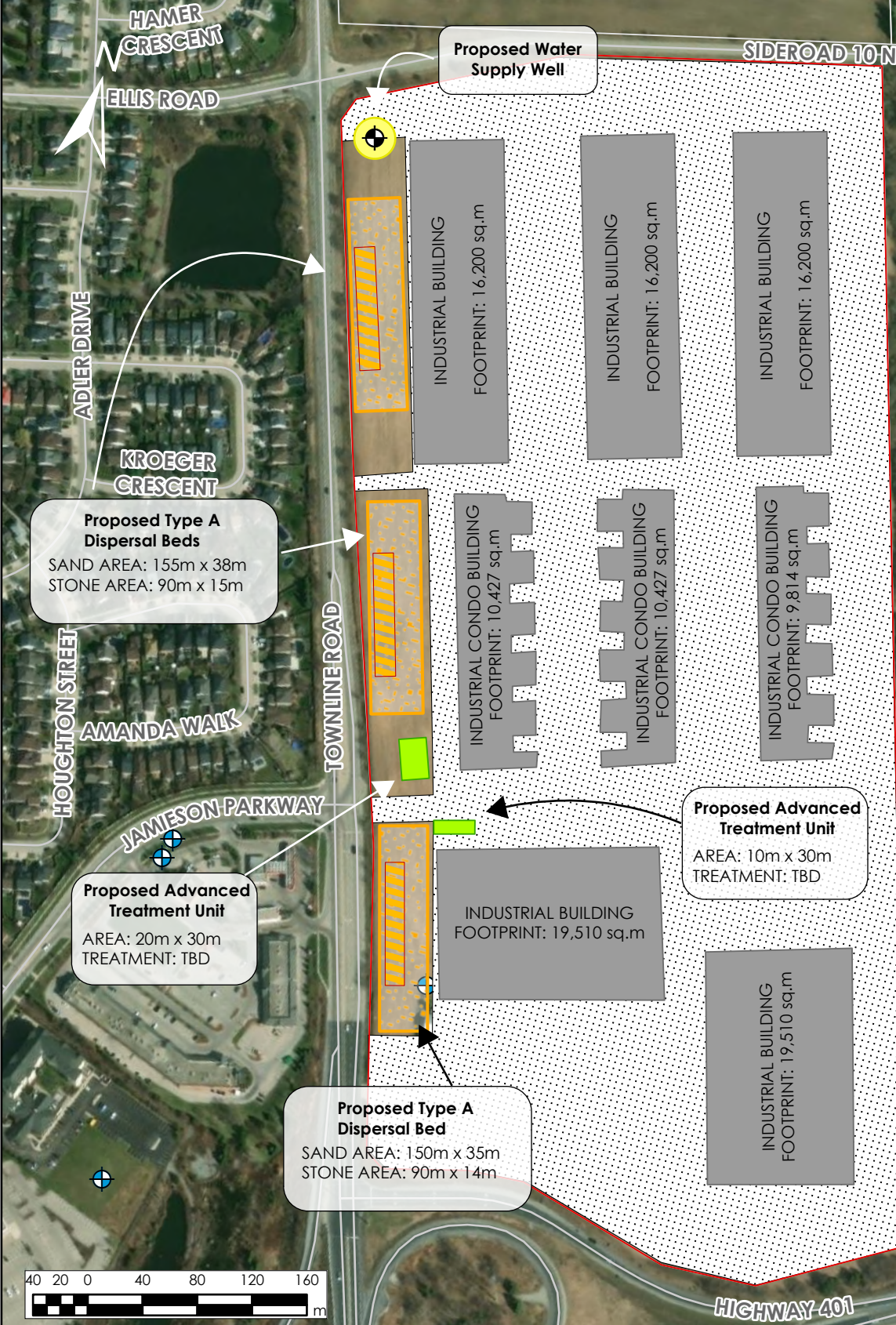
LEGEND

- Subject Property
- Road
- + Proposed Water Supply Well
- + MECP Wells
- 15 m Minimum Setback
- Proposed Buildings
- Proposed Advanced Treatment Unit
- Proposed Type A Dispersal Bed (Stone Area)
- Proposed Type A Dispersal Bed (Sand Area)
- Assessment Parcel (Wellington County)
- Proposed Municipally Serviced Area
- Proposed Parking/Internal Road



**2809 TOWNLINE ROAD
 TOWN OF PUSLINCH**

Proposed Private Servicing
 Strategy - Concept A



LEGEND

- Subject Property
- Road
- + Proposed Water Supply Well
- + MECP Wells
- 15 m Minimum Setback
- Proposed Buildings
- Proposed Advanced Treatment Unit
- Proposed Type A Dispersal Bed (Stone Area)
- Proposed Type A Dispersal Bed (Sand Area)
- Assessment Parcel (Wellington County)
- Proposed Parking/Internal Road

**2809 TOWNLINE ROAD
TOWN OF PUSLINCH**

Proposed Private Servicing Strategy - Concept B